

Gun Club Business Park Traffic Impact Study



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TABLE OF CONTENTS

1.0	Introduction	1
2.0	Project Description	2
3.0	Study Considerations.....	2
3.1	Data Collection.....	2
3.2	Approved Developments and New Roadways	3
3.3	Evaluation Methodology	3
3.4	Level of Service Capacity Analysis	3
4.0	Existing Conditions	4
4.1	Roadways.....	4
4.2	Intersections	5
4.3	Pedestrian and Bicycle.....	5
4.4	Transit	5
4.5	Year 2023 Existing Intersection Capacity Analysis.....	6
5.0	Future Conditions	7
5.1	Annual Growth Factor and Future Volume Methodology.....	7
5.2	Year 2027 Anticipated Transportation Network	9
5.3	Year 2027 Background Intersection Capacity Analysis.....	9
5.4	Year 2050 Planned Transportation Network	10
5.5	Year 2050 Background Intersection Capacity Analysis.....	11
6.0	Future Conditions with the Development	13
6.1	Trip Generation.....	13
6.2	Trip Distribution and Assignment	14
6.3	Proposed Roadway Network and Access	15
6.4	Future Multi-Modal Facilities	17
6.5	Year 2027 Background + Project Intersection Capacity Analysis.....	17
6.6	Year 2050 Background + Project Intersection Capacity Analysis.....	18

7.0	Queuing Analysis	19
8.0	Sensitivity Analysis	20
9.0	Roadway Capacity	21
10.0	Conclusions	21

LIST OF TABLES

Table 1 – Peak Hour Intersection LOS Summary for Existing Intersections.....	25-26
Table 2 – Peak Hour Estimated Queues and Proposed Auxiliary Lanes	27-28
Table 3 – Trip Generation Summary	29
Table 4 – Year 2050 Roadway Level of Service Summary.....	20

LIST OF FIGURES

Figure 1 – Vicinity Map	30
Figure 2 – Site Plan and Access Locations.....	31
Figure 3 – Existing Traffic Volumes	32
Figure 4 – Year 2027 Background Traffic Volumes	33
Figure 5 – Year 2050 Background Traffic Volumes	34
Figure 6A – Trip Distribution – Industrial	35
Figure 6B – Trip Distribution – Retail	36
Figure 6C – Trip Distribution – Retail Pass-By	37
Figure 7 – Site-Generated Traffic Volumes.....	38
Figure 8 – Year 2027 Background + Project Traffic Volumes	39
Figure 9 – Year 2050 Background + Project Traffic Volumes	40

APPENDIX

- Level of Service Definitions
- Existing Traffic Data
- Background Documentation
- Intersection Capacity Worksheets
- Signal Warrant Worksheets
- Internal Capture and Pass-By Calculations
- Sensitivity Analysis

GUN CLUB BUSINESS PARK TRAFFIC IMPACT STUDY

1.0 Introduction

The Fox Tuttle Transportation Group has prepared this traffic impact study for the development of the Gun Club Business Park project. The 188± acres of vacant property are located in the City of Aurora west of Gun Club Road and south of Jewell Avenue. The property is across Gun Club Road from the Murphy Creek Golf Course and adjacent to the recently proposed Aspen Business Industrial Park project. It is understood that the development will serve light industrial businesses and commercial retail. Access to the site is planned to be located on Gun Club Road (SH 30) at two (2) locations with cross access into Aspen Business Park which will have three (3) accesses on Gun Club Road. Majority of the surrounding land is vacant but has been identified for future development of commercial and residential uses for the ever-growing City of Aurora. **Figure 1** provides a vicinity map for the proposed project.

The purpose of this study is to assist in identifying potential traffic impacts within the study area as a result of this project. The traffic study addresses existing, short-term, and long-term peak hour intersection conditions in the study area with and without the project-generated traffic. The information contained in this study is anticipated to be used by the City of Aurora staff in identifying any intersection or roadway deficiencies and potential improvements for the build-out condition and long-term future scenarios. This study focused on the weekday AM and PM peak hours which represents the periods of highest trip generation for the proposed use and adjacent street traffic. The study is consistent with the requirements of the City of Aurora's *Traffic Impact Study Guidelines* (June 2015). The following supporting documents were reviewed and incorporated into this analysis as appropriate:

- *Northeast Area Transportation Study*. City of Aurora Planning & Development Services Department. October 2018.
- *Murphy Creek East*. LSC Transportation Consultants, Inc. March 2019.
- *Parklands Master Traffic Study*. Fox Tuttle Transportation Group. March 2022.
- *Southeast Area Transportation Study*. City of Aurora Planning & Development Services Department. August 2007.
- *Aspen Business Industrial Park Traffic Impact Study*. Fox Tuttle Transportation Group. September 2022.

2.0 Project Description

The Gun Club Business Park project plans to develop vacant land with up to $1,466,170 \pm$ square feet of light/airport industrial businesses and approximately 19,500 square feet of commercial retail space. The project proposes to construct two (2) accesses on Gun Club Road (SH 30). The primary access is proposed to be full movement and signalized and aligned with the future Addison Avenue that will travel east of Gun Club Road (SH 30). A secondary access (North Access) is planned to be constructed approximately 1,265 feet north of the Addison Avenue and restricted to $\frac{3}{4}$ movement with side-street stop-controlled. There will be cross access from the Gun Club Business Park property into the Aspen Business Park to the south, which has another three (3) accesses on Gun Club Road that could be utilized by traffic to/from the Gun Club Business Park project. Each of those accesses has varying permitted movements, including a full-movement, signalized intersection at Yale Avenue and then one $\frac{3}{4}$ movement access and one right-in, right-out access. Refer to **Section 6.3** for additional discussion on the proposed access. Internally, local streets will be built to provide the most beneficial access into and around the site. For the purpose of this traffic study, it was assumed that the Gun Club Business Park project will be completed by Year 2027. **Figure 2** shows the site plan and proposed access locations.

3.0 Study Considerations

3.1 Data Collection

Intersection turning movement volumes were collected in October/November 2023 at three (3) existing intersections during the weekday AM and PM peak hours, including pedestrians and bicyclists. Daily traffic volumes were also collected on Jewell Avenue west of Gun Club Road and on Gun Club Road south of Jewell Avenue for 72 hours. Existing and historic traffic volumes on the study roadways were gathered from nearby development traffic studies, City of Aurora's NEATS and SEATS reports, Colorado Department of Transportation's (CDOT) Transportation Data Management System (TDMS) and CDOT's Online Transportation Information System (OTIS).

The existing traffic volumes are illustrated on **Figure 3**. The existing intersection geometry and traffic control are also shown on this figure. Count data sheets are provided in the **Appendix**.

3.2 Approved Developments and New Roadways

The East Aurora area is continuing to grow with new developments and new roadways to provide connections for current and future land uses. Adjacent to the Gun Club Business Park property, the following known development projects are approved, in review, or under construction: (1) Murphy Creek East, (2) Parklands I, (3) Parklands II, (4) Waterstone, and (5) Aspen Business Industrial Park. The trips from each of these developments were assumed to be included within the background growth in NEATS and SEATS.

In the future, Yale Avenue will be constructed as a four-lane minor arterial extending east of Gun Club Road to Watkins Road. Addison Avenue will be constructed as a two-lane collector roadway east of Gun Club Road into Murphy Creek. These roadways will likely be built as Murphy Creek is developed.

3.3 Evaluation Methodology

The traffic operations analysis addressed the signalized and unsignalized intersection operations using the procedures and methodologies set forth by the Highway Capacity Manual (HCM)¹. Existing peak hour factors (PHF) by approach and peak hour were applied to the study intersections for the existing scenarios. For long-term future scenarios, the PHF were set to 0.98 on Gun Club Road and 0.92 on other roadways in the future unless the existing PHF was greater than these values. Study intersections were evaluated using Synchro software (v11).

3.4 Level of Service Capacity Analysis

A Level of Service analysis was conducted to determine the existing and future performance of the study area intersections and accesses to determine the most appropriate intersection traffic controls and auxiliary lanes for future conditions.

To measure and describe the operational status of the study intersections, transportation engineers and planners commonly use a grading system referred to as “Level of Service” (LOS) that is defined by the HCM. LOS characterizes the operational conditions of an intersections traffic flow, ranging from LOS A (indicating very good, free flow operations) and LOS F (indicating congested and sometimes oversaturated conditions). These grades represent the perspective of drivers and are an indication of the comfort and

¹ Highway Capacity Manual, Highway Research Board Special Report 209, Transportation Research Board, National Research Council, 6th Edition (2016).

convenience associated with traveling through the intersections. The intersection LOS is represented as a delay in seconds per vehicle for the intersection as a whole and for each turning movement.

Typically, LOS A through C is considered to be acceptable for the overall intersection operations and LOS D overall during peak hours is acceptable. Individual movements may be allowed to fall to LOS E at signalized intersections. Minor movements at unsignalized intersections, such as left turns onto a major arterial, may be allowed to fall below LOS D, specifically where there are low volumes and/or no viable alternative per the City of Aurora's *Traffic Impact Study Guidelines*. Criteria contained in the *HCM* was applied for these analyses in order to determine peak hour LOS for each scenario. A more detailed discussion of LOS methodology is contained in the **Appendix** for reference.

4.0 Existing Conditions

4.1 Roadways

The study area boundaries are based on the amount of traffic to be generated by the project and potential impact to the existing roadway network. The primary public roadways that serve the project site are discussed in the following text and illustrated on **Figure 1**.

E-470 is a six-lane divided toll highway that provides regional access around the eastern and northern Denver metro area. The toll road currently extends 47 miles from C-470 at I-25 in Douglas County (west of the project site) to I-25 near 160th Avenue in Thornton where it becomes the Northwest Parkway. E-470 connects three counties, six municipalities, and Denver International Airport. E-470 has full-movement interchanges at Jewell Avenue. This expressway has a posted speed limit of 75 miles per hour (mph) and serves approximately 36,000 vehicles per day (vpd) near the interchange with Jewell Avenue (CDOT, Year 2022).

Gun Club Road (State Highway 30) is a two-lane, north-south, regional roadway that provides access through rural, urban, and growing parts of eastern Aurora, connecting the study area to an interchange at I-70/E-470 to the north and transitioning to Aurora Parkway to the south. Adjacent to the project property, Gun Club Road provides access to the Denver-Arapahoe Landfill. Gun Club Road is a CDOT facility that is classified as NR-B north of Yale Avenue and NR-A south of Yale Avenue per data provided in CDOT OTIS database. Adjacent to the project property, this roadway is classified as NR-B. Refer to **Section 6.3** for discussion on access. South of Jewell Avenue this roadway serves roughly 22,310 vpd (Year 2023) and has a posted speed limit of 55 mph.

Jewell Avenue is an east-west roadway that provides local access to the Murphy Creek neighborhood, rural homes to the east, and E-470 to the west. Jewell Avenue has transitions from two-lanes to four-lanes within the study area. Between E-470 and SH 30, Jewell Avenue is a two-lane major arterial with a posted speed limit of 45 mph. East of SH 30, Jewell Avenue becomes four-lanes along the Murphy Creek community (roughly $\frac{1}{2}$ mile) and then narrows back to two lanes with the speed remaining 45 mph. Jewell Ave carries roughly 13,720 vpd between E-470 and Gun Club Road (Year 2023) and 7,300 vpd east of SH 30 (CDOT, Year 2022).

4.2 Intersections

The study area includes three (3) existing intersections that are listed below with the current traffic control and were analyzed for existing and future background year traffic operations:

1. Jewell Avenue at E-470 Southbound Ramps [stop-controlled]
2. Jewell Avenue at E-470 Northbound Ramps [stop-controlled]
3. Gun Club Road (SH 30) at Jewell Avenue [signalized]

The existing lane configuration at each of the study locations is illustrated on **Figure 3**.

4.3 Pedestrian and Bicycle

Currently, there are segments of sidewalk on both sides of Jewell Avenue east of Gun Club Road (SH 30). There are no sidewalks on Jewell Avenue west of Gun Club Road, or on Gun Club Road within the study area.

The High Plains Trail/E-470 Regional Trail follows the alignment of the E-470 toll road to the west of the study area, connecting several communities including the Town of Parker, City of Aurora, and City of Lone Tree. This multi-use path provides access to Chatfield Reservoir, the Platte River Trail, the Cherry Creek Trail, and several local recreational/commuter trails.

There are no on-street bike facilities or designated bike routes within the project study area. Bikes are encouraged to travel on the multi-use paths and are permitted to travel within general purpose lanes or shoulders of the study roadways.

4.4 Transit

The City of Aurora is serviced by Regional Transportation District (RTD). Currently, there is no transit service within the study area.

4.5 Year 2023 Existing Intersection Capacity Analysis

The existing volumes, lane configuration, and traffic control are illustrated on **Figure 3**. The details of LOS for each movement are provided in **Table 1** and the 95th percentile queues are provided in **Table 2** (refer to **Appendix**). The intersection Level of Service worksheets are attached in the **Appendix**. **Currently, the study intersections experience congestion and high delays, based on the capacity analysis.** The following study intersections currently operate at LOS E/F or have movements that operate at LOS E/F during the one of both peak hours:

- **#1 – Jewell Avenue at E-470 Southbound Ramps:** This stop-controlled intersection currently operates overall at LOS A in both peak hours; however, the southbound left-turn/through lane was estimated to operate at LOS F in both peak hours. The 95th percentile queues for this movement were calculated to be up to 60 feet (about three vehicles).

Recommendations: Signalize. The existing volumes at this intersection meet the signal warrant thresholds for Warrant 2 (4-hour) in 10 hours and the Warrant 3 (peak hour) in both peak hours². With the change in traffic control, the overall operations were estimated to be LOS B in both peak hours with all movements operating at LOS D or better in both peak hours. Refer to **Table 1** for the improved level of service and **Table 2** for reduced queues. It is understood that the City of Aurora is installing a temporary signal at this intersection before Year 2025.

- **#2 – Jewell Avenue at E-470 Northbound Ramps:** This stop-controlled intersection currently operates overall at LOS F in AM peak hour and LOS D in the PM peak hour due to the delay calculated for the northbound left-turn/through movement. The 95th percentile queues for this movement were calculated to be up to 445 feet (about 18 vehicles).

Recommendations: Signalize. The existing volumes at this intersection meet the signal warrant thresholds for Warrant 2 (4-hour) in nine (9) hours and the Warrant 3 (peak hour) in both peak hours. With the change in traffic control, the overall operations were estimated to be LOS B in both peak hours with all movements operating at LOS C or better in both peak hours. Refer to **Table 1** for the improved level of service and **Table 2** for reduced queues. It is understood that the City of Aurora is installing a temporary signal at this intersection before Year 2025.

² The *Manual on Uniform Traffic Control Devices* (MUTCD) signal Warrant 3 for the peak hour was utilized, for planning purposes, to determine if a signal should be considered as the traffic control. These intersections were further analyzed using MUTCD Warrant 2 for Four-Hour Vehicular Volume. Peak hour volumes were extrapolated to previous and later hours by utilizing hourly data provided in the daily roadway counts.

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- **#3 – Gun Club Road (SH 30) at Jewell Avenue:** This signalized intersection currently operates overall at LOS E in the AM peak hour and LOS D in the PM peak hour. There are several movements that currently operate at LOS E/F in one or both peak hour due to the high volumes on all approaches and limited green time. During the morning peak hour, the eastbound through, westbound left-turn, and westbound through movements operate at LOS E/F. During the afternoon peak hour, the same movements were estimated to operate at LOS F. The 95th percentile queue in the westbound left-turn lane is anticipated to be maintained in the existing storage length. The eastbound/westbound through movements were estimated to typically clear in two cycle lengths and 95th percentile queues were less than 450 feet.

Recommendations: During the AM peak hour, consider providing an overlap phase for the eastbound right-turn movement and retiming the signal to provide more green time to Jewell Avenue, which can be accomplished without sacrificing operational performance on Gun Club Road (SH 30). If 10 seconds were redistributed from the main through phase to the side-street through phase, then the overall LOS would improve to LOS D and the delay on the eastbound and westbound movements would decrease significantly.

During the PM peak hour, consider providing an overlap phase for the eastbound right-turn movement and retiming the signal to provide more green time to Jewell Avenue, which can be accomplished without sacrificing operational performance on Gun Club Road (SH 30). If 10 seconds were redistributed from the main through phase to the side street through phase, then the delay on the eastbound and westbound movements would decrease significantly. Refer to **Table 1** for the improved level of service and **Table 2** for reduced queues.

5.0 Future Conditions

5.1 Annual Growth Factor and Future Volume Methodology

In order to forecast the future peak hour traffic volumes, background traffic growth assumptions were estimated based on the City's NEATS Refresh report, Murphy Creek East Traffic Study, Parklands Traffic Impact Study, and CDOT 20-year factors. The listed studies incorporated vehicular traffic associated with the Gun Club Business Park project and nearby developments.

Based on CDOT data, the 20-Year factor on Gun Club Road was 1.79 between Jewell Avenue and Hampden Avenue, which equates to an annual growth rate of 3.0%. This was applied to the existing intersection volumes for the Year 2027 background scenario, and this growth is assumed to include some completion of the nearby developments listed in **Section 3.2**.

Year 2050 roadway volumes from the NEATS Refresh was assumed to include the site-generated trips from developments within and near the study area, as well as the trips associated with Gun Club Business Park and adjacent developments. The following methodology was used to attain the Year 2050 background peak hour traffic projections:

1. Utilized the 2040 Background + Project volumes in the Aspen Business Industrial Park Traffic Impact Study through the study intersections for the baseline for forecasting into the long-term future. This adjacent project utilized the forecasts in the Parklands project, which was based on NEATS Refresh roadway volumes to determine the future volumes through the intersections. The turning movements were based on the existing travel patterns and anticipated future roadway network. Refer to **Appendix** for relevant pages.
2. Included the volumes to/from Yale Avenue and Addison Avenue as estimated in the Murphy Creek East Traffic Study (refer to **Appendix** for relevant pages). Adjusted through volumes on Gun Club Road to balance the roadway network.
3. Compared the ‘Ultimate Roadway Volumes’ from the NEATS Refresh study to determine the anticipated growth by Year 2050. It was calculated that there will be approximately 2.0% annual growth within the study area from Year 2040 to Year 2050. This growth rate was applied to the intersection volumes to estimate the peak hour volumes. This was considered the “Year 2050 background + project” scenario as Gun Club Business Park traffic was included in the regional model.
4. Estimated the site-generated trips for the Gun Club Business Park project and assigned to the roadway network. Refer to **Section 6.0** for more details on trip generation, distribution, and assignment assumptions.
5. Subtracted the Gun Club Business Park trips from the “Year 2050 background + project volumes” from Step 3. These final volumes represent the “Year 2050 background” scenario. This ensures the project trips are not double counted since they were included in the NEATS Refresh forecasts.

Using these assumptions, the Year 2027 background traffic is summarized on **Figure 4** and the Year 2050 background traffic is summarized on **Figure 5**.

5.2 Year 2027 Anticipated Transportation Network

For comparison purposes, this traffic study assumes that some of the planned auxiliary lanes and mitigation measures listed previously would be included in the short-term background scenario. The following roadway and intersection improvements were assumed to be completed by Year 2027:

- **Gun Club Road (SH 30)** – Widen to two (2) lanes per direction.
- **Yale Avenue** – Construct east and west of Gun Club Road. To the east, this roadway will have two (2) lanes per direction and to the west it will be one (1) lane per direction.
- **Addison Avenue** – Construct east of Gun Club Road with one (1) lane per direction.
- **Jewell Avenue at E-470 Southbound Ramps** – Signalize [recommended in Existing scenario].
- **Jewell Avenue at E-470 Northbound Ramps** – Signalize [recommended in Existing scenario].
- **Gun Club Road (SH 30) at Yale Avenue** – Signalize as determined in the Murphy Creek East Traffic Study. Construct the east leg with two left-turn lanes and one right-turn lane. Construct one northbound right-turn lane and one southbound left-turn lane.

These roadway and intersection improvements were assumed to be in place in the background condition for the short-term scenario for comparison purposes with the project trips and are shown on **Figure 4**.

5.3 Year 2027 Background Intersection Capacity Analysis

The study area intersections were evaluated to determine baseline operations for the Year 2027 background scenario and to identify any capacity constraints associated with background traffic (refer to **Section 5.1** for growth assumptions). It was assumed that the roadway and intersection improvements listed in **Section 5.2** will be implemented by Year 2027 background. The background volumes, lane configuration, and traffic control are illustrated on **Figure 4**.

The Level of Service criteria discussed previously was applied to the study area intersections to determine the impacts with the short-term background volumes. This analysis assumes signal timing throughout the network would be adjusted to accommodate the additional lanes and changes in traffic volumes. The details of LOS for each movement are provided in **Table 1** and the 95th percentile queues are provided in **Table 2** (refer to **Appendix**). The intersection Level of Service worksheets are attached in the **Appendix**.

In summary, the majority of the study intersections were estimated to operate at LOS D or better in the Year 2027 background condition in both peak hours with majority of movements operating at LOS D or

better. The following intersections or movements were calculated to begin operating at LOS E/F in one or both peak hour in Year 2027 background as described below:

- **#3 – Gun Club Road (SH 30) at Jewell Avenue:** This signalized intersection was calculated to operate overall at LOS D in both peak hours. During the morning peak hour, the eastbound left-turn, westbound left-turn, westbound through, and northbound left-turn movements were estimated to operate at LOS E. During the afternoon peak hour, the eastbound left-turn, eastbound through, westbound left-turn, and northbound left-turn were estimated to operate at LOS E. The 95th percentile queues for the eastbound left-turn were calculated to be up to 122 feet (about five vehicles) and the queues for the westbound left-turn were calculated to be up to 173 feet (about seven vehicles). The 95th percentile queues for the northbound left-turn were calculated to be up to 211 feet (about nine vehicles). All of these queues can be maintained within the existing storage lengths.

Recommendations: No mitigation measures are recommended. The delays are less than the signal cycle length which indicates vehicles can clear in one cycle.

- **#103. Gun Club Road at Addison Avenue:** This future intersection was estimated to operate overall at LOS A in both peak hours with the westbound left-turn estimated to operate at LOS F in the PM peak hour. The high delay is related to the difficulty to turn onto Gun Club Road due to high volumes.

Recommendations: No mitigation measures are recommended. The intersection volumes are not approaching the signal warrant thresholds. If the experienced delays are uncomfortable for drivers, it is likely they will decide to turn right instead of left or use a different route. Monitor this intersection for safety.

5.4 Year 2050 Planned Transportation Network

For comparison purposes, this traffic study assumes that some of the planned roadways in NEATS Refresh and SEATS reports, as well as a new intersection and auxiliary lanes shown in other development traffic studies will be completed by Year 2050 background. If the future scenarios were evaluated with the existing roadway network, then it would be difficult to compare intersection operates when many of the intersections will be altered due to future volumes, capacity, and routing that are not associated with the project.

In addition to the improvements listed for Year 2027, the following roadway and intersection improvements were assumed to be completed by Year 2050:

- **Gun Club Road (SH 30)** – Widen to three (3) lanes per direction.
- **Jewell Avenue** – Widen to three (3) lanes per direction.
- **Jewell Avenue at E-470 Southbound Ramps** – Provide two (2) westbound left-turn lanes. With the widening of Jewell Avenue, continue to provide one (1) eastbound right-turn lane. The southbound right-turn lane volume indicated the need for channelization.
- **Jewell Avenue at E-470 Northbound Ramps** – Provide two (2) eastbound left-turn lanes. With the widening of Jewell Avenue, continue to provide one (1) westbound right-turn lane. The northbound right-turn lane volume indicated the need for channelization.
- **Gun Club Road (SH 30) at Jewell Avenue** – Provide two (2) left-turn lanes, three (3) through lanes, and one (1) right-turn lane on all four approaches. Consider constructing the northbound and eastbound right-turn as channelized. Optimize signal timing for future conditions.
- **Gun Club Road (SH 30) at Yale Avenue** – Provide a second southbound left-turn lane.

These roadway and intersection improvements were assumed to be in place in the background condition for the long-term scenario for comparison purposes with the project trips and are shown on **Figure 5**.

5.5 Year 2050 Background Intersection Capacity Analysis

The study area intersections were evaluated to determine baseline operations for the Year 2050 background scenario and to identify any capacity constraints associated with background traffic in the long-term scenario (refer to **Section 5.1** for growth assumptions). The long-term background volumes, lane configuration, and traffic control are illustrated on **Figure 5**.

The Level of Service criteria discussed previously was applied to the study area intersections to determine the impacts with the long-term background volumes. The analysis assumed the signal timing at all signalized intersections would be adjusted to accommodate the additional lanes and change in traffic volumes. It should be noted that the peak hour factors were adjusted to 0.98 on Gun Club Road and 0.92 on other roadways (if the existing factor is less than these values) since it is assumed that the peak periods will become longer with peak hour traffic spread more evenly over the hour as traffic increases than is experienced today.

The details of LOS for each movement are provided in **Table 1** and the 95th percentile queues are provided in **Table 2** (refer to **Appendix**). The intersection Level of Service worksheets are attached in the **Appendix**.

In summary, majority of the of the study intersections were estimated to operate overall at LOS D or better in both peak periods and the majority of movements were calculated to operate at LOS D or better. The following intersections or movements were calculated to operate at LOS E/F in one or both peak hour in Year 2050 background as described below:

- **#3 – Gun Club Road (SH 30) at Jewell Avenue:** This signalized intersection was estimated to operate at LOS F in both peak hours due to several movements operating at LOS E/F during one or both peak hours. All four left-turn movements were calculated to operate at LOS E/F in both peak hours due to the high volumes, protected only phasing, and limited green time. The 95th percentile queues for the eastbound left-turn were calculated to be up to 389 feet and for the westbound left-turn the queues were calculated to be up to 488 feet. The 95th percentile queues for the northbound left-turn were calculated to be up to 298 feet and for the southbound left-turn the queues were calculated to be up to 451 feet. During the AM peak hour, the westbound through, northbound through, and southbound through movements were estimated to operate below LOS D. During the PM peak hour, the through movements on all four (4) approaches were estimated to operate at LOS E/F.

Recommendations: No mitigation measures recommended. This intersection is built out with the auxiliary lanes necessary to accommodate the volumes. It is expected that protected-only left-turns will operate below LOS D at large intersections with limited green time to serve the high volumes on all approaches. When this intersection is widened, ensure the auxiliary lanes have adequate storage length to minimize impacts between adjacent turn and through lanes. If this level of delay is experienced, then there is potential that this intersection will need to be redesigned with non-traditional or grade-separated options to reduce safety concerns and improve efficiency. The exact intersection design was not evaluated as part of this traffic study.

6.0 Future Conditions with the Development

The Gun Club Business Park is anticipated to include light industrial uses and a small area of commercial retail space and for the purpose of this traffic study, it was assumed that the entire project will be complete by Year 2027.

6.1 Trip Generation

A trip generation estimate was performed to determine the traffic characteristics of the proposed development. The trip rates contained in the Institute of Transportation Engineers (ITE) *Trip Generation Handbook and Manual*³ were applied to estimate the traffic. The exact type of industrial land use is not known at this time; therefore, this traffic study applied the trip rates for “Industrial Park” [ITE #130], “Strip Retail Plaza (<40k)” [ITE 822], and “Fast Casual Restaurant” [ITE 930]. For the purpose of this analysis, it was assumed that the site will be constructed and occupied by Year 2027.

Table 3 provides the detailed trip generation estimates for the Gun Club Business Park project (refer to the **Appendix**). The proposed project is expected to experience mostly new trips, also known as ‘primary trips’, as well as non-auto trips which are discussed below:

Primary Trips. These trips are made specifically to visit the site and are considered “new” trips. Primary trips would not have been made if the proposed project did not exist. Therefore, this is the only trip type that increases the total number of trips made on a regional basis.

Non-Auto Trips. These trips are those that are completed by carpool, walking, biking, or transit. The non-auto trips were assumed to be 5% which includes any trips that remain internal to the site between two businesses.

Multi-Use (Internal) Trips. These internal trips occur from one land use or building to another within the site boundaries. Multi-use or multi-purpose trips typically do not affect the exterior site access points, nor add any additional traffic volumes to the adjacent street network. It is anticipated there will be some internal trips within Gun Club Business Park due to the mix of uses, especially between commercial businesses. ITE recommends that the internal capture calculations be conducted with the methodology presented in National Cooperative Highway Research Program’s (NCHRP) *Report 684: Enhancing Internal Trip Capture Estimation for Mixed-*

³ *Trip Generation Handbook and Manual, 11th Edition*, Institute of Transportation Engineers, 2021.

Use Developments⁴ which were utilized in this traffic study. Based on this methodology, it was calculated that the morning peak hour will have an internal capture rate of 1% and the evening peak hour will have an internal capture rate of 8%, which were applied to the trip estimate.

Pass-By Trips. A pass-by trip is an intermediate stop on route from an origin to the ultimate trip destination without route diversion. These are drivers that already utilize the adjacent roadways and choose to make a stop within the site and then continue to their destination. Pass-by trips do not create any increase in the traffic volumes within the primary impact area. In fact, the only impact of the pass-by trips is at the site driveways and adjacent intersections where through movements become turning movements into and out of the site. Therefore, pass-by trips have no additional impact on the road system beyond the site's driveways or immediately adjacent intersections. With or without pass-by trips, the total trips to/from a project will remain the same. Pass-by was only applied to the commercial retail portion of the project. Per ITE data, the pass-by percentage for retail is 40% and for the restaurant is 43% in the PM peak hour. For pass-by trips, the methodology set forth in the Trip Generation Manual (Chapter 10) was utilized. Calculations are provided in the **Appendix**.

The Gun Club Business Park project was estimated to generate approximately 5,004 daily trips with 507 trips in the AM peak hour and 521 trips in the PM peak hour. Per the ITE Trip Generation data, truck traffic is approximately 7% of the daily site traffic and 3% during the AM and PM peak hours. This is detailed on **Table 3**.

6.2 Trip Distribution and Assignment

The estimated trip volumes were distributed onto the study area street network based on existing traffic characteristics, land uses, and traffic patterns in the area, as well as regional growth and future roadway infrastructure. The existing volumes were utilized to determine where vehicles are coming from and going to within the study area, plus the route to get to major highways and anticipated destinations.

⁴

NCHRP Report 684: *Enhancing Internal Trip Capture Estimation for Mixed-Use Developments*. Bochner, B., K. Hooper, B. Sperry, and R. Dunphy. Washington, DC: Transportation Research Board, 2011.

The following distributions were assumed for this project and are shown on **Figure 6**:

- North E-470 via Jewell Avenue: 20%
- South E-470 via Jewell Avenue: 15%
- North Gun Club Road (SH 30): 15%
- South Gun Club Road (SH 30): 30%
- West Jewell Avenue via Gun Club Road: 10%
- East Jewell Avenue via Gun Club Road: 10%

Using the distribution assumptions, the projected site traffic was assigned to the study area roadway network for the weekday AM and PM peak hour periods. The different land uses were assumed to use different access points to have the shortest route to their destination. The trip distribution for the industrial land use is shown on **Figure 7A**, the trip distribution for commercial retail is shown on **Figure 7B**, and the pass-by trip distribution is shown on **Figure 7C**.

6.3 Proposed Roadway Network and Access

Access to the Gun Club Business Park site is planned via two (2) driveways along Gun Club Road (SH 30). Additionally, project traffic will have the opportunity to use the three (3) accesses on Gun Club Road that will be constructed with the Aspen Business Industrial Park project due to an internal roadway connection. The primary access (middle access) is proposed to align with the future Addison Avenue which will be signalized in the long-term scenario per the *Murphy Creek East Traffic Study*. With the Gun Club Business Park project, it is anticipated that their primary access will need to include one eastbound left-turn lane, one eastbound through/right-turn lane, one northbound left-turn lane, and one southbound right-turn lane (based on auxiliary lane volume thresholds). The secondary access is proposed to be located on the north edge of the property and be restricted to $\frac{3}{4}$ movement. Trips from the Gun Club Business Park are anticipated to also use the north access (3/4 movement) of Aspen Business Industrial Park and the signal at Yale Avenue.

A secondary access (north access) is anticipated to need one eastbound left-turn lane, one eastbound through/right-turn lane, one northbound left-turn lane, and one southbound right-turn lane (based on auxiliary lane volume thresholds).

Gun Club Road (SH 30) is a CDOT facility and classified as NR-B along the project property per the CDOT OTIS database. The *State Highway Access Code (March 2002)* states that one (1) access will be granted per parcel and additional access will be allowed to relieve congestion on the roadway system, to maintain good highway design, and to maintain good land use design without causing hardship to an adjacent property or roadway system. The proposed accesses for the Gun Club Business Park project are anticipated to serve the site and future adjacent developments, provide good internal circulation, provide emergency access to all buildings, and reduce the width of the Addison Avenue approaches without impacting the roadway system or highway traffic flow, as described below:

- **Addison Avenue/Middle Access** is proposed to be the only full movement access for this project and be signalized. This access will also serve the future development of Murphy Creek to the east of Gun Club Road and provide additional east-west connectivity for the City of Aurora. This full movement access is approximately one-half mile from the signalized intersection at Jewell Avenue (to the north) and one-half mile from the future signal at Yale Avenue (to the south).
- **North Access** is proposed to be restricted to $\frac{3}{4}$ movement access and located approximately $\frac{1}{4}$ mile (1,265 feet) north of Addison Avenue/Middle Access and approximately 0.3-mile (1,520 feet) south of Jewell Avenue. This access will provide a second left-turn into the site and reduce southbound traffic on Gun Club Road as site traffic can enter the property prior to the Addison Avenue signal. The proposed southbound right-turn will reduce the impact of site-generated trips on the through traffic. The analysis indicates that this access will not negatively impact Gun Club Road (SH 30) or the traffic flow.
- **South Access** is proposed to be restricted to $\frac{3}{4}$ movement access (per *Aspen Business Park Traffic Impact Study*) and located approximately 1,125 feet south of the Addison Avenue/Middle Access. This access will be shared with Aspen Business Park. This access will provide another left-turn into the site and reduce northbound traffic on Gun Club Road. The proposed southbound right-turn will reduce the impact of site-generated trips on the through traffic. The analysis indicates that this access will not negatively impact Gun Club Road (SH 30) or the traffic flow.

All proposed accesses will require an access permit for approval by CDOT. The access permits will be submitted separately.

The proposed access intersections to accommodate the proposed trip volume are illustrated on **Figure 7**. The anticipated lane configuration and traffic control at the access and internal intersections are shown on **Figure 8** for Year 2027 and **Figure 9** for Year 2050. The need for turn lanes was based on turn volume, opposing volume, operations, and roadway classification. Internally, the circulating streets will be

constructed to provide the most beneficial access into and around the site with pedestrian and bicycle friendly amenities.

6.4 Future Multi-Modal Facilities

The Gun Club Business Park project proposes to have sidewalks throughout the property to connect internally and externally. Sidewalks will be included along the project frontage on Gun Club Road. Refer to the design plans for the locations, widths, and connections of the pedestrian and bicycle facilities associated with this project.

According to *NEATS Refresh* report, Jewell Avenue has been identified to be a primary bike route and provide future high frequency transit services. An enhanced grade-separated crossing was identified to be installed on Jewell Avenue at Murphy Creek trail. Gun Club Road is identified as a route for high frequency transit services in the future and pedestrian/bicyclist travel is anticipated to be off-street on parallel multi-use trails along Murphy Creek and E-470.

6.5 Year 2027 Background + Project Intersection Capacity Analysis

This section discusses impacts associated with the addition of the Gun Club Business Park development trips in the short-term scenario. The site-generated volumes were added to the Year 2027 background volumes and are illustrated on **Figure 8**. This figure also illustrates the necessary traffic control and lane configurations for all of the study intersections and proposed accesses. The recommended improvements in the Year 2027 background scenario were assumed to be implemented.

The study intersections are anticipated to operate similarly to the background conditions with the addition of project trips since the majority of the intersection and movement levels of service remain the same letter grade. The details of LOS for each movement are provided in **Table 1** and the 95th percentile queues are provided in **Table 2** (refer to **Appendix**). The intersection Level of Service worksheets are attached in the **Appendix**.

The following intersections are anticipated to have one or more movements that begin to operate below LOS D with the additional project trips:

- **#3 – Gun Club Road (SH 30) at Jewell Avenue:** This signalized intersection was estimated to continue to operate at LOS D in both peak hours. During the PM peak hour, the northbound left-turn was estimated to begin operating at LOS F. This is due to the limited capacity of two busy roadways. The 95th percentile queue was calculated to be up to 374 feet.

Recommendations: No mitigation measures recommended since this intersection is built out and it is typical for arterial arterial intersections to have several movements experiencing delays equating to LOS E/F due to limited green time to serve several high-volume movements. Adjust the signal timing and phasing as traffic grows and patterns change to reduce delays as most appropriate.

6.6 Year 2050 Background + Project Intersection Capacity Analysis

This section discusses impacts associated with the addition of the Gun Club Business Park development trips in the long-term scenario. The site-generated volumes were added to the Year 2050 background volumes and are illustrated on **Figure 9**. This figure also illustrates the necessary traffic control and lane configurations for all of the study intersections and proposed accesses. The recommended improvements in the Year 2050 background scenario were assumed to be implemented.

The study intersections are anticipated to operate similarly to the background conditions with the project trips. The majority of the intersection and movement levels of service were estimated to the same letter grade with the majority of increased delays under 20 seconds. The details of LOS for each movement are provided in **Table 1** and the 95th percentile queues are provided in **Table 2** (refer to **Appendix**). The intersection Level of Service worksheets are attached in the **Appendix**.

The following intersections are anticipated to have one or more movements that begin to operate below LOS D with the additional project trips:

- **#3 – Gun Club Road (SH 30) at Jewell Avenue:** This signalized intersection was estimated to continue to operate at LOS F in both peak hours. Similar to the background scenario, several movements are expected to operate at LOS E/F in one or both peak hours due to high volumes on all approaches. The added trips increase the delays on already congested movements with the most impact on the northbound left-turn and northbound through movements in the PM peak hour. The northbound left-turn was estimated to begin to need at least two cycle lengths to clear the queue and the northbound through movement was estimated to experience an additional 10 seconds of delay that changes the letter grade from LOS E to LOS F.

The 95th percentile queue was calculated on the left-turn lanes were calculated to be up to 500 feet and the longest queue was calculated to be on the northbound through movement at approximately 1,095 feet in the AM peak hour.

Recommendations: No mitigation measures recommended since this intersection is built out and it is typical for arterial arterial intersections to have several movements experiencing delays equating to LOS E/F due to limited green time to serve several high-volume movements. When this intersection is widened, ensure the auxiliary lanes have adequate storage length to minimize impacts between adjacent turn and through lanes. Adjust the signal timing and phasing as traffic grows and patterns change to reduce delays as most appropriate. If this level of delay is experienced, then there is potential that this intersection will need to be redesigned with non-traditional or grade-separated options to reduce safety concerns and improve efficiency. The exact intersection design was not evaluated as part of this traffic study.

- **#104 – Gun Club Road at the North Access [3/4 movement]:** This project intersection was estimated to operate overall at LOS A in both peak periods. During the morning peak hour, the northbound left-turn was estimated to operate at LOS F and the 95th percentile queue was calculated to be approximately 23 feet (one vehicle). During the afternoon peak hour, the eastbound right-turn and the northbound left-turn were calculated to operate at LOS E and LOS F, respectively. The 95th percentile queues for these two movements were calculated to be 20 feet (one vehicle).

Recommendations: No mitigation measures recommended. The queues are minimum and can be maintained within the future storage lengths. It is typical for a minor intersection to experience delay on an arterial roadway.

7.0 Queuing Analysis

A queuing analysis was performed to determine if the 95th percentile queues would be accommodated by the existing storage length, to determine the storage lengths for future auxiliary lanes, and if any of the queues would impact an upstream intersection/access. **Table 2** provides the existing and proposed storage lengths, as well as the 95th percentile queues for each existing and future scenario as calculated by Synchro (assuming each vehicle utilizes 25 feet of space). It should be noted that the 95th percentile queue length is a theoretical queue that is 1.65 standard deviations above the average queue length. In theory, the 95th percentile queue would be exceeded 5% of the time based on the average queue length, but it is also possible that a queue this long may not occur.

As shown in **Table 2**, majority of the queues are shorter than the provided storage length in all scenarios. The project trips do not significantly increase queues at the existing study intersections. The study

intersections that are at or near capacity will experience longer queues with any additional traffic. The queues between the proposed access intersections were evaluated to determine if queued vehicles would impact an upstream intersection. It was determined that the proposed intersections will not have queues that extend into or near adjacent intersections.

Recommended turn lanes storage lengths and taper lengths are also listed in **Table 2**, which are based on the volume thresholds set forth in the CDOT *State Highway Access Code* for the assumed posted speed of each study roadway. Gun Club Road (SH 30) is classified as NR-B adjacent to the Gun Club Business Park property.

8.0 Sensitivity Analysis

A sensitivity analysis was performed to address some of the CDOT concerns with truck volume conversion to passenger-car-equivalents at the access intersections on Gun Club Road. For this sensitivity analysis, the proposed accesses were evaluated to ensure the recommended traffic control and design can adequately accommodate the Gun Club Business Park trucks and the adjacent Aspen Business Park project.

The Gun Club Business Park project plans to have two (2) accesses adjacent to the property (as described in **Section 6.3**) and share the $\frac{3}{4}$ movement access at the property line with Aspen Business Industrial Park. The truck volume was multiplied by three (3) in the peak hours at each of the proposed accesses. The Gun Club Business Park project was estimated to have 13 trucks in the AM peak hour (8 entering and 5 exiting) and up to 13 trucks in the PM peak hour (7 entering and 6 exiting). Per ITE, the truck volumes are a subset of the vehicle trips and does not increase the total volume estimated for the project. The truck volumes were multiplied by three (3) and a capacity analysis was performed for Year 2027 and Year 2050 at the three (3) access intersections to understand if there are impacts due to truck traffic. Refer to **Table A2** for the summary of level of service and 95th percentile queues at the proposed accesses.

In summary, the proposed accesses will operate the same as previously evaluated and discussed. The overall LOS remains the same with minor increases in delay on the eastbound movements at all access intersections in the Year 2027 Background + Project scenario. It was calculated that the 95th percentile queues will be similar with some increase on the northbound left-turn and southbound right-turn movements. **The proposed accesses and recommended roadway improvements can accommodate the truck trips.**

9.0 Roadway Capacity

The City of Aurora defines the roadway capacity in the *Northeast Aurora Transportation Study (NEATS) Refresh* report. Roadway capacity can be defined as “the maximum traffic volume that a roadway can carry at a desired level of service”. Roadway capacity varies for different roadway classifications, surface types, number of lanes, geometric widths, area type (rural, urban), and terrain type (level, rolling, mountainous). *NEATS Refresh* provides acceptable buildout performance (LOS D-E threshold) based on classification, number of lanes and the Year 2050 daily volume. **Table 4** provides the roadway level of service within the Gun Club Business Park study area.

Table 4: Year 2050 Roadway Level of Service Summary

Roadway	NEATS Classification	Lanes per Direction	Segment	2050	
				Volume	LOS
Jewell Avenue	Major Arterial	3	E-470 to Gun Club Road	57,190	D
			East of Gun Club Road	59,030	D
Gun Club Road	Major Arterial	3	Jewell Avenue to Addison Avenue	51,540	C
			Addison Avenue to Yale Avenue	44,480	B

10.0 Conclusions

The Gun Club Business Park project plans to develop vacant land with up to 1,466,170± square feet of light/airport industrial businesses and approximately 19,500 square feet of commercial retail space. The project proposes to construct two (2) accesses on Gun Club Road (SH 30). The primary access is proposed to be full movement and signalized and aligned with the future Addison Avenue that will travel east of Gun Club Road (SH 30). A secondary access (North Access) is planned to be constructed approximately 1,265 feet north of the Addison Avenue and restricted to $\frac{3}{4}$ movement with side-street stop-controlled. There will be cross access from the Gun Club Business Park property into the Aspen Business Park to the south, which has another three (3) accesses on Gun Club Road that could be utilized by traffic to/from the Gun Club Business Park project. Each of those accesses has varying permitted movements, including a full-movement, signalized intersection at Yale Avenue and then one $\frac{3}{4}$ movement access and one right-in, right-out access. Refer to **Section 6.3** for additional discussion on the proposed access. Internally, local

streets will be built to provide the most beneficial access into and around the site. For the purpose of this traffic study, it was assumed that the Gun Club Business Park project will be completed by Year 2027.

The Gun Club Business Park is estimated to generate approximately 5,004 daily trips with about 507 trips occurring in the AM peak hour and 521 trips occurring in the PM peak hour at full build-out. **It was determined that the proposed roadway system can adequately accommodate the projected traffic volumes for buildout conditions.** It is acknowledged that the adjacent study intersections will continue to be congested and delays will be increased with any additional traffic in the peak periods. The recommendations listed on the following pages should be considered.

Existing/Background Conditions (Non-Project Related):

- **Yale Avenue** – Construct east of Gun Club Road with two lanes per direction. *[Short-term Background]*
- **Addison Avenue** – Construct east of Gun Club Road with one lane per direction. *[Short-term Background]*
- **Gun Club Road** – Widen to two lanes per direction. *[Short-term Background]*. Widen to three lanes per direction. *[Long-term Background]*
- **Jewell Avenue** – Widen to three lanes per direction. *[Long-term Background]*
- **Jewell Avenue at E-470 Southbound Ramps:** Signalize. *[Existing]* Provide two westbound left-turn lanes and continue to provide one eastbound right-turn lane. Channelize the southbound right-turn. *[Long-term Background]*
- **Jewell Avenue at E-470 Northbound Ramps:** Signalize. *[Existing]* Provide two eastbound left-turn lanes and continue to provide one westbound right-turn lane. Channelize the northbound right-turn. *[Long-term Background]*
- **Gun Club Road (SH 30) at Jewell Avenue:** During both peak periods, provide an overlap phase for the eastbound right-turn movement and increase green time for the through movements on Jewell Avenue by 10 seconds by reducing the through phase on Gun Club Road. *[Existing]* Provide two left-turn lanes, three through lanes, and one right-turn lane on all four approaches. Consider constructing the eastbound right-turn and northbound right-turn as channelized. *[Long-term Background]*

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- **Gun Club Road (SH 30) at Yale Avenue** – Signalize. Per *Murphy Creek East Traffic Study*, construct the two westbound left-turn lanes, one westbound right-turn lane, one northbound right-turn lane, and one southbound left-turn lane. *[Short-term Background]* Provide a second southbound left-turn. *[Long-term Background]*
 - **All signalized intersections:** Adjust signal timing as appropriate for increases in volume. Balance the green time to serve all the movements and pedestrian crossings.

Project Conditions:

- **North Access (3/4 movement) on Gun Club Road (SH 30):** Construct with one inbound and outbound lane. Construct one northbound left-turn lane and one southbound right-turn lane.
- **Gun Club Road (SH 30) at Addison Avenue:** Align with the future Addison Avenue and signalize. With the Gun Club Business Park, construct the eastbound approach with one eastbound left-turn lane, one eastbound through/right-turn lane. Construct one northbound left-turn lane and one southbound right-turn lane.

The proposed lengths of auxiliary lanes are listed in **Table 2**. Note that the traffic study provides technical information and evaluates the need for transportation mitigation as traffic grows, but it does not address infrastructure commitments or obligations of the Gun Club Business Park development.

Tables and Figures:

Table 1 – Peak Hour Intersection LOS Summary for Existing Intersections

Table 2 – Peak Hour Estimated Queues and Proposed Auxiliary Lanes

Table 3 –Trip Generation Summary

Table 4 – Year 2050 Roadway Level of Service Summary [IN REPORT]

Figure 1 – Vicinity Map

Figure 2 – Site Plan and Access Locations

Figure 3 – Existing Traffic Volumes

Figure 4 – Year 2027 Background Traffic Volumes

Figure 5 – Year 2050 Background Traffic Volumes

Figure 6A – Trip Distribution – Industrial

Figure 6B – Trip Distribution – Retail

Figure 6C – Trip Distribution – Retail Pass-By

Figure 7 – Site-Generated Traffic Volumes

Figure 8 – Year 2027 Background + Project Traffic Volumes

Figure 9 – Year 2050 Background + Project Traffic Volumes

Table 1 - Peak Hour Intersection Level of Service Summary

Intersection and Lanes Groups	2023 Existing		2023 Existing with Improvements		2027 Background		2027 Bkgrd + Project		2050 Background		2050 Bkgrd + Project		
	AM Peak Delay	PM Peak LOS	AM Peak Delay	PM Peak LOS	AM Peak Delay	PM Peak LOS	AM Peak Delay	PM Peak LOS	AM Peak Delay	PM Peak LOS	AM Peak Delay	PM Peak LOS	
STOP SIGN CONTROL													
1. Jewell Ave at E-470 Southbound Ramps	6 A	5 A											
Eastbound Through	0 A	0 A	Refer to Signal Section		Analyzed as Signalized		Analyzed as Signalized		Analyzed as Signalized		Analyzed as Signalized		
Eastbound Right	0 A	0 A											
Westbound Left	10 A	11 B											
Westbound Through	0 A	0 A											
Southbound Left+Through	94 F	77 F											
Southbound Right	25 C	21 C											
2. Jewell Ave at E-470 Northbound Ramps	81 F	31 D											
Eastbound Left	11 B	10 B	Refer to Signal Section		Analyzed as Signalized		Analyzed as Signalized		Analyzed as Signalized		Analyzed as Signalized		
Eastbound Through	0 A	0 A											
Westbound Through	0 A	0 A											
Westbound Right	0 A	0 A											
Northbound Left+Through	>120 F	>120 F											
Northbound Right	11 B	13 B											
101. Gun Club Rd at Aspen Business Park Access (3/4 Mvmt)					0 A	0 A	1 A	0 A	0 A	0 A	0 A	0 A	
Eastbound Right		Future Intersection		Not Applicable	13 B	15 B	13 B	16 C	12 B	15 B	12 B	16 C	
Northbound Left					12 B	13 B	13 B	13 B	12 B	12 B	11 B	13 B	
Northbound Through					0 A	0 A	0 A	0 A	0 A	0 A	0 A	0 A	
Southbound Through					0 A	0 A	0 A	0 A	0 A	0 A	0 A	0 A	
Southbound Right					0 A	0 A	0 A	0 A	0 A	0 A	0 A	0 A	
103. Gun Club Rd at Addison Avenue					0 A	3 A					0 A	0 A	
Westbound Left		Future Intersection		Not Applicable	19 C	>120 F		Analyzed as Signalized		Not Applicable	12 B	13 B	
Westbound Right					11 B	16 C					11 B	11 B	
Northbound Through					0 A	0 A					0 A	0 A	
Northbound Right					0 A	0 A					0 A	0 A	
Southbound Left					9 A	14 B					0 A	0 A	
Southbound Through					0 A	0 A					0 A	0 A	
104. Gun Club Rd at North Access [3/4 Mvmt]								0 A	0 A		0 A	0 A	
Eastbound Right		Project Intersection		Not Applicable			Project Intersection	15 C	13 B		Project Intersection	25 D	36 E
Northbound Left					14 B	10 A					55 F	70 F	
Northbound Through					0 A	0 A					0 A	0 A	
Southbound Through					0 A	0 A					0 A	0 A	
Southbound Right					0 A	0 A					0 A	0 A	
SIGNAL CONTROL													
1. Jewell Ave at E-470 Southbound Ramps			12 B	17 B	15 B	22 C	18 B	24 C	10 B	17 B	12 B	21 C	
Eastbound Through	Analyzed as Stop-Control		18 B	21 C	23 C	33 C	26 C	36 D	20 B	23 C	20 B	28 C	
Eastbound Right		Signalize	13 B	14 B	13 B	16 B	14 B	16 B	16 B	12 B	16 B	13 B	
Westbound Left			11 B	13 B	14 B	19 B	15 B	29 C	16 B	36 D	17 B	52 D	
Westbound Through			1 A	1 A	1 A	0 A	1 A	0 A	0 A	0 A	0 A	0 A	
Southbound Left+Through			28 C	28 C	32 C	29 C	38 D	30 C	38 D	54 D	52 D	54 D	
Southbound Right			38 D	46 D	42 D	54 D	42 D	54 D	0 A	0 A	0 A	0 A	

Gun Club Business Park Traffic Impact Study

Table 1 - Peak Hour Intersection Level of Service Summary

Intersection and Lanes Groups	2023 Existing				2023 Existing with Improvements				2027 Background				2027 Bkgrd + Project				2050 Background				2050 Bkgrd + Project			
	AM Peak Delay		PM Peak LOS		AM Peak Delay		PM Peak LOS		AM Peak Delay		PM Peak LOS		AM Peak Delay		PM Peak LOS		AM Peak Delay		PM Peak LOS		AM Peak Delay		PM Peak LOS	
	SIGNAL CONTROL																							
2. Jewell Ave at E-470 Northbound Ramps					19	B	14	B	24	C	22	C	28	C	29	C	26	C	14	B	28	C	15	B
Eastbound Left	Analyzed as Stop-Control		18 B		13	B	32	C	26	C	40	D	48	D	30	C	24	C	30	C	25	C		
Eastbound Through			1 A		1	A	1	A	1	A	1	A	0	A	1	A	1	A	1	A	1	A		
Westbound Through			24 C		21	C	34	C	35	C	44	D	50	D	40	D	28	C	45	D	29	C		
Westbound Right			12 B		12	B	13	B	14	B	13	B	14	B	16	B	21	C	16	B	22	C		
Northbound Left+Through			34 C		31	C	37	D	32	C	37	D	34	C	53	D	34	C	53	D	34	C		
Northbound Right			28 C		29	C	34	C	30	C	39	D	34	C	0	A	0	A	0	A	0	A		
3. Gun Club Rd at Jewell Ave	73	E	45	D	38	D	36	D	40	D	40	D	44	D	47	D	102	F	116	F	104	F	128	F
Eastbound Left	45	D	43	D	47	D	45	D	74	E	63	E	73	E	62	E	>150	F	91	F	>150	F	90	F
Eastbound Through	>120	F	81	F	52	D	60	E	48	D	58	E	45	D	57	E	46	D	110	F	48	D	110	F
Eastbound Right	52	D	47	D	32	C	38	D	35	D	29	C	50	D	28	C	0	A	0	A	0	A		
Westbound Left	63	E	86	F	38	D	49	D	59	E	68	E	62	E	70	E	78	E	>150	F	79	E	>150	F
Westbound Through	>120	F	123	F	66	E	55	D	57	E	55	D	46	D	53	D	114	F	97	F	114	F	97	F
Westbound Right	46	D	43	D	37	D	38	D	34	C	36	D	31	C	36	D	33	C	38	D	33	C	38	D
Northbound Left	16	B	24	C	24	C	30	C	63	E	63	E	67	E	104	F	86	F	85	F	108	F	>150	F
Northbound Through	23	C	20	C	34	C	23	C	24	C	20	B	28	C	21	C	94	F	73	E	97	F	83	F
Northbound Right	12	B	16	B	16	B	18	B	19	B	21	C	22	C	23	C	0	A	0	A	0	A	0	A
Southbound Left	18	B	16	B	25	C	18	B	24	C	22	C	28	C	25	C	>150	F	>150	F	>150	F	>150	F
Southbound Through	23	C	29	C	32	C	34	C	31	C	34	C	38	D	39	D	62	E	84	F	67	E	103	F
Southbound Right	17	B	17	B	24	C	19	B	29	C	28	C	34	C	31	C	53	D	29	C	53	D	30	C
102. Gun Club Rd at Yale Ave									26	C	23	C	12	B	27	C	30	C	38	D	33	C	36	D
Eastbound Left	Future Intersection		Not Applicable		51 D		50 D		51 D		50 D		35 C		32 C		35 C		32 C		35 C		32 C	
Eastbound Through+Right			54 D		37 D		54 D		37 D		37 D		33 C		37 D		33 C		38 D		33 C		36 D	
Westbound Left			51 D		35 C		51 D		35 C		38 D		33 C		38 D		36 D		36 D		36 D		36 D	
Westbound Through+Right			53 D		38 D		53 D		38 D		36 D		36 D		36 D		36 D		36 D		36 D		36 D	
Northbound Left			7 A		14 B		8 A		16 B		18 B		29 C		19 B		19 B		19 B		30 C		30 C	
Northbound Through			11 B		19 B		11 B		20 B		34 C		41 D		40 D		20 B		40 D		43 D		43 D	
Northbound Right			7 A		13 B		7 A		13 B		19 B		40 D		40 D		20 B		40 D		40 D		40 D	
Southbound Left			7 A		14 B		8 A		15 B		39 D		46 D		43 D		43 D		46 D		46 D		46 D	
Southbound Through			10 A		19 B		10 B		27 C		22 C		35 C		35 C		22 C		30 C		30 C		30 C	
Southbound Right			8 A		11 B		8 A		15 B		17 B		20 B		18 B		18 B		15 B		15 B		15 B	
103. Gun Club Rd at Addison Avenue									14	B	17	B	5	A	7	A	15	B	23	C				
Eastbound Left	Future Intersection		Analyzed as Stop-Control		38 D		47 D										49 D		51 D		52 D		46 D	
Eastbound Through+Right			41 D		36 D		41 D		34 C		53 D		51 D		47 D		49 D		49 D		49 D		49 D	
Westbound Left			36 D		34 C		41 D		39 D		0 A		51 D		50 D		50 D		52 D		52 D		52 D	
Westbound Through+Right			11 B		15 B		12 B		5 A		7 A		9 A		15 B		15 B		22 C		22 C		22 C	
Northbound Left			8 A		0 A		9 A		4 A		4 A		5 A		8 A		8 A		9 A		9 A		9 A	
Northbound Through			14 B		23 C		2 A		2 A		4 A		13 B		20 C		20 C		31 C		31 C		31 C	
Northbound Right			10 B		12 B		4 A		4 A		4 A		12 B		12 B		12 B		25 C		25 C		25 C	
Southbound Left			9 A		10 B		9 A		9 A		13 B		12 B		12 B		12 B		13 B		13 B		13 B	
Southbound Through			14 B		23 C		2 A		2 A		4 A		4 A		4 A		4 A		9 A		9 A		9 A	
Southbound Right			10 B		12 B		4 A		4 A		4 A		4 A		4 A		4 A		9 A		9 A		9 A	

Table 2 - Peak Hour 95th Percentile Queue Summary and Proposed Auxiliary Lanes

Intersection and Lanes Groups	Existing Storage Length (Feet)	2023 Existing		2023 Existing with Improvement		2027 Background		2027 Bkgrd + Project		2050 Background		2050 Bkgrd + Project		Max. Queue	City and CDOT SHAC Requirement (NR-B)			Proposed Minimum Future Storage	
		AM	PM	Imprv.	AM	PM	AM	PM	AM	PM	AM	PM	AM	PM	Speed (mph)	Total (feet)	Storage (feet)	Taper (feet)	
1. Jewell Ave at E-470 Southbound Ramps		Side-Street Stop		Signalized		Signalized		Signalized		Signalized		Signalized		Signalized					
Eastbound Through	-	0' 0'			336' 472'		466' 667'		565' 687'		327' 826'		339' 909'		-	-	-	-	-
Eastbound Right	300'	0' 0'		31' 40'	33' 41'		40' 49'		40' 72'		72'		45		435	273'	162	273'	
Westbound Left	250'	13' 13'		25' 38'	26' 92'		45' 128'		75' 117'		79'		139'		45	435	273'	162	273'
Westbound Through	-	0' 0'		143' 236'	141' 226'		136' 211'		428' 161'		426'		164'		-	-	-	-	-
Southbound Left+Through	-	60' 50'		47' 44'	139' 64'		229' 88'		232' 250'		354'		256'		-	-	-	-	-
Southbound Right	-	73' 80'		48' 51'	81' 90'		83' 103'		148' 380'		148'		303'		35	310	190'	120	305'
2. Jewell Ave at E-470 Northbound Ramps		Side-Street Stop		Signalized		Signalized		Signalized		Signalized		Signalized		Signalized					
Eastbound Left	250'	25' 20'		175' 47'	184' 106'		156' 113'		91' 69'		77' 67'		77'		45	435	273'	162	273'
Eastbound Through	-	0' 0'		109' 48'	250' 59'		312' 77'		41' 177'		54'		176'		-	-	-	-	-
Westbound Through	-	0' 0'		471' 417'	683' 724'		719' 811'		7020' 450'		714'		549'		-	-	-	-	-
Westbound Right	250'	0' 0'		3' 15'	13' 35'		22' 40'		57' 10'		60'		80'		45	435	273'	162	273'
Northbound Left+Through	-	445' 263'		160' 102'	181' 112'		181' 115'		350' 161'		350'		246'		-	-	-	-	-
Northbound Right	175'	8' 13'		0' 9'	42' 25'		50' 32'		149' 179'		203'		314'		35	310	190'	120	315'
3. Gun Club Rd at Jewell Ave		Signalized		Signalized		Signalized		Signalized		Signalized		Signalized		Signalized					
Eastbound Left	220'	144' 163'		155' 151'	122' 117'		122' 117'		389' 389'		389'		389'		45	435	273'	162	390'
Eastbound Through	-	374' 420'		255' 326'	273' 364'		275' 364'		438' 1031'		438'		1031'		-	-	-	-	-
Eastbound Right	330'	16' 58'		11' 60'	194' 125'		404' 174'		0' 0'		0'		0'		45	435	273'	162	273'
Westbound Left	300'	267' 273'		789' 159'	173' 157'		194' 173'		300' 488'		327'		505'		45	435	273'	162	510'
Westbound Through	-	637' 520'		412' 327'	402' 365'		402' 365'		1095' 792'		1095'		792'		-	-	-	-	-
Westbound Right	-	0' 0'		0' 0'	0' 0'		0' 0'		396' 404'		396'		404'		45	435	273'	162	400'
Northbound Left	485'	88' 98'		118' 243'	148' 211'		169' 374'		298' 203'		352'		402'		55	600	378'	222	400'
Northbound Through	-	369' 445'		463' 532'	336' 273'		273' 300'		732' 590'		745'		632'		-	-	-	-	-
Northbound Right	360'	8' 42'		0' 50'	47' 55'		7' 57'		0' 0'		0'		0'		55	600	378'	222	378'
Southbound Left	200'	22' 13'		29' 17'	38' 17'		38' 17'		277' 451'		277'		451'		55	600	378'	222	450'
Southbound Through	-	376' 585'		451' 756'	287' 453'		323' 485'		420' 721'		474'		736'		-	-	-	-	-
Southbound Right	325'	36' 35'		43' 42'	47' 54'		47' 54'		244' 189'		244'		244'		55	600	378'	222	378'
101. Gun Club Rd at Aspen Business Park Access (3/4 Mvmt)				Side-Street Stop		Side-Street Stop		Side-Street Stop		Side-Street Stop		Side-Street Stop							
Eastbound Right		Future Intersection		Not Applicable	0' 5'		3' 10'		0' 5'		3' 10'		10'		30	250	154'	96	154'
Northbound Left					8' 0'		13' 3'		8' 0'		10' 3'		10'		55	600	378'	222	378'
Northbound Through					0' 0'		0' 0'		0' 0'		0' 0'		-		-	-	-	-	-
Southbound Through					0' 0'		0' 0'		0' 0'		0' 0'		-		-	-	-	-	-
Southbound Right					0' 0'		0' 0'		0' 0'		0' 0'		0'		55	600	378'	222	378'
102. Gun Club Rd at Yale Ave				Signalized		Signalized		Signalized		Signalized		Signalized							
Eastbound Left		Future Intersection		Not Applicable	70' 318'		70' 318'		61' 333'		61' 333'		333'		35	310	190'	120	330'
Eastbound Through+Right					27' 42'		28' 43'		22' 42'		23' 43'		-		-	-	-	-	-
Westbound Left					77' 88'		77' 88'		155' 182'		155' 182'		182'		35	310	190'	120	190'
Westbound Through+Right					17' 13'		17' 13'		89' 246'		90'		246'		35	310	190'	120	250'
Northbound Left					45' 11'		48' 12'		62' 18'		70'		19'		55	600	378'	222	378'
Northbound Through					318' 351'		370' 370'		694' 438'		761'		455'		-	-	-	-	-
Northbound Right					0' 0'		0' 0'		52' 81'		52'		87'		55	600	378'	222	378'
Southbound Left					33' 48'		36' 25'		123' 244'		123'		248'		55	600	378'	222	378'
Southbound Through					377' 393'		280' 510'		327' 561'		336'		635'		-	-	-	-	-
Southbound Right					59' 0'		29' 0'		35' 0'		35'		0'		55	600	378'	222	378'

Table 2 - Peak Hour 95th Percentile Queue Summary and Proposed Auxiliary Lanes

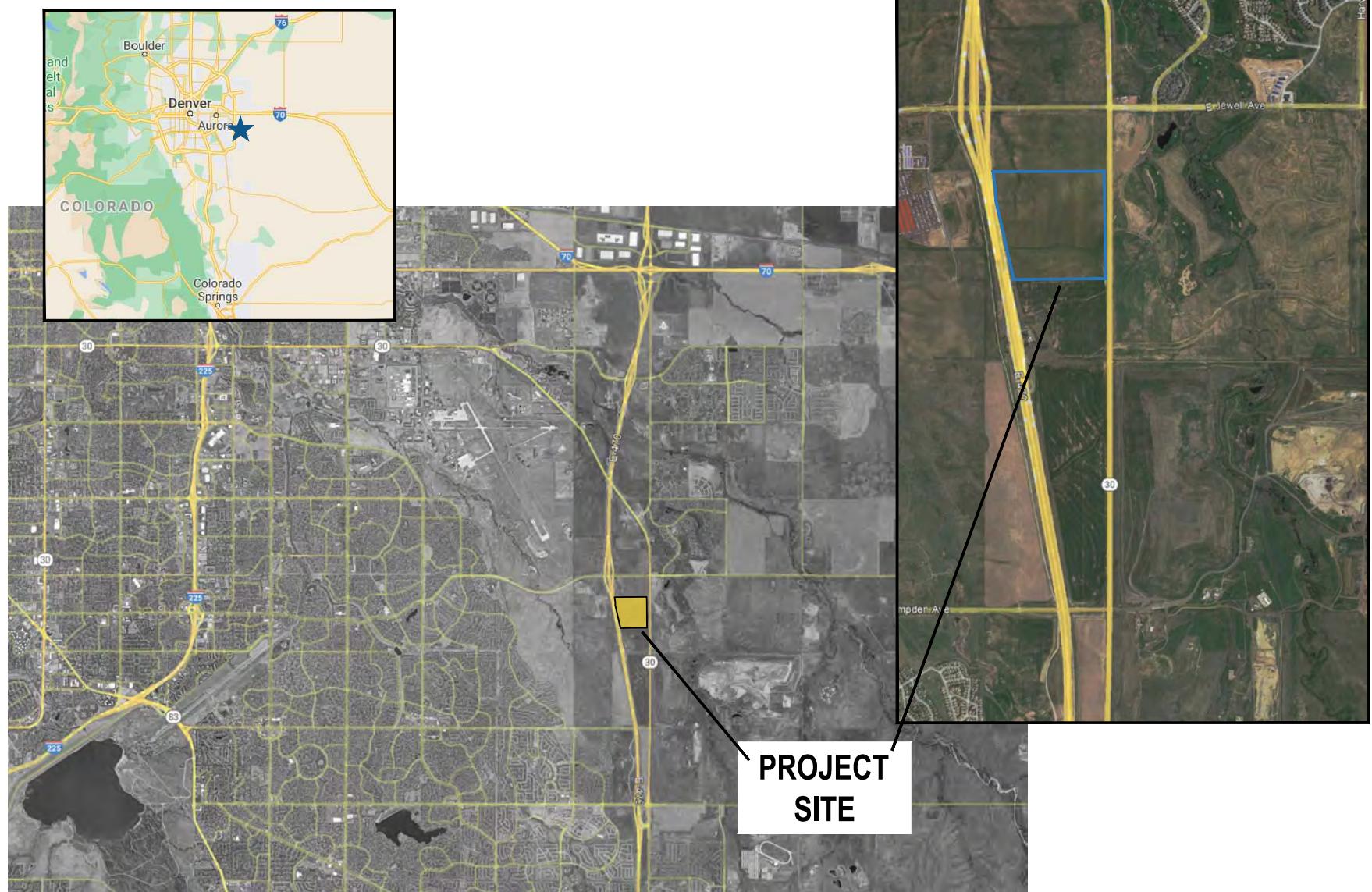
Intersection and Lanes Groups	Existing Storage Length (Feet)	2023 Existing		2023 Existing with Improvement		2027 Background		2027 Bkgrd + Project		2050 Background		2050 Bkgrd + Project		Max. Queue	City and CDOT SHAC Requirement (NR-B)			Proposed Minimum Future Storage	
		AM	PM	Imprv.	AM	PM	AM	PM	AM	PM	AM	PM	AM	PM	Speed (mph)	Total (feet)	Storage (feet)	Taper (feet)	
103. Gun Club Rd at Addison Avenue																			
Eastbound Left																			
Eastbound Through+Right																			
Westbound Left																			
Westbound Right																			
Westbound Through+Right																			
Northbound Left																			
Northbound Through																			
Northbound Right																			
Southbound Left																			
Southbound Through																			
Southbound Right																			
104. Gun Club Rd at North Access [3/4 Mvmt]																			
Eastbound Right																			
Northbound Left																			
Northbound Through																			
Southbound Through																			
Southbound Right																			

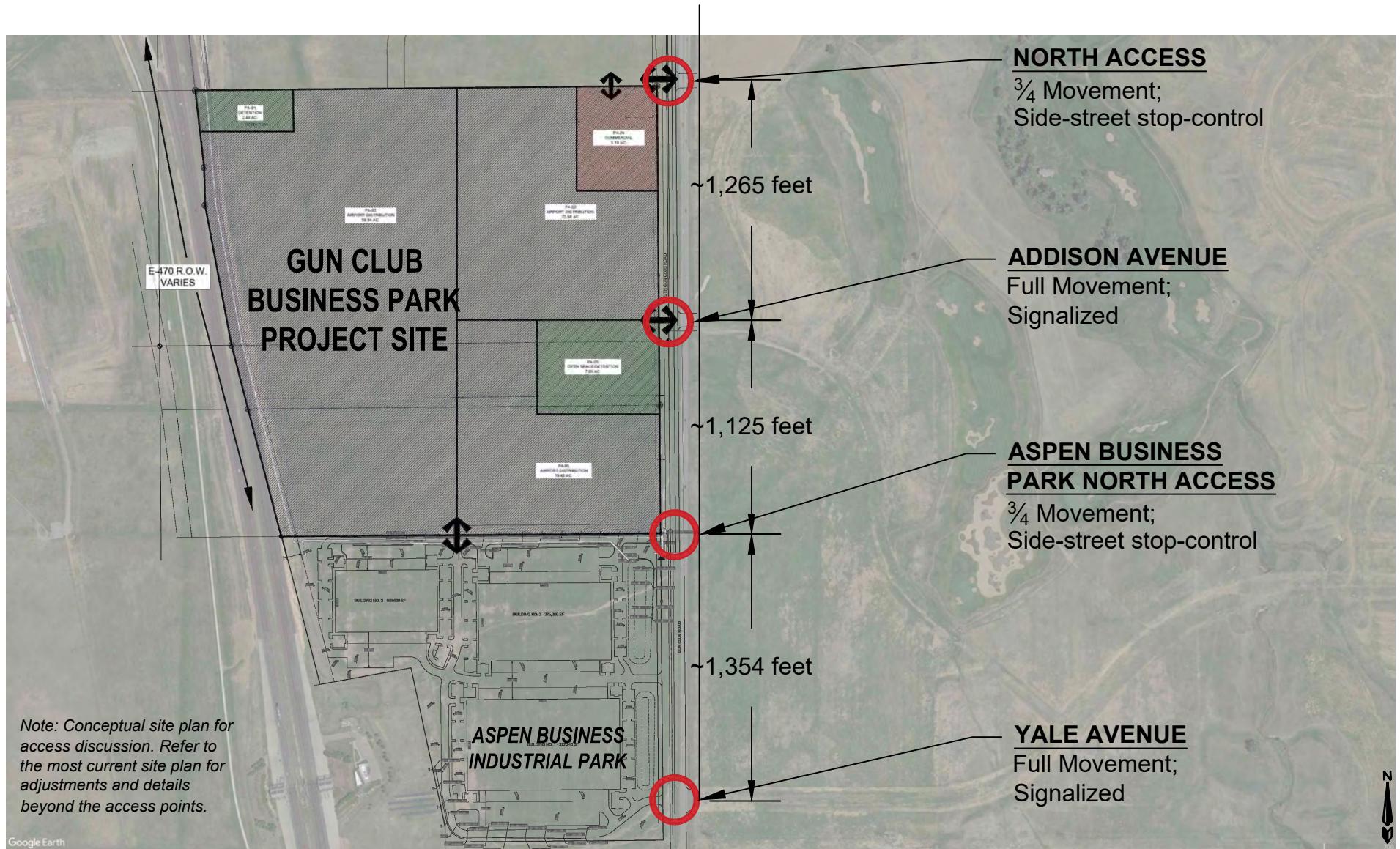
Table 3 - Trip Generation Summary

Land Use	Size	Unit	Internal Capture		Non-Auto Factor	Average Daily Trips				AM Peak Hour Trips				PM Peak Hour Trips			
			AM	PM		Rate	Total	In	Out	Rate	Total	In	Out	Rate	Total	In	Out
ITE#130: Industrial Park	1,466.2	ksf	0.99	0.92	0.95	3.37	4,318	2,159	2,159	0.34	469	380	89	0.34	436	96	340
ITE#822: Strip Retail Plaza (<40k)	13.0	ksf	0.99	0.92	0.95	54.45	619	310	309	2.36	29	17	12	6.59	75	38	37
ITE#930: Fast Casual Restaurant	6.5	ksf	0.99	0.92	0.95	97.14	552	276	276	1.43	9	5	4	12.55	71	39	32
<i>Subtotal</i>						5,489	2,745	2,744		507	402	105		582	173	409	
<i>Pass-by Trips: Retail (PM)</i>						40%	248	124	124		0	0	0		30	15	15
<i>Pass-by Trips: Restaurant (PM)</i>						43%	237	119	118		0	0	0		31	17	14
Total New Vehicle Trips							5,004	2,502	2,502		507	402	105		521	141	380

Source : ITE Trip Generation 11th Edition, 2021.

Industrial Trucks (ITE)	0.25	320	160	160	0.01	13	8	5	0.01	13	7	6
Passenger Cars (Total Vehicles minus Trucks)	7%	4,684	2,342	2,342	3%	494	394	100	3%	508	134	374





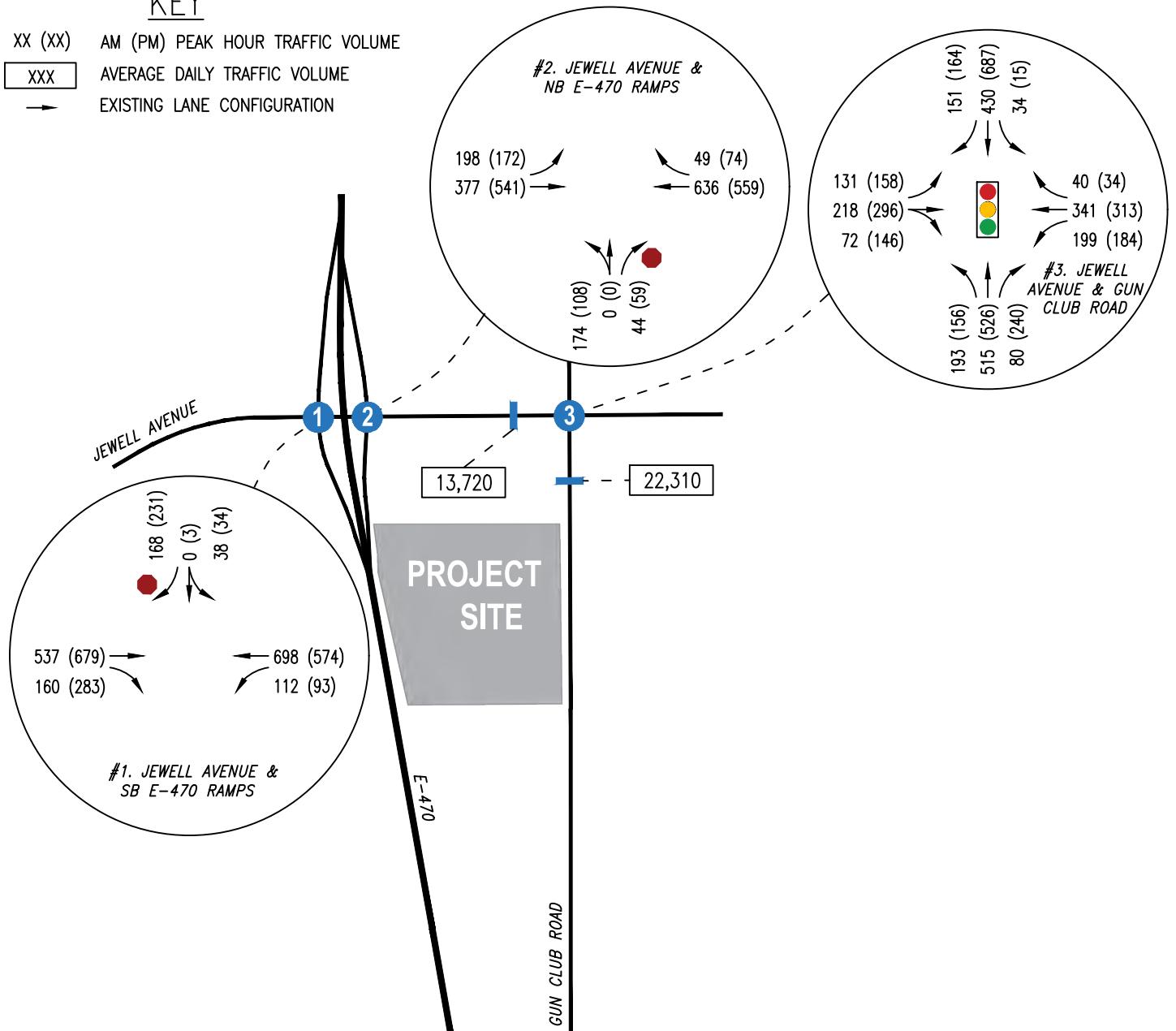
FOX TUTTLE
TRANSPORTATION GROUP

GUN CLUB BUSINESS PARK TRAFFIC IMPACT STUDY - AURORA, CO
SITE PLAN AND ACCESS LOCATIONS

FT Project #	23087	Original Scale	NTS	Date	2/22/2024	Drawn by	CRS	Figure #	2
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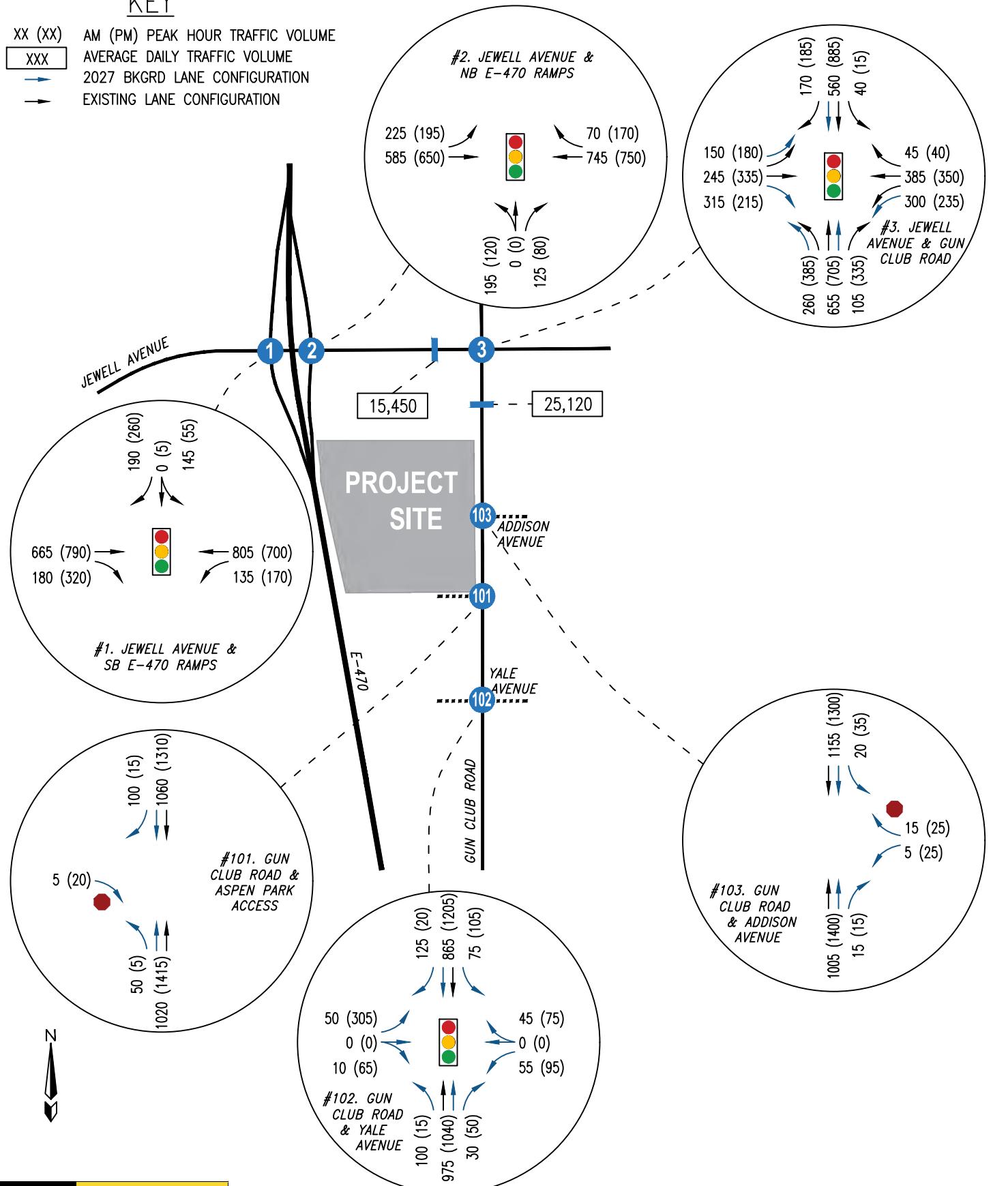
KEY

- XX (XX) AM (PM) PEAK HOUR TRAFFIC VOLUME
- XXX AVERAGE DAILY TRAFFIC VOLUME
- EXISTING LANE CONFIGURATION



KEY

- XX (XX) AM (PM) PEAK HOUR TRAFFIC VOLUME
- XXX AVERAGE DAILY TRAFFIC VOLUME
- 2027 BKGRD LANE CONFIGURATION
- EXISTING LANE CONFIGURATION



FOX TUTTLE
TRANSPORTATION GROUP

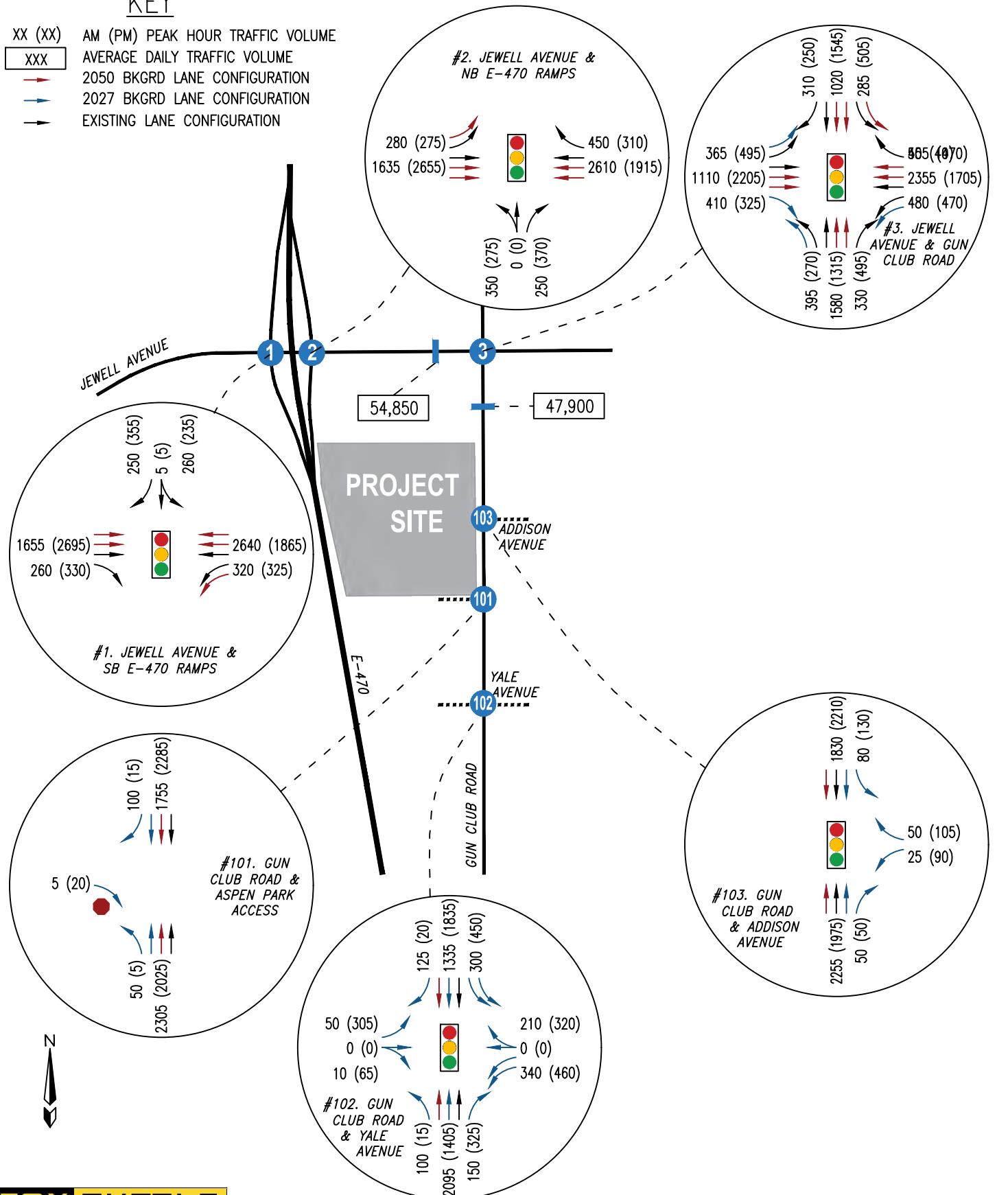
GUN CLUB BUSINESS PARK TRAFFIC IMPACT STUDY - AURORA, CO

YEAR 2027 BACKGROUND TRAFFIC VOLUMES

FT #	23087	Original Scale	NTS	Date	2/22/2024	Drawn by	CRS	Figure #	4
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KEY

- XX (XX) AM (PM) PEAK HOUR TRAFFIC VOLUME
- XXX AVERAGE DAILY TRAFFIC VOLUME
- 2050 BKGRD LANE CONFIGURATION
- ← 2027 BKGRD LANE CONFIGURATION
- ↔ EXISTING LANE CONFIGURATION



KEY

XX% [XX] ENTERING [EXITING] TRIP DISTRIBUTION
→ TRIP DIRECTION

20%
TO/FROM THE
NORTH VIA E-470

10%
TO/FROM THE
WEST VIA
JEWELL AVENUE

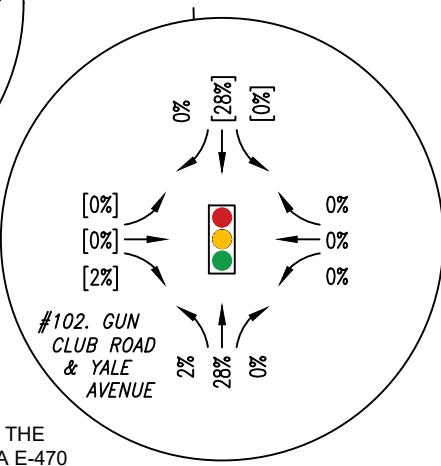
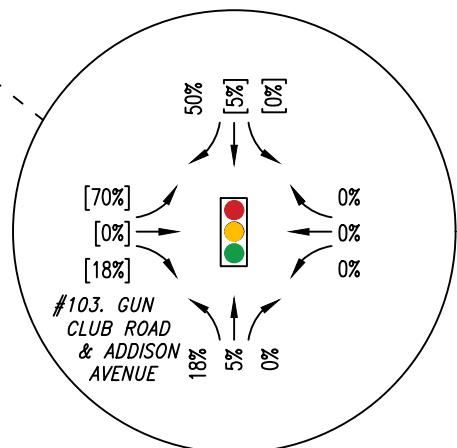
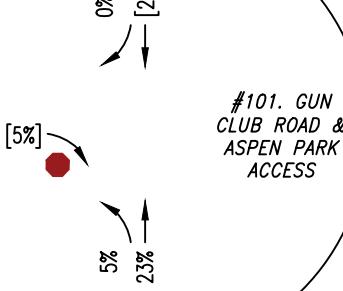
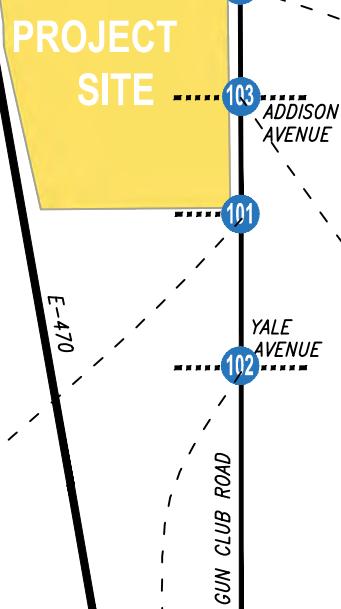
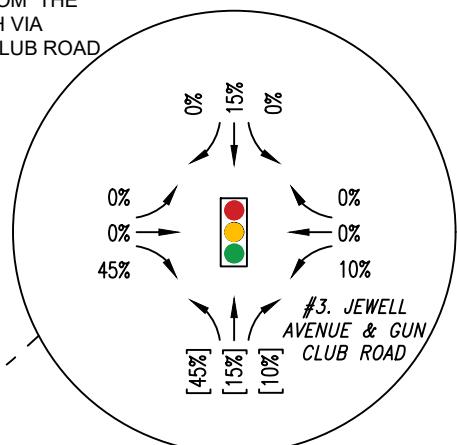
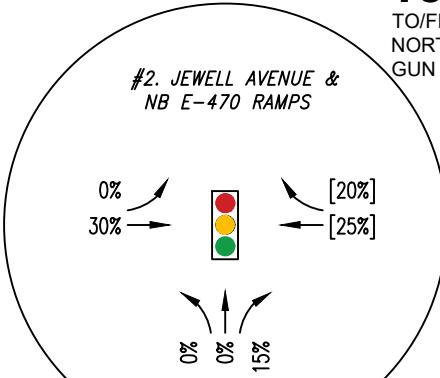
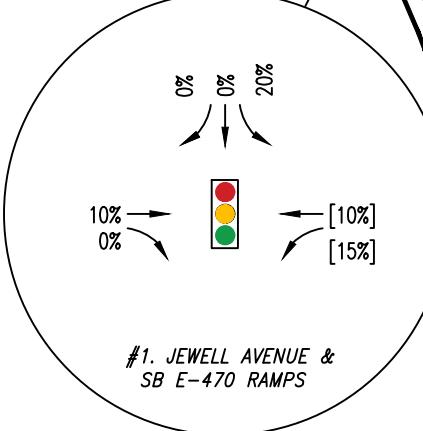
15%
TO/FROM THE
SOUTH VIA E-470

15%

TO/FROM THE
NORTH VIA
GUN CLUB ROAD

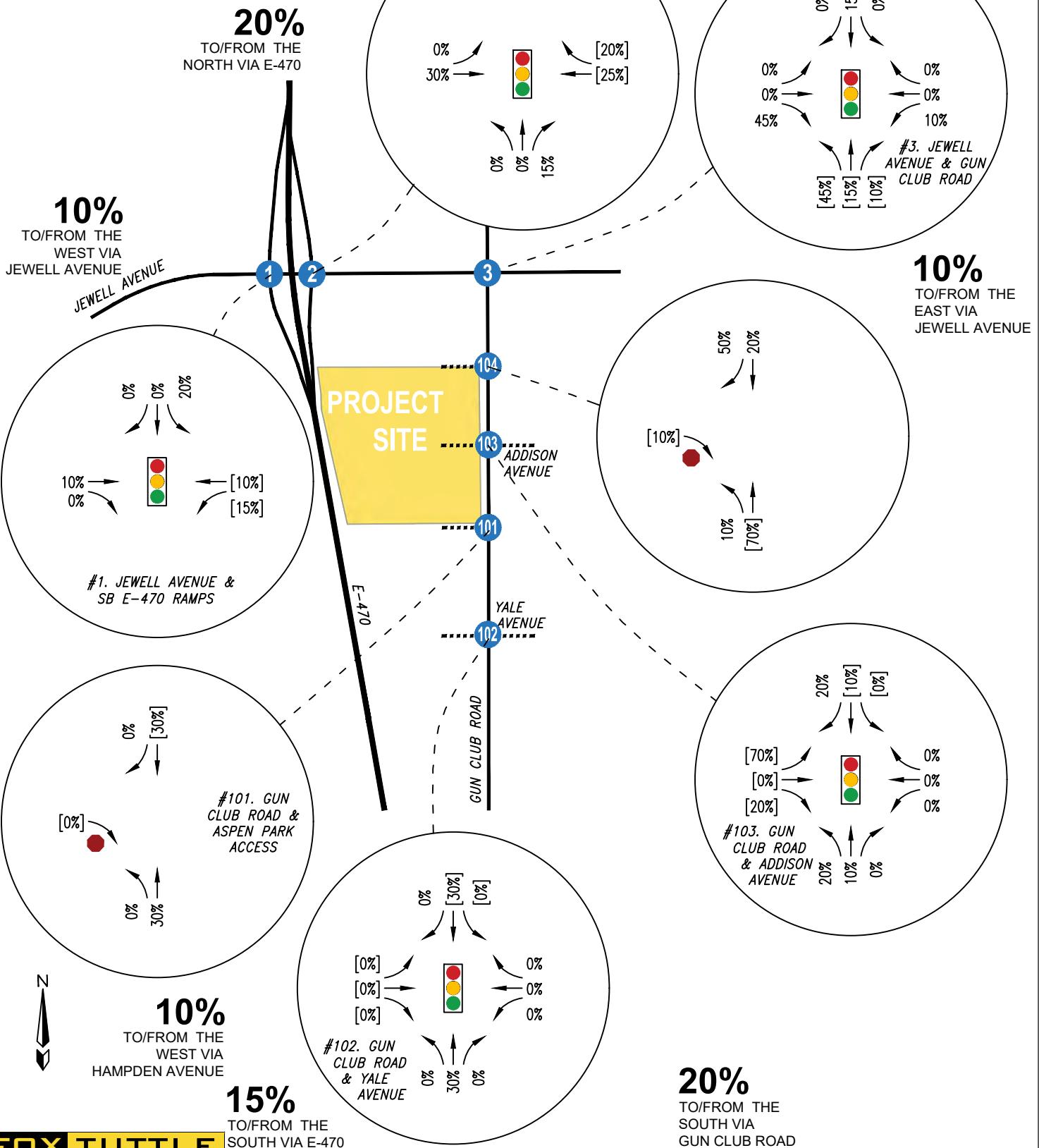
10%
TO/FROM THE
EAST VIA
JEWELL AVENUE

20%
TO/FROM THE
SOUTH VIA
GUN CLUB ROAD



KEY

XX% [XX] ENTERING [EXITING] TRIP DISTRIBUTION
→ TRIP DIRECTION

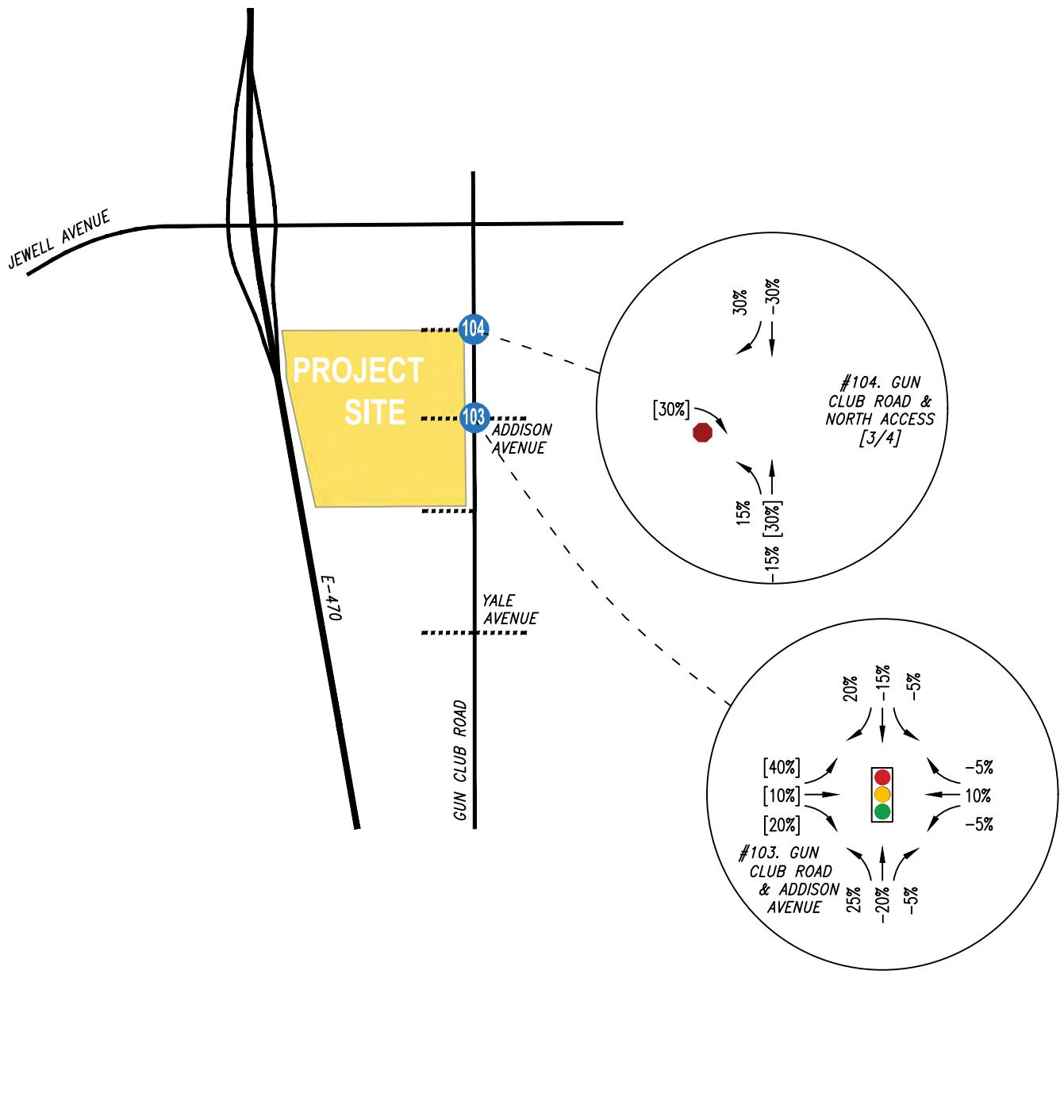


GUN CLUB BUSINESS PARK TRAFFIC IMPACT STUDY - AURORA, CO
TRIP DISTRIBUTION - RETAIL

FT # 23087 Original Scale NTS Date 2/22/2024 Drawn by CRS Figure # 6B

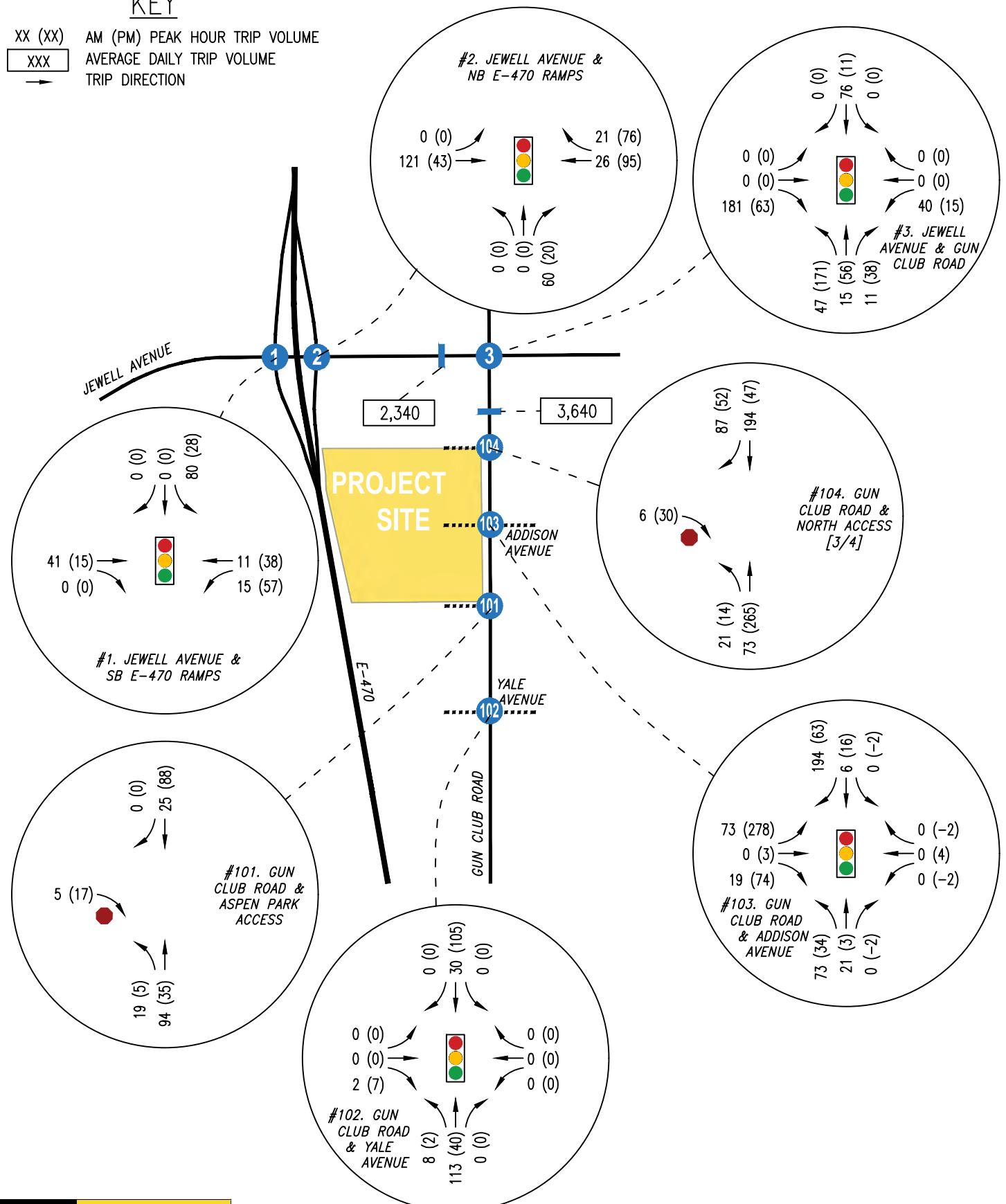
KEY

XX% [XX] ENTERING [EXITING] TRIP DISTRIBUTION
→ TRIP DIRECTION



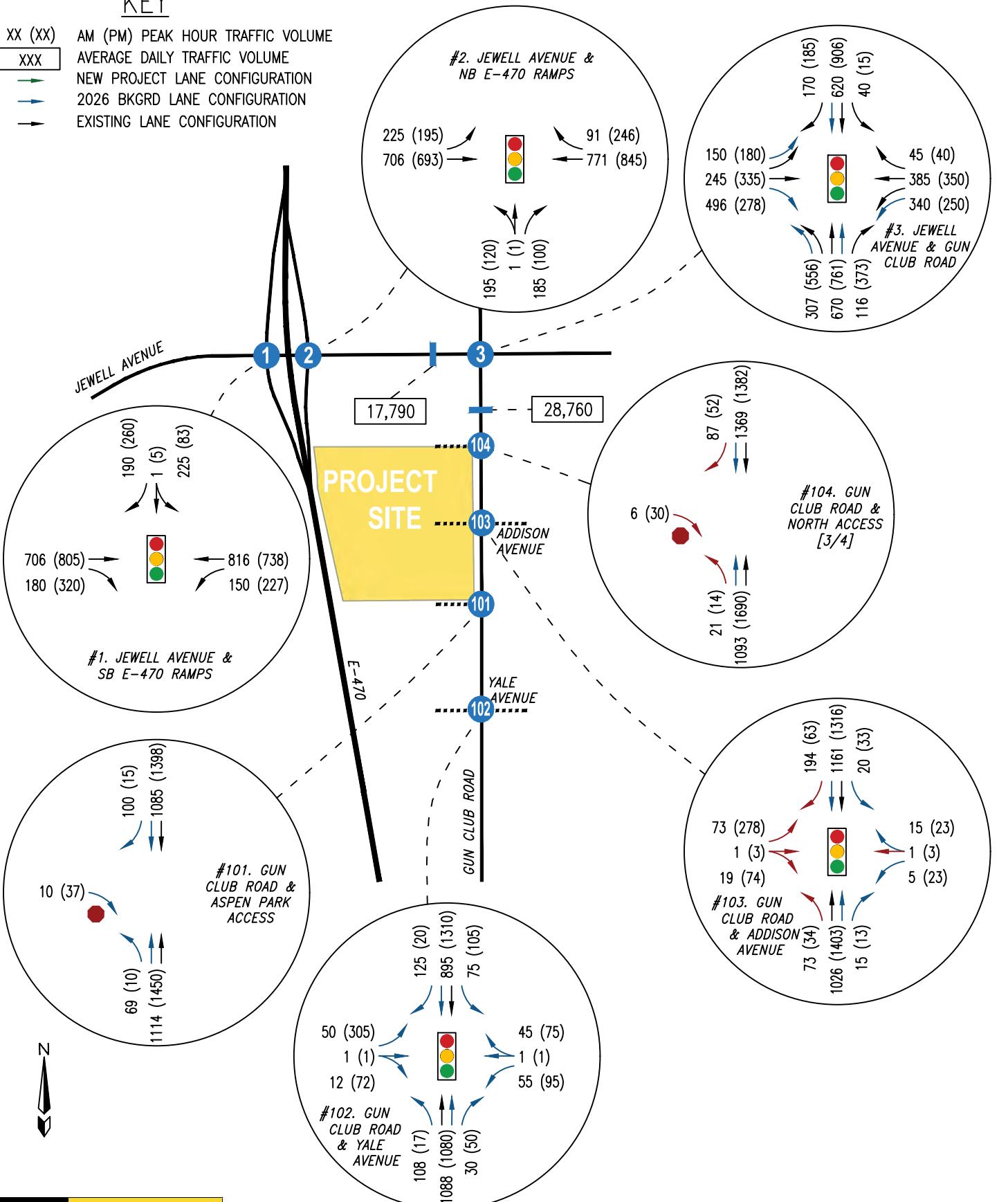
KEY

XX (XX) AM (PM) PEAK HOUR TRIP VOLUME
 XXX AVERAGE DAILY TRIP VOLUME
 → TRIP DIRECTION



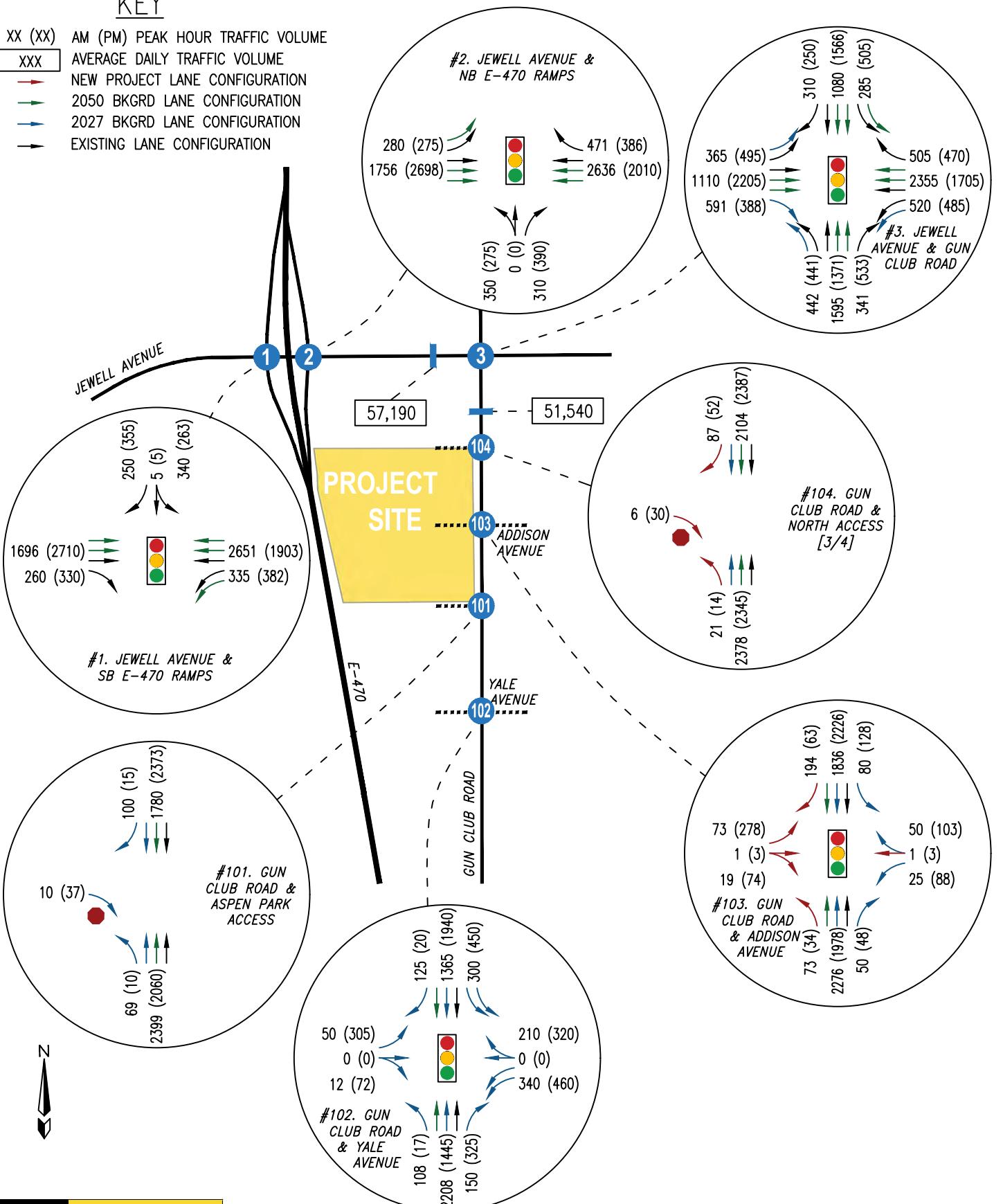
KEY

- XX (XX) AM (PM) PEAK HOUR TRAFFIC VOLUME
- XXX AVERAGE DAILY TRAFFIC VOLUME
- NEW PROJECT LANE CONFIGURATION
- 2026 BKGRD LANE CONFIGURATION
- EXISTING LANE CONFIGURATION



KEY

- XX (XX) AM (PM) PEAK HOUR TRAFFIC VOLUME
- XXX AVERAGE DAILY TRAFFIC VOLUME
- NEW PROJECT LANE CONFIGURATION
- 2050 BKGRD LANE CONFIGURATION
- 2027 BKGRD LANE CONFIGURATION
- EXISTING LANE CONFIGURATION



Appendix:

Level of Service Definitions

Existing Traffic Data

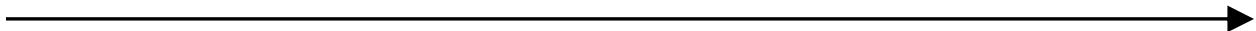
Background Documentation

Intersection Capacity Worksheets

Signal Warrant Worksheets

Internal Capture and Pass-By Calculations

Sensitivity Analysis



Level of Service Definitions

LEVEL OF SERVICE DEFINITIONS

In rating roadway and intersection operating conditions with existing or future traffic volumes, "Levels of Service" (LOS) A through F are used, with LOS A indicating very good operation and LOS F indicating poor operation. Levels of service at signalized and unsignalized intersections are closely associated with vehicle delays experienced in seconds per vehicle. More complete level of service definitions and delay data for signal and stop sign controlled intersections are contained in the following table for reference.

Level of Service Rating	Delay in seconds per vehicle (a)		Definition
	Signalized	Unsignalized	
A	0.0 to 10.0	0.0 to 10.0	Low vehicular traffic volumes; primarily free flow operations. Density is low and vehicles can freely maneuver within the traffic stream. Drivers are able to maintain their desired speeds with little or no delay.
B	10.1 to 20.0	10.1 to 15.0	Stable vehicular traffic volume flow with potential for some restriction of operating speeds due to traffic conditions. Vehicle maneuvering is only slightly restricted. The stopped delays are not bothersome and drivers are not subject to appreciable tension.
C	20.1 to 35.0	15.1 to 25.0	Stable traffic operations, however the ability for vehicles to maneuver is more restricted by the increase in traffic volumes. Relatively satisfactory operating speeds prevail, but adverse signal coordination or longer vehicle queues cause delays along the corridor.
D	35.1 to 55.0	25.1 to 35.0	Approaching unstable vehicular traffic flow where small increases in volume could cause substantial delays. Most drivers are restricted in ability to maneuver and selection of travel speeds due to congestion. Driver comfort and convenience are low, but tolerable.
E	55.1 to 80.0	35.1 to 50.0	Traffic operations characterized by significant approach delays and average travel speeds of one-half to one-third the free flow speed. Vehicular flow is unstable and there is potential for stoppages of brief duration. High signal density, extensive vehicle queuing, or corridor signal progression/timing are the typical causes of vehicle delays at signalized corridors.
F	> 80.0	> 50.0	Forced vehicular traffic flow and operations with high approach delays at critical intersections. Vehicle speeds are reduced substantially, and stoppages may occur for short or long periods of time because of downstream congestion.

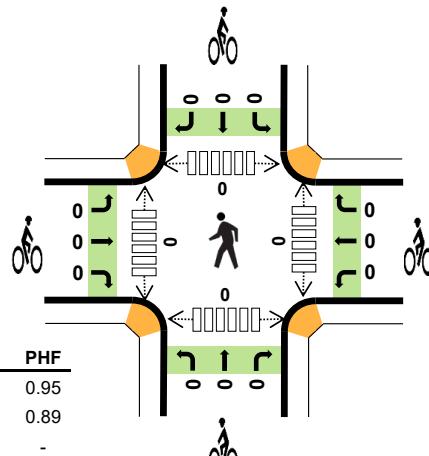
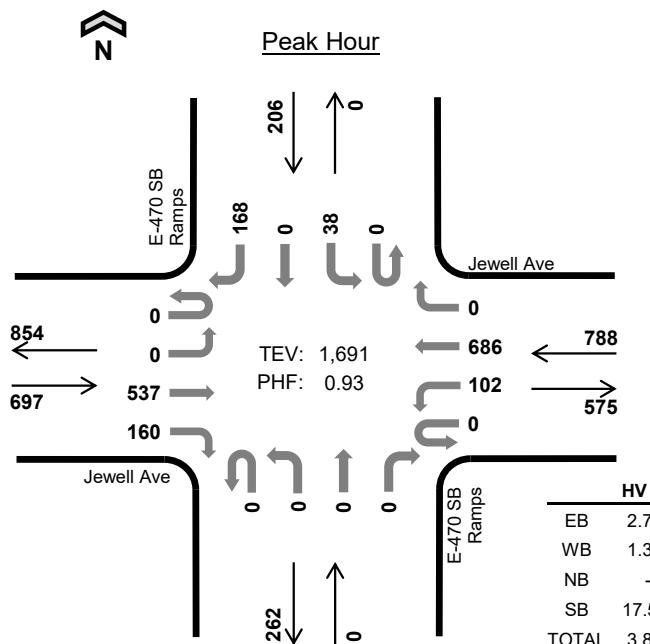
(a) Delay ranges based on Highway Capacity Manual (6th Edition, 2016) criteria.

Existing Traffic Data

E-470 SB Ramps Jewell Ave



Date: 11/01/2023
 Count Period: 7:00 AM to 9:00 AM
 Peak Hour: 7:15 AM to 8:15 AM



Two-Hour Count Summaries

Interval Start	Jewell Ave				Jewell Ave				E-470 SB Ramps				E-470 SB Ramps				15-min Total	Rolling One Hour
	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT		
7:00 AM	0	0	117	13	0	11	164	0	0	0	0	0	0	8	0	38	351	0
7:15 AM	0	0	144	22	0	21	167	0	0	0	0	0	0	9	0	41	404	0
7:30 AM	0	0	137	47	0	30	192	0	0	0	0	0	0	9	0	42	457	0
7:45 AM	0	0	129	53	0	30	183	0	0	0	0	0	0	9	0	50	454	1,666
8:00 AM	0	0	127	38	0	21	144	0	0	0	0	0	0	11	0	35	376	1,691
8:15 AM	0	0	117	40	0	43	140	0	0	0	0	0	0	5	1	34	380	1,667
8:30 AM	0	0	107	41	0	36	142	0	0	0	0	0	0	10	1	25	362	1,572
8:45 AM	0	0	96	30	0	36	125	0	0	0	0	0	0	9	0	24	320	1,438
Count Total	0	0	974	284	0	228	1,257	0	0	0	0	0	0	70	2	289	3,104	0
Peak Hour	All	0	0	537	160	0	102	686	0	0	0	0	0	38	0	168	1,691	0
	HV	0	0	10	9	0	2	8	0	0	0	0	0	22	0	14	65	0
	HV%	-	-	2%	6%	-	2%	1%	-	-	-	-	-	58%	-	8%	4%	0

Note: Two-hour count summary volumes include heavy vehicles but exclude bicycles in overall count.

Interval Start	Heavy Vehicle Totals				Bicycles				Pedestrians (Crossing Leg)				
	EB	WB	NB	SB	EB	WB	NB	SB	East	West	North	South	Total
7:00 AM	3	2	0	19	24	0	0	0	0	0	0	0	0
7:15 AM	4	2	0	7	13	0	0	0	0	0	0	0	0
7:30 AM	3	5	0	7	15	0	0	0	0	0	0	0	0
7:45 AM	5	2	0	9	16	0	0	0	0	0	0	0	0
8:00 AM	7	1	0	13	21	0	0	0	0	0	0	0	0
8:15 AM	10	4	0	3	17	0	0	0	0	0	0	0	0
8:30 AM	3	4	0	7	14	0	0	0	0	0	0	0	0
8:45 AM	6	6	0	4	16	0	0	0	0	0	0	0	0
Count Total	41	26	0	69	136	0	0	0	0	0	0	0	0
Peak Hour	19	10	0	36	65	0	0	0	0	0	0	0	0

Two-Hour Count Summaries - Heavy Vehicles																				
Interval Start	Jewell Ave				Jewell Ave				E-470 SB Ramps				E-470 SB Ramps				15-min Total	Rolling One Hour		
	Eastbound				Westbound				Northbound				Southbound							
	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT				
7:00 AM	0	0	2	1	0	1	1	0	0	0	0	0	0	3	0	16	24	0		
7:15 AM	0	0	3	1	0	1	1	0	0	0	0	0	0	6	0	1	13	0		
7:30 AM	0	0	2	1	0	0	5	0	0	0	0	0	0	5	0	2	15	0		
7:45 AM	0	0	1	4	0	1	1	0	0	0	0	0	0	5	0	4	16	68		
8:00 AM	0	0	4	3	0	0	1	0	0	0	0	0	0	6	0	7	21	65		
8:15 AM	0	0	7	3	0	1	3	0	0	0	0	0	0	2	0	1	17	69		
8:30 AM	0	0	2	1	0	0	4	0	0	0	0	0	0	4	0	3	14	68		
8:45 AM	0	0	6	0	0	2	4	0	0	0	0	0	0	3	0	1	16	68		
Count Total	0	0	27	14	0	6	20	0	0	0	0	0	0	34	0	35	136	0		
Peak Hour	0	0	10	9	0	2	8	0	0	0	0	0	0	22	0	14	65	0		
Two-Hour Count Summaries - Bikes																				
Interval Start	Jewell Ave				Jewell Ave				E-470 SB Ramps				E-470 SB Ramps				15-min Total	Rolling One Hour		
	Eastbound				Westbound				Northbound				Southbound							
	LT	TH	RT		LT	TH	RT		LT	TH	RT		LT	TH	RT					
7:00 AM	0	0	0		0	0	0		0	0	0		0	0	0		0	0		
7:15 AM	0	0	0		0	0	0		0	0	0		0	0	0		0	0		
7:30 AM	0	0	0		0	0	0		0	0	0		0	0	0		0	0		
7:45 AM	0	0	0		0	0	0		0	0	0		0	0	0		0	0		
8:00 AM	0	0	0		0	0	0		0	0	0		0	0	0		0	0		
8:15 AM	0	0	0		0	0	0		0	0	0		0	0	0		0	0		
8:30 AM	0	0	0		0	0	0		0	0	0		0	0	0		0	0		
8:45 AM	0	0	0		0	0	0		0	0	0		0	0	0		0	0		
Count Total	0	0	0		0	0	0		0	0	0		0	0	0		0	0		
Peak Hour	0	0	0		0	0	0		0	0	0		0	0	0		0	0		

Note: U-Turn volumes for bikes are included in Left-Turn, if any.

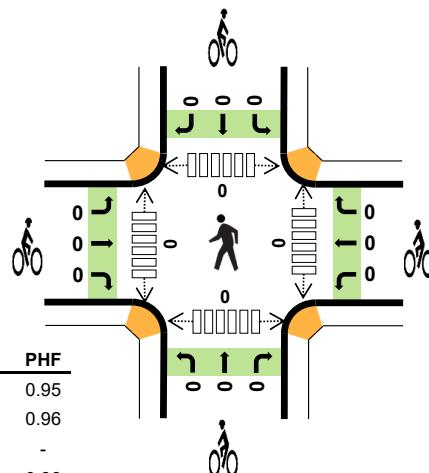
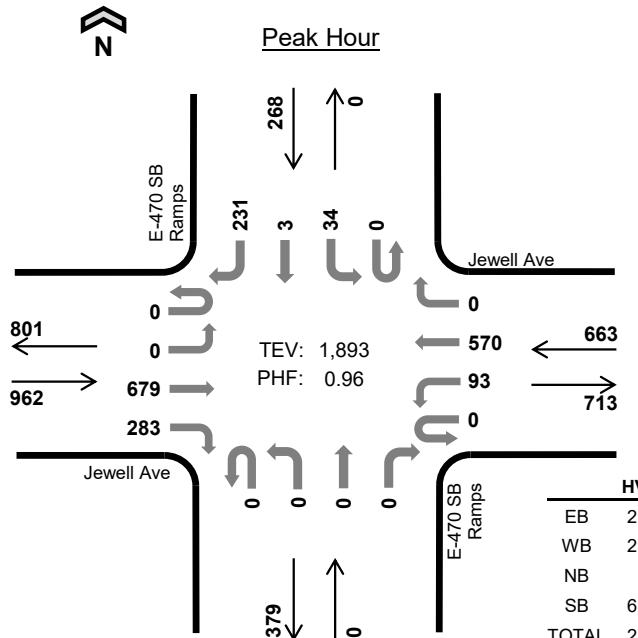
E-470 SB Ramps Jewell Ave



Date: 11/01/2023

Count Period: 4:00 PM to 6:00 PM

Peak Hour: 4:00 PM to 5:00 PM



Two-Hour Count Summaries

Interval Start		Jewell Ave				Jewell Ave				E-470 SB Ramps				E-470 SB Ramps				15-min Total	Rolling One Hour
		Eastbound		Westbound		Northbound		Southbound											
	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT			
4:00 PM	0	0	177	62	0	24	146	0	0	0	0	0	0	8	0	54	471	0	
4:15 PM	0	0	153	73	0	24	134	0	0	0	0	0	0	8	1	54	447	0	
4:30 PM	0	0	165	78	0	21	141	0	0	0	0	0	0	12	1	65	483	0	
4:45 PM	0	0	184	70	0	24	149	0	0	0	0	0	0	6	1	58	492	1,893	
5:00 PM	0	0	151	61	0	18	130	0	0	0	0	0	0	3	0	80	443	1,865	
5:15 PM	0	0	148	49	0	21	136	0	0	0	0	0	0	10	0	52	416	1,834	
5:30 PM	0	0	145	42	0	13	147	0	0	0	0	0	0	11	0	63	421	1,772	
5:45 PM	0	0	148	35	0	12	147	0	0	0	0	0	0	11	0	42	395	1,675	
Count Total		0	0	1,271	470	0	157	1,130	0	0	0	0	0	69	3	468	3,568	0	
Peak Hour	All	0	0	679	283	0	93	570	0	0	0	0	0	34	3	231	1,893	0	
	HV	0	0	16	4	0	3	11	0	0	0	0	0	12	0	5	51	0	
	HV%	-	-	2%	1%	-	3%	2%	-	-	-	-	-	35%	0%	2%	3%	0	

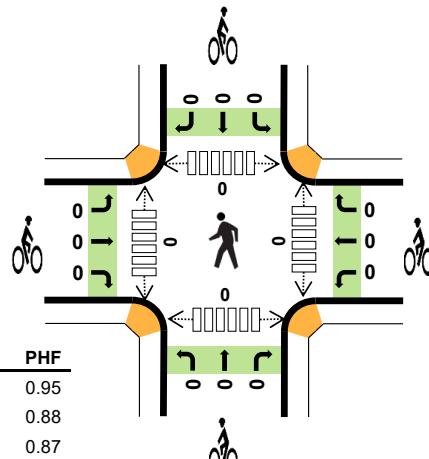
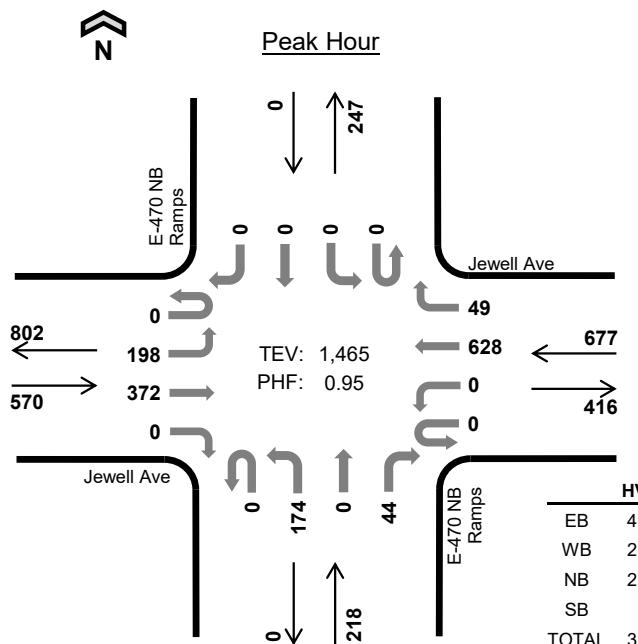
Note: Two-hour count summary volumes include heavy vehicles but exclude bicycles in overall count.

Two-Hour Count Summaries - Heavy Vehicles																		
Interval Start	Jewell Ave				Jewell Ave				E-470 SB Ramps				E-470 SB Ramps				15-min Total	Rolling One Hour
	Eastbound		Westbound		Northbound		Southbound											
	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT		
4:00 PM	0	0	5	2	0	2	2	0	0	0	0	0	0	0	0	1	12	0
4:15 PM	0	0	2	0	0	0	2	0	0	0	0	0	0	3	0	2	9	0
4:30 PM	0	0	5	0	0	1	3	0	0	0	0	0	0	7	0	2	18	0
4:45 PM	0	0	4	2	0	0	4	0	0	0	0	0	0	2	0	0	12	51
5:00 PM	0	0	1	3	0	0	4	0	0	0	0	0	0	0	0	6	14	53
5:15 PM	0	0	1	0	0	1	2	0	0	0	0	0	0	2	0	2	8	52
5:30 PM	0	0	2	2	0	0	0	0	0	0	0	0	0	2	0	1	7	41
5:45 PM	0	0	0	0	0	0	3	0	0	0	0	0	0	5	0	0	8	37
Count Total	0	0	20	9	0	4	20	0	0	0	0	0	0	21	0	14	88	0
Peak Hour	0	0	16	4	0	3	11	0	0	0	0	0	0	12	0	5	51	0
Two-Hour Count Summaries - Bikes																		
Interval Start	Jewell Ave				Jewell Ave				E-470 SB Ramps				E-470 SB Ramps				15-min Total	Rolling One Hour
	Eastbound		Westbound		Northbound		Southbound											
	LT	TH	RT		LT	TH	RT		LT	TH	RT		LT	TH	RT			
4:00 PM	0	0	0		0	0	0		0	0	0		0	0	0		0	0
4:15 PM	0	0	0		0	0	0		0	0	0		0	0	0		0	0
4:30 PM	0	0	0		0	0	0		0	0	0		0	0	0		0	0
4:45 PM	0	0	0		0	0	0		0	0	0		0	0	0		0	0
5:00 PM	0	0	0		0	0	0		0	0	0		0	0	0		0	0
5:15 PM	0	0	0		0	0	0		0	0	0		0	0	0		0	0
5:30 PM	0	0	0		0	0	0		0	0	0		0	0	0		0	0
5:45 PM	0	0	0		0	0	0		0	0	0		0	0	0		0	0
Count Total	0	0	0		0	0	0		0	0	0		0	0	0		0	0
Peak Hour	0	0	0		0	0	0		0	0	0		0	0	0		0	0
Note: U-Turn volumes for bikes are included in Left-Turn, if any.																		

E-470 NB Ramps Jewell Ave



Date: 11/01/2023
 Count Period: 7:00 AM to 9:00 AM
 Peak Hour: 7:15 AM to 8:15 AM



Two-Hour Count Summaries

Interval Start	Jewell Ave				Jewell Ave				E-470 NB Ramps				E-470 NB Ramps				15-min Total	Rolling One Hour	
	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT			
7:00 AM	0	38	83	0	0	0	138	13	0	36	0	17	0	0	0	0	325	0	
7:15 AM	0	56	94	0	0	0	147	23	0	40	0	6	0	0	0	0	366	0	
7:30 AM	0	43	96	0	0	0	182	11	0	45	0	8	0	0	0	0	385	0	
7:45 AM	0	60	82	0	0	0	171	9	0	48	0	15	0	0	0	0	385	1,461	
8:00 AM	0	39	100	0	0	0	128	6	0	41	0	15	0	0	0	0	329	1,465	
8:15 AM	0	36	94	0	0	0	153	6	0	30	0	9	0	0	0	0	328	1,427	
8:30 AM	0	37	85	0	0	0	134	13	0	39	0	9	0	0	0	0	317	1,359	
8:45 AM	0	27	75	0	0	0	125	3	0	29	0	11	0	0	0	0	270	1,244	
Count Total	0	336	709	0	0	0	1,178	84	0	308	0	90	0	0	0	0	2,705	0	
Peak Hour	All	0	198	372	0	0	0	628	49	0	174	0	44	0	0	0	0	1,465	0
HV	0	2	26	0	0	0	9	9	0	1	0	5	0	0	0	0	52	0	
HV%	-	1%	7%	-	-	-	1%	18%	-	1%	-	11%	-	-	-	-	4%	0	

Note: Two-hour count summary volumes include heavy vehicles but exclude bicycles in overall count.

Interval Start	Heavy Vehicle Totals					Bicycles					Pedestrians (Crossing Leg)				
	EB	WB	NB	SB	Total	EB	WB	NB	SB	Total	East	West	North	South	Total
7:00 AM	7	4	0	0	11	0	0	0	0	0	0	0	0	0	0
7:15 AM	8	7	2	0	17	0	0	0	0	0	0	0	0	0	0
7:30 AM	7	5	2	0	14	0	0	0	0	0	0	0	0	0	0
7:45 AM	6	1	2	0	9	0	0	0	0	0	0	0	0	0	0
8:00 AM	7	5	0	0	12	0	0	0	0	0	0	0	0	0	0
8:15 AM	11	5	1	0	17	0	0	0	0	0	0	0	0	0	0
8:30 AM	6	4	3	0	13	0	0	0	0	0	0	0	0	0	0
8:45 AM	9	5	2	0	16	0	0	0	0	0	0	0	0	0	0
Count Total	61	36	12	0	109	0	0	0	0	0	0	0	0	0	0
Peak Hour	28	18	6	0	52	0	0	0	0	0	0	0	0	0	0

Two-Hour Count Summaries - Heavy Vehicles																				
Interval Start	Jewell Ave				Jewell Ave				E-470 NB Ramps				E-470 NB Ramps				15-min Total	Rolling One Hour		
	Eastbound				Westbound				Northbound				Southbound							
	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT				
7:00 AM	0	3	4	0	0	0	2	2	0	0	0	0	0	0	0	0	11	0		
7:15 AM	0	2	6	0	0	0	2	5	0	0	0	2	0	0	0	0	17	0		
7:30 AM	0	0	7	0	0	0	5	0	0	1	0	1	0	0	0	0	14	0		
7:45 AM	0	0	6	0	0	0	1	0	0	0	0	2	0	0	0	0	9	51		
8:00 AM	0	0	7	0	0	0	1	4	0	0	0	0	0	0	0	0	12	52		
8:15 AM	0	2	9	0	0	0	4	1	0	0	0	1	0	0	0	0	17	52		
8:30 AM	0	1	5	0	0	0	3	1	0	1	0	2	0	0	0	0	13	51		
8:45 AM	0	1	8	0	0	0	5	0	0	1	0	1	0	0	0	0	16	58		
Count Total	0	9	52	0	0	0	23	13	0	3	0	9	0	0	0	0	109	0		
Peak Hour	0	2	26	0	0	0	9	9	0	1	0	5	0	0	0	0	52	0		
Two-Hour Count Summaries - Bikes																				
Interval Start	Jewell Ave				Jewell Ave				E-470 NB Ramps				E-470 NB Ramps				15-min Total	Rolling One Hour		
	Eastbound				Westbound				Northbound				Southbound							
	LT	TH	RT		LT	TH	RT		LT	TH	RT		LT	TH	RT					
7:00 AM	0	0	0		0	0	0		0	0	0		0	0	0		0	0		
7:15 AM	0	0	0		0	0	0		0	0	0		0	0	0		0	0		
7:30 AM	0	0	0		0	0	0		0	0	0		0	0	0		0	0		
7:45 AM	0	0	0		0	0	0		0	0	0		0	0	0		0	0		
8:00 AM	0	0	0		0	0	0		0	0	0		0	0	0		0	0		
8:15 AM	0	0	0		0	0	0		0	0	0		0	0	0		0	0		
8:30 AM	0	0	0		0	0	0		0	0	0		0	0	0		0	0		
8:45 AM	0	0	0		0	0	0		0	0	0		0	0	0		0	0		
Count Total	0	0	0		0	0	0		0	0	0		0	0	0		0	0		
Peak Hour	0	0	0		0	0	0		0	0	0		0	0	0		0	0		

Note: U-Turn volumes for bikes are included in Left-Turn, if any.

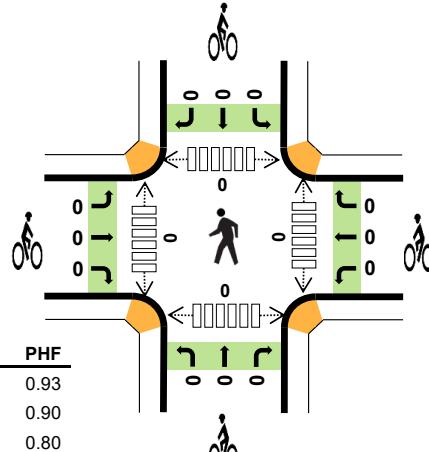
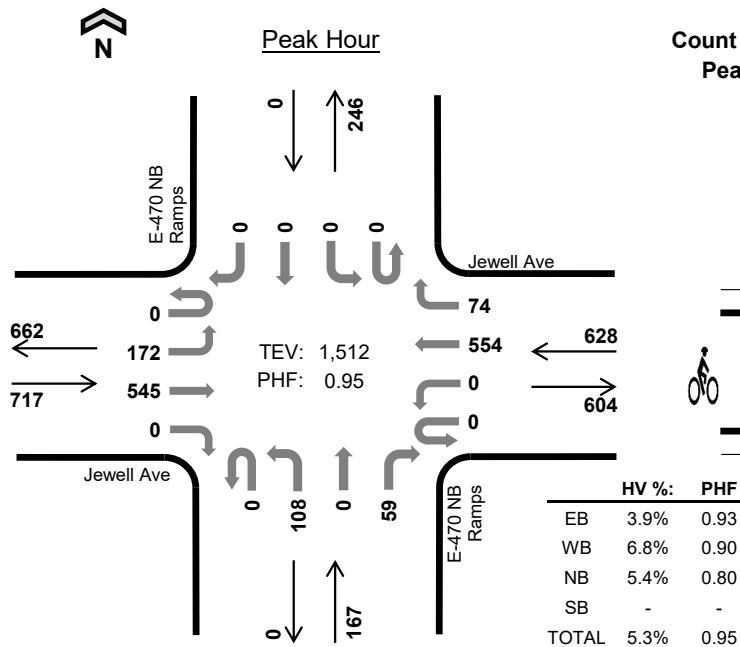
E-470 NB Ramps Jewell Ave



Date: 11/01/2023

Count Period: 4:00 PM to 6:00 PM

Peak Hour: 4:00 PM to 5:00 PM



Two-Hour Count Summaries

Interval Start	Jewell Ave				Jewell Ave				E-470 NB Ramps				E-470 NB Ramps				15-min Total	Rolling One Hour							
	Eastbound		Westbound		Northbound		Southbound		UT		LT		TH		RT		UT		LT		TH		RT		
4:00 PM	0	46	139	0	0	0	156	18	0	22	0	12	0	0	0	0	0	0	0	0	0	0	0	393	0
4:15 PM	0	45	114	0	0	0	123	17	0	28	0	12	0	0	0	0	0	0	0	0	0	0	0	339	0
4:30 PM	0	45	136	0	0	0	127	22	0	30	0	22	0	0	0	0	0	0	0	0	0	0	0	382	0
4:45 PM	0	36	156	0	0	0	148	17	0	28	0	13	0	0	0	0	0	0	0	0	0	0	0	398	1,512
5:00 PM	0	34	108	0	0	0	115	13	0	30	1	17	0	0	0	0	0	0	0	0	0	0	0	318	1,437
5:15 PM	0	32	136	0	0	0	120	16	0	35	0	16	0	0	0	0	0	0	0	0	0	0	0	355	1,453
5:30 PM	0	29	129	0	0	0	132	15	0	32	0	20	0	0	0	0	0	0	0	0	0	0	0	357	1,428
5:45 PM	0	29	126	0	0	0	127	16	0	35	0	14	0	0	0	0	0	0	0	0	0	0	0	347	1,377
Count Total	0	296	1,044	0	0	0	1,048	134	0	240	1	126	0	0	0	0	0	0	0	0	0	0	0	2,889	0
Peak Hour	All	0	172	545	0	0	0	554	74	0	108	0	59	0	0	0	0	0	0	0	0	0	0	1,512	0
	HV	0	13	15	0	0	0	10	33	0	4	0	5	0	0	0	0	0	0	0	0	0	0	80	0
	HV%	-	8%	3%	-	-	-	2%	45%	-	4%	-	8%	-	-	-	-	-	-	-	-	-	-	5%	0

Note: Two-hour count summary volumes include heavy vehicles but exclude bicycles in overall count.

Interval Start	Heavy Vehicle Totals					Bicycles					Pedestrians (Crossing Leg)				
	EB	WB	NB	SB	Total	EB	WB	NB	SB	Total	East	West	North	South	Total
4:00 PM	4	15	2	0	21	0	0	0	0	0	0	0	0	0	0
4:15 PM	7	6	2	0	15	0	0	0	0	0	0	0	0	0	0
4:30 PM	11	15	1	0	27	0	0	0	0	0	0	0	0	0	0
4:45 PM	6	7	4	0	17	0	0	0	0	0	0	0	0	0	0
5:00 PM	1	7	2	0	10	0	0	0	0	0	0	0	0	0	0
5:15 PM	3	7	3	0	13	0	0	0	0	0	0	0	0	0	0
5:30 PM	4	2	1	0	7	0	0	0	0	0	0	0	0	0	0
5:45 PM	5	3	3	0	11	0	0	0	0	0	0	0	0	0	0
Count Total	41	62	18	0	121	0	0	0	0	0	0	0	0	0	0
Peak Hour	28	43	9	0	80	0	0	0	0	0	0	0	0	0	0

Two-Hour Count Summaries - Heavy Vehicles																				
Interval Start	Jewell Ave				Jewell Ave				E-470 NB Ramps				E-470 NB Ramps				15-min Total	Rolling One Hour		
	Eastbound				Westbound				Northbound				Southbound							
	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT				
4:00 PM	0	4	0	0	0	0	4	11	0	0	0	2	0	0	0	0	21	0		
4:15 PM	0	4	3	0	0	0	1	5	0	1	0	1	0	0	0	0	15	0		
4:30 PM	0	3	8	0	0	0	4	11	0	0	0	1	0	0	0	0	27	0		
4:45 PM	0	2	4	0	0	0	1	6	0	3	0	1	0	0	0	0	17	80		
5:00 PM	0	1	0	0	0	0	2	5	0	2	0	0	0	0	0	0	10	69		
5:15 PM	0	1	2	0	0	0	1	6	0	2	0	1	0	0	0	0	13	67		
5:30 PM	0	2	2	0	0	0	0	2	0	0	0	1	0	0	0	0	7	47		
5:45 PM	0	0	5	0	0	0	0	3	0	3	0	0	0	0	0	0	11	41		
Count Total	0	17	24	0	0	0	13	49	0	11	0	7	0	0	0	0	121	0		
Peak Hour	0	13	15	0	0	0	10	33	0	4	0	5	0	0	0	0	80	0		
Two-Hour Count Summaries - Bikes																				
Interval Start	Jewell Ave				Jewell Ave				E-470 NB Ramps				E-470 NB Ramps				15-min Total	Rolling One Hour		
	Eastbound				Westbound				Northbound				Southbound							
	LT	TH	RT		LT	TH	RT		LT	TH	RT		LT	TH	RT					
4:00 PM	0	0	0		0	0	0		0	0	0		0	0	0		0	0		
4:15 PM	0	0	0		0	0	0		0	0	0		0	0	0		0	0		
4:30 PM	0	0	0		0	0	0		0	0	0		0	0	0		0	0		
4:45 PM	0	0	0		0	0	0		0	0	0		0	0	0		0	0		
5:00 PM	0	0	0		0	0	0		0	0	0		0	0	0		0	0		
5:15 PM	0	0	0		0	0	0		0	0	0		0	0	0		0	0		
5:30 PM	0	0	0		0	0	0		0	0	0		0	0	0		0	0		
5:45 PM	0	0	0		0	0	0		0	0	0		0	0	0		0	0		
Count Total	0	0	0		0	0	0		0	0	0		0	0	0		0	0		
Peak Hour	0	0	0		0	0	0		0	0	0		0	0	0		0	0		
Note: U-Turn volumes for bikes are included in Left-Turn, if any.																				

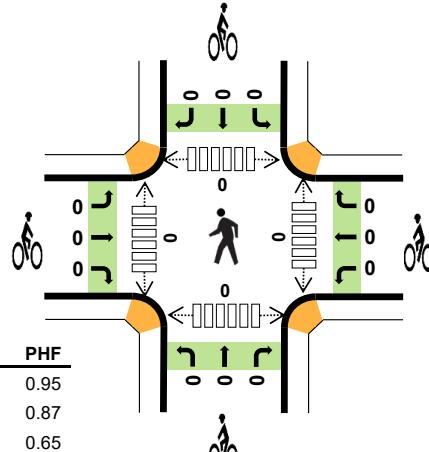
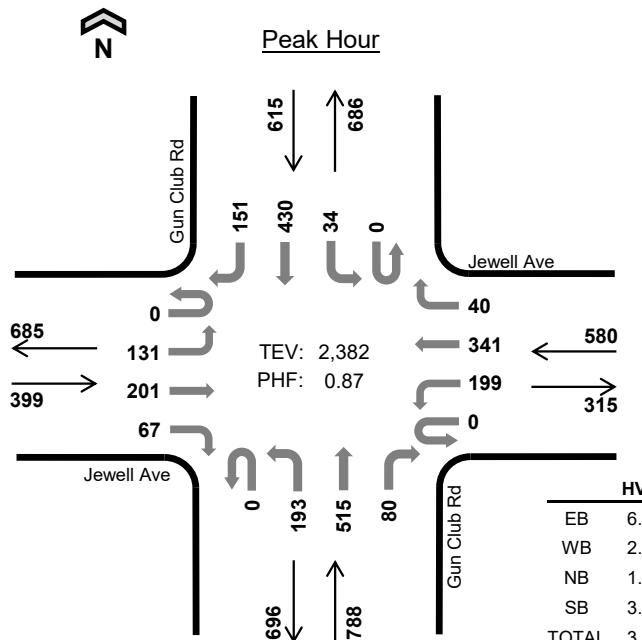
Gun Club Rd Jewell Ave



Date: 11/01/2023

Count Period: 7:00 AM to 9:00 AM

Peak Hour: 7:00 AM to 8:00 AM



Two-Hour Count Summaries

Interval Start	Jewell Ave				Jewell Ave				Gun Club Rd				Gun Club Rd				15-min Total	Rolling One Hour	
	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT			
7:00 AM	0	28	68	9	0	38	64	12	0	59	145	25	0	14	75	35	572	0	
7:15 AM	0	28	49	16	0	55	72	7	0	70	208	26	0	8	111	38	688	0	
7:30 AM	0	37	43	21	0	62	94	10	0	44	108	21	0	5	116	37	598	0	
7:45 AM	0	38	41	21	0	44	111	11	0	20	54	8	0	7	128	41	524	2,382	
8:00 AM	0	44	47	24	0	47	79	11	0	19	76	16	0	11	101	40	515	2,325	
8:15 AM	0	37	44	18	1	42	69	8	0	53	103	15	0	2	93	42	527	2,164	
8:30 AM	0	36	44	19	0	26	48	12	0	59	87	24	0	4	92	40	491	2,057	
8:45 AM	0	36	41	10	0	35	50	8	0	45	53	15	0	5	82	34	414	1,947	
Count Total	0	284	377	138	1	349	587	79	0	369	834	150	0	56	798	307	4,329	0	
Peak Hour	All	0	131	201	67	0	199	341	40	0	193	515	80	0	34	430	151	2,382	0
	HV	0	3	13	10	0	3	8	3	0	5	6	3	0	3	18	3	78	0
	HV%	-	2%	6%	15%	-	2%	2%	8%	-	3%	1%	4%	-	9%	4%	2%	3%	0

Note: Two-hour count summary volumes include heavy vehicles but exclude bicycles in overall count.

Interval Start	Heavy Vehicle Totals					Bicycles					Pedestrians (Crossing Leg)				
	EB	WB	NB	SB	Total	EB	WB	NB	SB	Total	East	West	North	South	Total
7:00 AM	4	2	3	2	11	0	0	0	0	0	0	0	0	0	0
7:15 AM	7	3	8	5	23	0	0	0	0	0	0	0	0	0	0
7:30 AM	8	3	3	8	22	0	0	0	0	0	0	0	0	0	0
7:45 AM	7	6	0	9	22	0	0	0	0	0	0	0	0	0	0
8:00 AM	7	2	2	10	21	0	0	0	0	0	0	0	0	0	0
8:15 AM	11	1	4	10	26	0	0	0	0	0	0	0	0	0	0
8:30 AM	9	2	3	8	22	0	0	0	0	0	0	0	0	0	0
8:45 AM	8	3	6	11	28	0	0	0	0	0	0	0	0	0	0
Count Total	61	22	29	63	175	0	0	0	0	0	0	0	0	0	0
Peak Hour	26	14	14	24	78	0	0	0	0	0	0	0	0	0	0

Two-Hour Count Summaries - Heavy Vehicles																			
Interval Start	Jewell Ave				Jewell Ave				Gun Club Rd				Gun Club Rd				15-min Total	Rolling One Hour	
	Eastbound				Westbound				Northbound				Southbound						
	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT			
7:00 AM	0	1	3	0	0	0	2	0	0	1	2	0	0	1	0	1	11	0	
7:15 AM	0	0	4	3	0	0	3	0	0	4	3	1	0	2	3	0	23	0	
7:30 AM	0	1	2	5	0	1	2	0	0	0	1	2	0	0	0	6	22	0	
7:45 AM	0	1	4	2	0	2	1	3	0	0	0	0	0	0	0	9	0	22	78
8:00 AM	0	0	3	4	0	0	2	0	0	2	0	0	0	1	8	1	21	88	
8:15 AM	0	4	3	4	0	0	1	0	0	2	2	0	0	0	8	2	26	91	
8:30 AM	0	0	6	3	0	0	1	1	0	1	2	0	0	0	5	3	22	91	
8:45 AM	0	1	5	2	0	1	1	1	0	2	1	3	0	0	9	2	28	97	
Count Total	0	8	30	23	0	4	13	5	0	12	11	6	0	4	48	11	175	0	
Peak Hour	0	3	13	10	0	3	8	3	0	5	6	3	0	3	18	3	78	0	
Two-Hour Count Summaries - Bikes																			
Interval Start	Jewell Ave				Jewell Ave				Gun Club Rd				Gun Club Rd				15-min Total	Rolling One Hour	
	Eastbound				Westbound				Northbound				Southbound						
	LT	TH	RT		LT	TH	RT		LT	TH	RT		LT	TH	RT				
7:00 AM	0	0	0		0	0	0		0	0	0		0	0	0		0	0	
7:15 AM	0	0	0		0	0	0		0	0	0		0	0	0		0	0	
7:30 AM	0	0	0		0	0	0		0	0	0		0	0	0		0	0	
7:45 AM	0	0	0		0	0	0		0	0	0		0	0	0		0	0	
8:00 AM	0	0	0		0	0	0		0	0	0		0	0	0		0	0	
8:15 AM	0	0	0		0	0	0		0	0	0		0	0	0		0	0	
8:30 AM	0	0	0		0	0	0		0	0	0		0	0	0		0	0	
8:45 AM	0	0	0		0	0	0		0	0	0		0	0	0		0	0	
Count Total	0	0	0		0	0	0		0	0	0		0	0	0		0	0	
Peak Hour	0	0	0		0	0	0		0	0	0		0	0	0		0	0	
Note: U-Turn volumes for bikes are included in Left-Turn, if any.																			

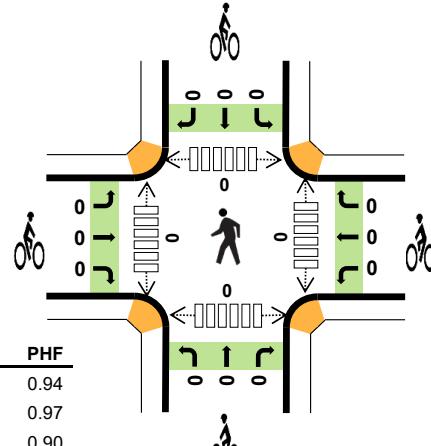
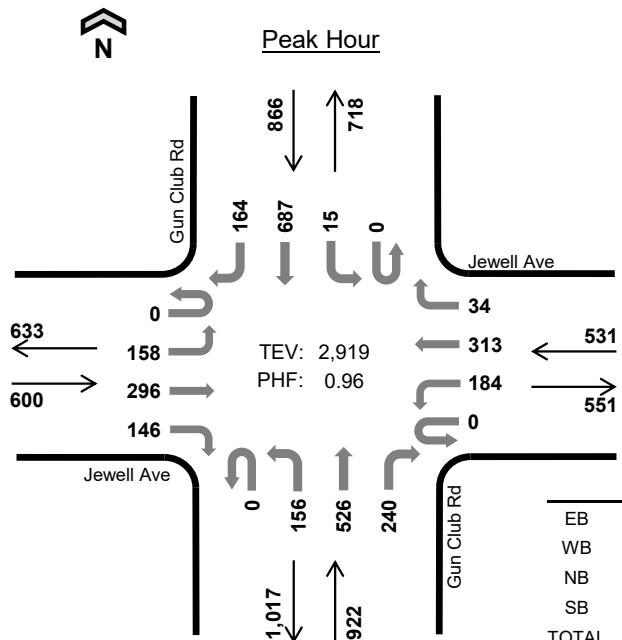
Gun Club Rd Jewell Ave



Date: 11/01/2023

Count Period: 4:00 PM to 6:00 PM

Peak Hour: 4:00 PM to 5:00 PM



Two-Hour Count Summaries

Interval Start	Jewell Ave				Jewell Ave				Gun Club Rd				Gun Club Rd				15-min Total	Rolling One Hour	
	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT			
4:00 PM	0	44	57	46	0	57	71	5	0	49	120	55	0	1	172	55	732	0	
4:15 PM	0	36	71	28	0	51	78	8	0	30	127	68	0	4	158	37	696	0	
4:30 PM	0	38	86	34	0	33	84	13	0	43	147	65	0	7	179	33	762	0	
4:45 PM	0	40	82	38	0	43	80	8	0	34	132	52	0	3	178	39	729	2,919	
5:00 PM	0	40	68	30	0	48	55	24	0	31	119	63	0	5	178	36	697	2,884	
5:15 PM	0	38	74	36	0	39	61	5	0	37	125	60	0	5	196	46	722	2,910	
5:30 PM	0	45	65	29	0	40	58	12	0	42	172	57	0	6	187	39	752	2,900	
5:45 PM	0	42	67	37	0	35	61	7	0	45	122	44	0	0	161	39	660	2,831	
Count Total	0	323	570	278	0	346	548	82	0	311	1,064	464	0	31	1,409	324	5,750	0	
Peak Hour	All	0	158	296	146	0	184	313	34	0	156	526	240	0	15	687	164	2,919	0
	HV	0	4	7	10	0	1	15	3	0	26	30	5	0	0	6	1	108	0
	HV%	-	3%	2%	7%	-	1%	5%	9%	-	17%	6%	2%	-	0%	1%	1%	4%	0

Note: Two-hour count summary volumes include heavy vehicles but exclude bicycles in overall count.

Interval Start	Heavy Vehicle Totals					Bicycles					Pedestrians (Crossing Leg)				
	EB	WB	NB	SB	Total	EB	WB	NB	SB	Total	East	West	North	South	Total
4:00 PM	4	9	13	3	29	0	0	0	0	0	0	0	0	0	0
4:15 PM	3	3	17	2	25	0	0	0	0	0	0	0	0	0	0
4:30 PM	9	3	17	1	30	0	0	0	0	0	0	0	0	0	0
4:45 PM	5	4	14	1	24	0	0	0	0	0	0	0	0	0	0
5:00 PM	2	4	16	4	26	0	0	0	0	0	0	0	0	0	0
5:15 PM	3	0	12	2	17	0	0	0	0	0	0	0	0	0	0
5:30 PM	2	1	2	2	7	0	0	0	0	0	0	0	0	0	0
5:45 PM	3	0	8	0	11	0	0	0	0	0	0	0	0	0	0
Count Total	31	24	99	15	169	0	0	0	0	0	0	0	0	0	0
Peak Hour	21	19	61	7	108	0	0	0	0	0	0	0	0	0	0

Two-Hour Count Summaries - Heavy Vehicles																		
Interval Start	Jewell Ave				Jewell Ave				Gun Club Rd				Gun Club Rd				15-min Total	Rolling One Hour
	Eastbound				Westbound				Northbound				Southbound					
	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT		
4:00 PM	0	1	2	1	0	1	8	0	0	7	5	1	0	0	2	1	29	0
4:15 PM	0	0	1	2	0	0	2	1	0	4	9	4	0	0	2	0	25	0
4:30 PM	0	1	3	5	0	0	2	1	0	11	6	0	0	0	1	0	30	0
4:45 PM	0	2	1	2	0	0	3	1	0	4	10	0	0	0	1	0	24	108
5:00 PM	0	1	0	1	0	1	2	1	0	8	7	1	0	0	3	1	26	105
5:15 PM	0	0	1	2	0	0	0	0	0	4	7	1	0	0	1	1	17	97
5:30 PM	0	0	0	2	0	0	0	1	0	1	0	1	0	0	2	0	7	74
5:45 PM	0	0	0	3	0	0	0	0	0	3	5	0	0	0	0	0	11	61
Count Total	0	5	8	18	0	2	17	5	0	42	49	8	0	0	12	3	169	0
Peak Hour	0	4	7	10	0	1	15	3	0	26	30	5	0	0	6	1	108	0
Two-Hour Count Summaries - Bikes																		
Interval Start	Jewell Ave				Jewell Ave				Gun Club Rd				Gun Club Rd				15-min Total	Rolling One Hour
	Eastbound				Westbound				Northbound				Southbound					
	LT	TH	RT		LT	TH	RT		LT	TH	RT		LT	TH	RT			
4:00 PM	0	0	0		0	0	0		0	0	0		0	0	0		0	0
4:15 PM	0	0	0		0	0	0		0	0	0		0	0	0		0	0
4:30 PM	0	0	0		0	0	0		0	0	0		0	0	0		0	0
4:45 PM	0	0	0		0	0	0		0	0	0		0	0	0		0	0
5:00 PM	0	0	0		0	0	0		0	0	0		0	0	0		0	0
5:15 PM	0	0	0		0	0	0		0	0	0		0	0	0		0	0
5:30 PM	0	0	0		0	0	0		0	0	0		0	0	0		0	0
5:45 PM	0	0	0		0	0	0		0	0	0		0	0	0		0	0
Count Total	0	0	0		0	0	0		0	0	0		0	0	0		0	0
Peak Hour	0	0	0		0	0	0		0	0	0		0	0	0		0	0
Note: U-Turn volumes for bikes are included in Left-Turn, if any.																		

Vehicle Classification Report Summary

Location: Jewell Ave W/O Gun Club Rd

Count Direction: Eastbound / Westbound

Date Range: 10/31/2023 to 11/2/2023

Site Code: 01

Direction	FHWA Vehicle Classification													Total Volume
	1	2	3	4	5	6	7	8	9	10	11	12	13	
Eastbound	53	14,386	3,316	126	739	253	0	28	285	16	0	0	20	19,222
	0.3%	74.8%	17.3%	0.7%	3.8%	1.3%	0.0%	0.1%	1.5%	0.1%	0.0%	0.0%	0.1%	
Westbound	86	17,166	2,862	105	550	844	0	5	165	33	0	0	117	21,933
	0.4%	78.3%	13.0%	0.5%	2.5%	3.8%	0.0%	0.0%	0.8%	0.2%	0.0%	0.0%	0.5%	
Total	139	31,552	6,178	231	1,289	1,097	0	33	450	49	0	0	137	41,155
	0.3%	76.7%	15.0%	0.6%	3.1%	2.7%	0.0%	0.1%	1.1%	0.1%	0.0%	0.0%	0.3%	

FHWA Vehicle Classification													
Class 1 - Motorcycles							Class 8 - Four or Fewer Axle Single-Trailer Trucks						
Class 2 - Passenger Cars							Class 9 - Five-Axle Single-Trailer Trucks						
Class 3 - Other Two-Axle, Four-Tire Single Unit Vehicles							Class 10 - Six or More Axle Single-Trailer Trucks						
Class 4 - Buses							Class 11 - Five or fewer Axle Multi-Trailer Trucks						
Class 5 - Two-Axle, Six-Tire, Single-Unit Trucks							Class 12 - Six-Axle Multi-Trailer Trucks						
Class 6 - Three-Axle Single-Unit Trucks							Class 13 - Seven or More Axle Multi-Trailer Trucks						
Class 7 - Four or More Axle Single-Unit Trucks													

Vehicle Speed Report Summary



Location: Jewell Ave W/O Gun Club Rd

Direction: Eastbound / Westbound

Date Range: 10/31/2023 to 11/2/2023

Site Code: 01

Direction	Speed Range (mph)																	Total Volume
	0 - 10	10 - 15	15 - 20	20 - 25	25 - 30	30 - 35	35 - 40	40 - 45	45 - 50	50 - 55	55 - 60	60 - 65	65 - 70	70 - 75	75 - 80	80 - 85	85 +	
Eastbound	39	76	81	72	61	191	874	3,366	6,822	5,538	1,691	321	60	30	0	0	0	19,222
	0.2%	0.4%	0.4%	0.4%	0.3%	1.0%	4.5%	17.5%	35.5%	28.8%	8.8%	1.7%	0.3%	0.2%	0.0%	0.0%	0.0%	
Westbound	1	2	2	3	39	292	1,702	5,629	8,350	4,515	1,110	214	49	25	0	0	0	21,933
	0.0%	0.0%	0.0%	0.0%	0.2%	1.3%	7.8%	25.7%	38.1%	20.6%	5.1%	1.0%	0.2%	0.1%	0.0%	0.0%	0.0%	
Total	40	78	83	75	100	483	2,576	8,995	15,172	10,053	2,801	535	109	55	0	0	0	41,155
	0.1%	0.2%	0.2%	0.2%	0.2%	1.2%	6.3%	21.9%	36.9%	24.4%	6.8%	1.3%	0.3%	0.1%	0.0%	0.0%	0.0%	

Total Study Percentile Speed Summary		Total Study Speed Statistics	
Eastbound		Eastbound	
50th Percentile (Median)	48.7 mph	Mean (Average) Speed	48.2 mph
85th Percentile	53.9 mph	10 mph Pace	43.6 - 53.6 mph
95th Percentile	57.5 mph	Percent in Pace	65.6 %
Westbound		Westbound	
50th Percentile (Median)	47.0 mph	Mean (Average) Speed	47.0 mph
85th Percentile	52.2 mph	10 mph Pace	42.5 - 52.5 mph
95th Percentile	55.7 mph	Percent in Pace	67.2 %

Location: Jewell Ave W/O Gun Club Rd
 Date Range: 10/31/2023 - 11/6/2023
 Site Code: 01

Time	Tuesday 10/31/2023			Wednesday 11/1/2023			Thursday 11/2/2023			Friday 11/3/2023			Saturday 11/4/2023			Sunday 11/5/2023			Monday 11/6/2023			Mid-Week Average			
	EB	WB	Total	EB	WB	Total	EB	WB	Total	EB	WB	Total	EB	WB	Total	EB	WB	Total	EB	WB	Total	EB	WB	Total	
12:00 AM	43	20	63	40	24	64	36	29	65	-	-	-	-	-	-	-	-	-	-	-	-	-	40	24	64
1:00 AM	21	13	34	20	22	42	22	27	49	-	-	-	-	-	-	-	-	-	-	-	-	-	21	21	42
2:00 AM	21	16	37	20	19	39	17	11	28	-	-	-	-	-	-	-	-	-	-	-	-	-	19	15	35
3:00 AM	14	21	35	21	44	65	20	34	54	-	-	-	-	-	-	-	-	-	-	-	-	-	18	33	51
4:00 AM	51	81	132	45	87	132	38	86	124	-	-	-	-	-	-	-	-	-	-	-	-	-	45	85	129
5:00 AM	62	235	297	81	269	350	89	273	362	-	-	-	-	-	-	-	-	-	-	-	-	-	77	259	336
6:00 AM	254	402	656	248	410	658	272	425	697	-	-	-	-	-	-	-	-	-	-	-	-	-	258	412	670
7:00 AM	437	836	1,273	402	750	1,152	451	745	1,196	-	-	-	-	-	-	-	-	-	-	-	-	-	430	777	1,207
8:00 AM	278	640	918	403	592	995	304	586	890	-	-	-	-	-	-	-	-	-	-	-	-	-	328	606	934
9:00 AM	265	380	645	269	446	715	247	359	606	-	-	-	-	-	-	-	-	-	-	-	-	-	260	395	655
10:00 AM	222	300	522	247	333	580	275	341	616	-	-	-	-	-	-	-	-	-	-	-	-	-	248	325	573
11:00 AM	264	313	577	286	355	641	321	398	719	-	-	-	-	-	-	-	-	-	-	-	-	-	290	355	646
12:00 PM	341	307	648	333	329	662	338	373	711	-	-	-	-	-	-	-	-	-	-	-	-	-	337	336	674
1:00 PM	337	313	650	330	316	646	373	354	727	-	-	-	-	-	-	-	-	-	-	-	-	-	347	328	674
2:00 PM	453	408	861	412	444	856	476	447	923	-	-	-	-	-	-	-	-	-	-	-	-	-	447	433	880
3:00 PM	505	544	1,049	496	571	1,067	553	597	1,150	-	-	-	-	-	-	-	-	-	-	-	-	-	518	571	1,089
4:00 PM	708	575	1,283	612	655	1,267	566	737	1,303	-	-	-	-	-	-	-	-	-	-	-	-	-	629	656	1,284
5:00 PM	696	536	1,232	573	564	1,137	588	541	1,129	-	-	-	-	-	-	-	-	-	-	-	-	-	619	547	1,166
6:00 PM	524	343	867	504	410	914	506	429	935	-	-	-	-	-	-	-	-	-	-	-	-	-	511	394	905
7:00 PM	324	308	632	319	233	552	325	234	559	-	-	-	-	-	-	-	-	-	-	-	-	-	323	258	581
8:00 PM	260	306	566	255	147	402	254	147	401	-	-	-	-	-	-	-	-	-	-	-	-	-	256	200	456
9:00 PM	211	206	417	185	105	290	180	112	292	-	-	-	-	-	-	-	-	-	-	-	-	-	192	141	333
10:00 PM	116	120	236	114	64	178	122	98	220	-	-	-	-	-	-	-	-	-	-	-	-	-	117	94	211
11:00 PM	77	54	131	71	39	110	79	45	124	-	-	-	-	-	-	-	-	-	-	-	-	-	76	46	122
Total	6,484	7,277	13,761	6,286	7,228	13,514	6,452	7,428	13,880	-	-	-	-	-	-	-	-	-	-	-	-	-	6,407	7,311	13,718
Percent	47%	53%		47%	53%		46%	54%		-	-	-	-	-	-	-	-	-	-	-	-	-	47%	53%	
AM Peak	07:00	07:00	07:00	08:00	07:00	07:00	07:00	07:00	07:00	-	-	-	-	-	-	-	-	-	-	-	-	-	07:00	07:00	07:00
Vol.	437	836	1,273	403	750	1,152	451	745	1,196	-	-	-	-	-	-	-	-	-	-	-	-	-	430	777	1,207
PM Peak	16:00	16:00	16:00	16:00	16:00	16:00	17:00	16:00	16:00	-	-	-	-	-	-	-	-	-	-	-	-	-	16:00	16:00	16:00
Vol.	708	575	1,283	612	655	1,267	588	737	1,303	-	-	-	-	-	-	-	-	-	-	-	-	-	629	656	1,284

1. Mid-week average includes data between Tuesday and Thursday.

Vehicle Classification Report Summary

Location: Gun Club Rd S/O Jewell Ave

Count Direction: Northbound / Southbound

Date Range: 10/31/2023 to 11/2/2023

Site Code: 02

Direction	FHWA Vehicle Classification													Total Volume
	1	2	3	4	5	6	7	8	9	10	11	12	13	
Northbound	98	26,237	6,401	155	1,216	376	0	9	420	19	0	1	40	34,972
	0.3%	75.0%	18.3%	0.4%	3.5%	1.1%	0.0%	0.0%	1.2%	0.1%	0.0%	0.0%	0.1%	
Southbound	104	24,710	5,380	115	957	329	0	11	289	35	0	0	27	31,957
	0.3%	77.3%	16.8%	0.4%	3.0%	1.0%	0.0%	0.0%	0.9%	0.1%	0.0%	0.0%	0.1%	
Total	202	50,947	11,781	270	2,173	705	0	20	709	54	0	1	67	66,929
	0.3%	76.1%	17.6%	0.4%	3.2%	1.1%	0.0%	0.0%	1.1%	0.1%	0.0%	0.0%	0.1%	

FHWA Vehicle Classification													
Class 1 - Motorcycles							Class 8 - Four or Fewer Axle Single-Trailer Trucks						
Class 2 - Passenger Cars							Class 9 - Five-Axle Single-Trailer Trucks						
Class 3 - Other Two-Axle, Four-Tire Single Unit Vehicles							Class 10 - Six or More Axle Single-Trailer Trucks						
Class 4 - Buses							Class 11 - Five or fewer Axle Multi-Trailer Trucks						
Class 5 - Two-Axle, Six-Tire, Single-Unit Trucks							Class 12 - Six-Axle Multi-Trailer Trucks						
Class 6 - Three-Axle Single-Unit Trucks							Class 13 - Seven or More Axle Multi-Trailer Trucks						
Class 7 - Four or More Axle Single-Unit Trucks													

Vehicle Speed Report Summary



Location: Gun Club Rd S/O Jewell Ave

Direction: Northbound / Southbound

Date Range: 10/31/2023 to 11/2/2023

Site Code: 02

Direction	Speed Range (mph)																	Total Volume
	0 - 10	10 - 15	15 - 20	20 - 25	25 - 30	30 - 35	35 - 40	40 - 45	45 - 50	50 - 55	55 - 60	60 - 65	65 - 70	70 - 75	75 - 80	80 - 85	85 +	
Northbound	15	54	126	294	257	331	1,002	4,315	9,965	11,335	5,567	1,347	247	117	0	0	0	34,972
	0.0%	0.2%	0.4%	0.8%	0.7%	0.9%	2.9%	12.3%	28.5%	32.4%	15.9%	3.9%	0.7%	0.3%	0.0%	0.0%	0.0%	
Southbound	210	161	101	114	223	791	2,298	6,041	9,557	8,150	3,337	752	147	75	0	0	0	31,957
	0.7%	0.5%	0.3%	0.4%	0.7%	2.5%	7.2%	18.9%	29.9%	25.5%	10.4%	2.4%	0.5%	0.2%	0.0%	0.0%	0.0%	
Total	225	215	227	408	480	1,122	3,300	10,356	19,522	19,485	8,904	2,099	394	192	0	0	0	66,929
	0.3%	0.3%	0.3%	0.6%	0.7%	1.7%	4.9%	15.5%	29.2%	29.1%	13.3%	3.1%	0.6%	0.3%	0.0%	0.0%	0.0%	

Total Study Percentile Speed Summary		Total Study Speed Statistics	
Northbound		Northbound	
50th Percentile (Median)	50.4 mph	Mean (Average) Speed	50.0 mph
85th Percentile	56.3 mph	10 mph Pace	45.7 - 55.7 mph
95th Percentile	59.9 mph	Percent in Pace	61.2 %
Southbound		Southbound	
50th Percentile (Median)	48.2 mph	Mean (Average) Speed	47.5 mph
85th Percentile	54.6 mph	10 mph Pace	44.1 - 54.1 mph
95th Percentile	58.5 mph	Percent in Pace	56.2 %

Location: Gun Club Rd S/O Jewell Ave
 Date Range: 10/31/2023 - 11/6/2023
 Site Code: 02

Time	Tuesday 10/31/2023			Wednesday 11/1/2023			Thursday 11/2/2023			Friday 11/3/2023			Saturday 11/4/2023			Sunday 11/5/2023			Monday 11/6/2023			Mid-Week Average			
	NB	SB	Total	NB	SB	Total	NB	SB	Total	NB	SB	Total	NB	SB	Total	NB	SB	Total	NB	SB	Total	NB	SB	Total	
12:00 AM	33	53	86	57	75	132	40	64	104	-	-	-	-	-	-	-	-	-	-	-	-	-	43	64	107
1:00 AM	29	44	73	39	42	81	36	35	71	-	-	-	-	-	-	-	-	-	-	-	-	-	35	40	75
2:00 AM	59	35	94	64	39	103	51	31	82	-	-	-	-	-	-	-	-	-	-	-	-	-	58	35	93
3:00 AM	101	52	153	97	46	143	91	42	133	-	-	-	-	-	-	-	-	-	-	-	-	-	96	47	143
4:00 AM	207	81	288	215	82	297	218	77	295	-	-	-	-	-	-	-	-	-	-	-	-	-	213	80	293
5:00 AM	478	184	662	478	183	661	512	217	729	-	-	-	-	-	-	-	-	-	-	-	-	-	489	195	684
6:00 AM	792	448	1,240	825	464	1,289	790	474	1,264	-	-	-	-	-	-	-	-	-	-	-	-	-	802	462	1,264
7:00 AM	1,101	700	1,801	799	666	1,465	1,118	726	1,844	-	-	-	-	-	-	-	-	-	-	-	-	-	1,006	697	1,703
8:00 AM	965	725	1,690	418	568	986	881	664	1,545	-	-	-	-	-	-	-	-	-	-	-	-	-	755	652	1,407
9:00 AM	523	492	1,015	601	464	1,065	547	500	1,047	-	-	-	-	-	-	-	-	-	-	-	-	-	557	485	1,042
10:00 AM	462	498	960	489	446	935	461	469	930	-	-	-	-	-	-	-	-	-	-	-	-	-	471	471	942
11:00 AM	521	539	1,060	473	505	978	521	539	1,060	-	-	-	-	-	-	-	-	-	-	-	-	-	505	528	1,033
12:00 PM	557	519	1,076	562	520	1,082	547	549	1,096	-	-	-	-	-	-	-	-	-	-	-	-	-	555	529	1,085
1:00 PM	567	623	1,190	555	581	1,136	588	546	1,134	-	-	-	-	-	-	-	-	-	-	-	-	-	570	583	1,153
2:00 PM	605	790	1,395	660	700	1,360	641	820	1,461	-	-	-	-	-	-	-	-	-	-	-	-	-	635	770	1,405
3:00 PM	874	964	1,838	724	974	1,698	782	995	1,777	-	-	-	-	-	-	-	-	-	-	-	-	-	793	978	1,771
4:00 PM	965	994	1,959	942	1,005	1,947	969	1,020	1,989	-	-	-	-	-	-	-	-	-	-	-	-	-	959	1,006	1,965
5:00 PM	929	995	1,924	902	1,027	1,929	955	926	1,881	-	-	-	-	-	-	-	-	-	-	-	-	-	929	983	1,911
6:00 PM	655	683	1,338	791	720	1,511	754	760	1,514	-	-	-	-	-	-	-	-	-	-	-	-	-	733	721	1,454
7:00 PM	485	395	880	503	423	926	542	474	1,016	-	-	-	-	-	-	-	-	-	-	-	-	-	510	431	941
8:00 PM	484	375	859	386	318	704	394	313	707	-	-	-	-	-	-	-	-	-	-	-	-	-	421	335	757
9:00 PM	297	286	583	246	225	471	237	225	462	-	-	-	-	-	-	-	-	-	-	-	-	-	260	245	505
10:00 PM	180	190	370	145	183	328	166	191	357	-	-	-	-	-	-	-	-	-	-	-	-	-	164	188	352
11:00 PM	108	127	235	81	116	197	102	136	238	-	-	-	-	-	-	-	-	-	-	-	-	-	97	126	223
Total	11,977	10,792	22,769	11,052	10,372	21,424	11,943	10,793	22,736	-	-	-	-	-	-	-	-	-	-	-	-	-	11,657	10,652	22,310
Percent	53%	47%		52%	48%		53%	47%		-	-	-	-	-	-	-	-	-	-	-	-	-	52%	48%	
AM Peak	07:00	08:00	07:00	06:00	07:00	07:00	07:00	07:00	07:00	-	-	-	-	-	-	-	-	-	-	-	-	-	07:00	07:00	07:00
Vol.	1,101	725	1,801	825	666	1,465	1,118	726	1,844	-	-	-	-	-	-	-	-	-	-	-	-	-	1,006	697	1,703
PM Peak	16:00	17:00	16:00	16:00	17:00	16:00	16:00	16:00	16:00	-	-	-	-	-	-	-	-	-	-	-	-	-	16:00	16:00	16:00
Vol.	965	995	1,959	942	1,027	1,947	969	1,020	1,989	-	-	-	-	-	-	-	-	-	-	-	-	-	959	1,006	1,965

1. Mid-week average includes data between Tuesday and Thursday.

***Background
Documentation***

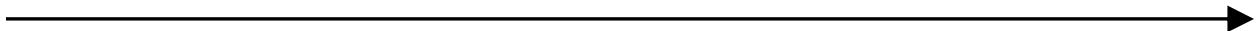
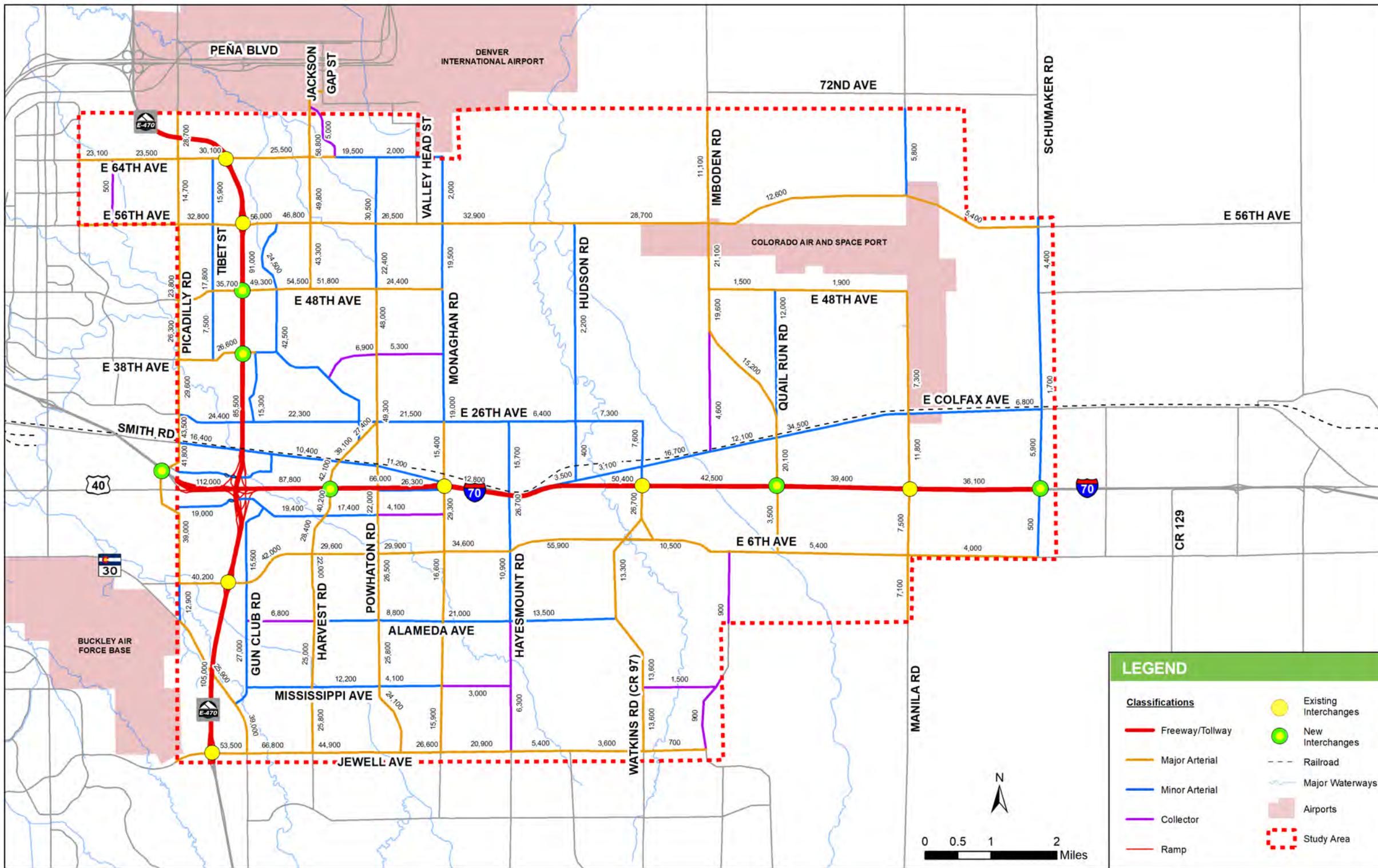
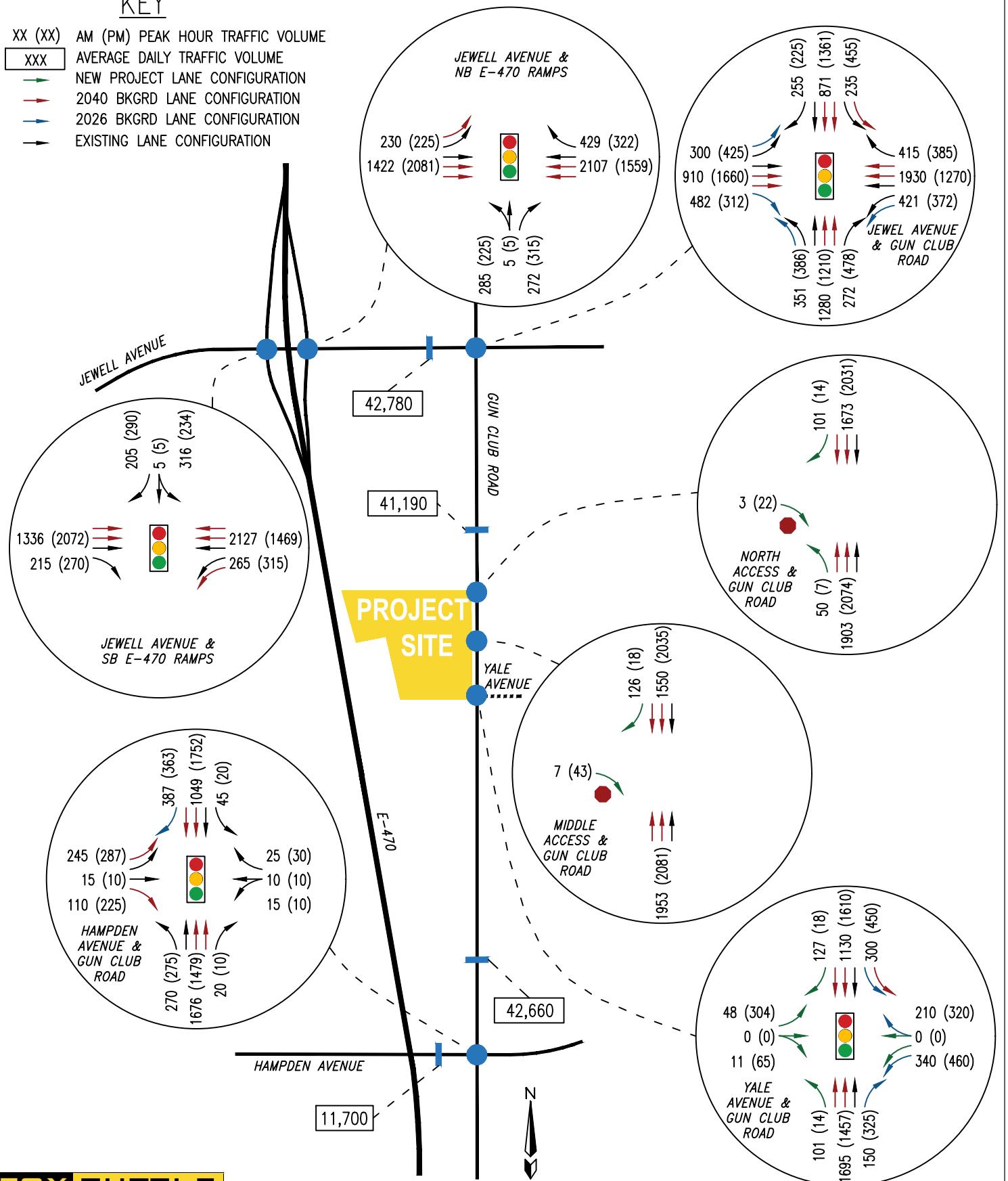


Figure 10.
Buildout Daily Traffic Volumes



KEY

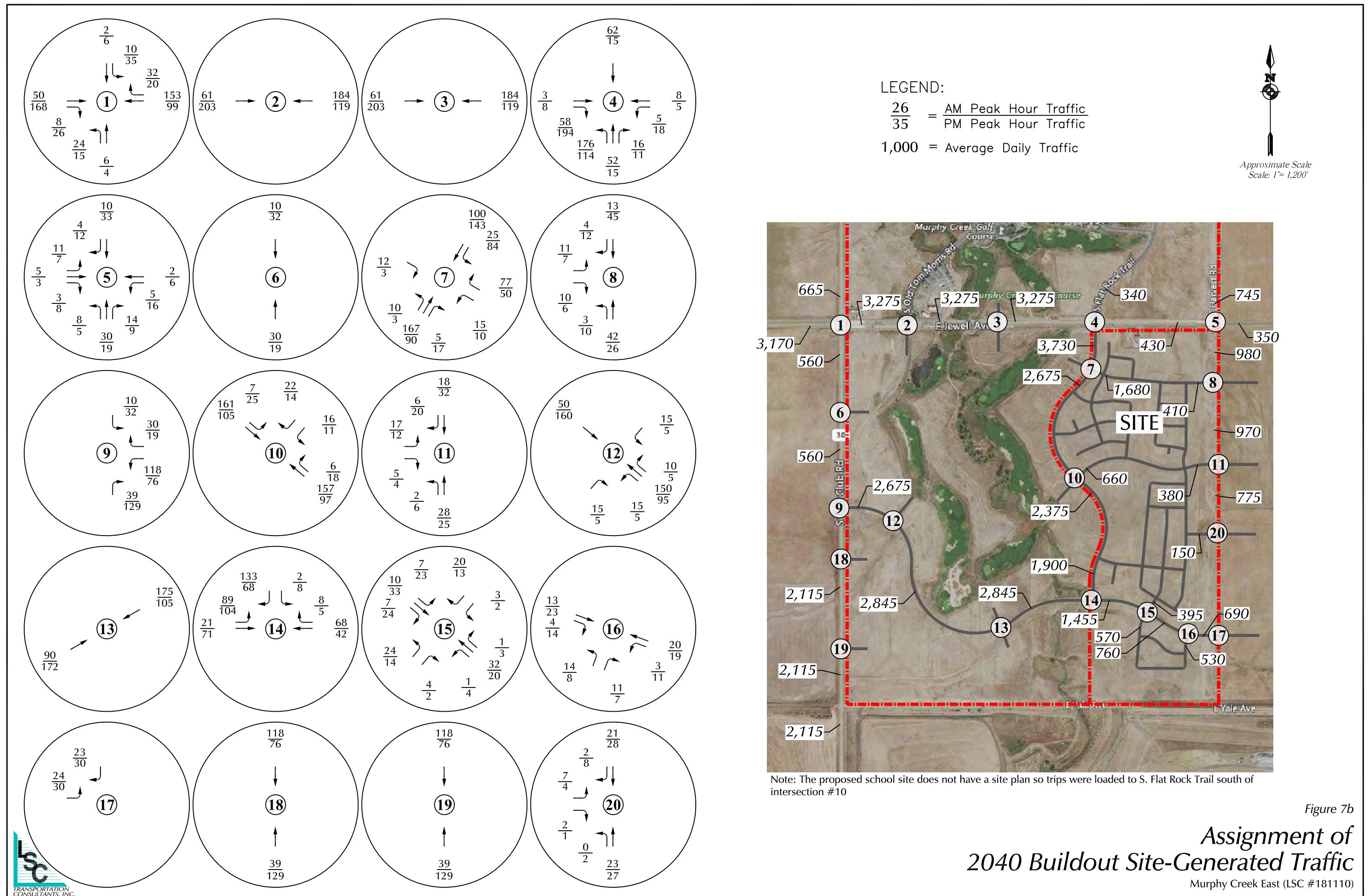
- XX (XX) AM (PM) PEAK HOUR TRAFFIC VOLUME
- XXX AVERAGE DAILY TRAFFIC VOLUME
- NEW PROJECT LANE CONFIGURATION
- 2040 BKGRD LANE CONFIGURATION
- 2026 BKGRD LANE CONFIGURATION
- EXISTING LANE CONFIGURATION



FOX TUTTLE
TRANSPORTATION GROUP

ASPEN BUSINESS INDUSTRIAL PARK TRAFFIC IMPACT STUDY - AURORA, CO
YEAR 2040 BACKGROUND + PROJECT TRAFFIC VOLUMES

FT #	22001	Original Scale	NTS	Date	7/7/2022	Drawn by	MAR	Figure #	9
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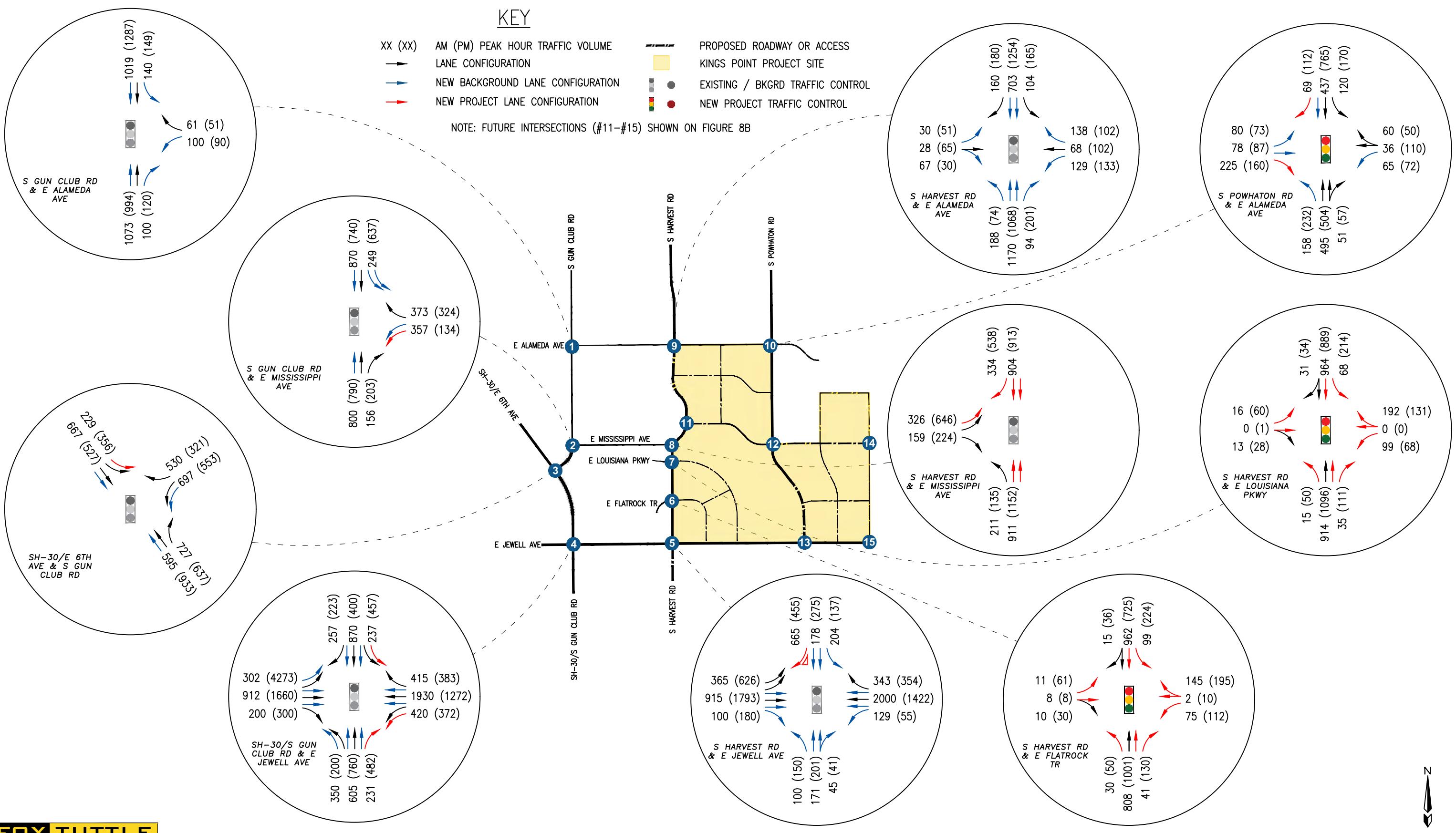


KEY

- XX (XX) AM (PM) PEAK HOUR TRAFFIC VOLUME
- LANE CONFIGURATION
- NEW BACKGROUND LANE CONFIGURATION
- NEW PROJECT LANE CONFIGURATION

- PROPOSED ROADWAY OR ACCESS
- KING'S POINT PROJECT SITE
- EXISTING / BKGRD TRAFFIC CONTROL
- NEW PROJECT TRAFFIC CONTROL

NOTE: FUTURE INTERSECTIONS (#11–#15) SHOWN ON FIGURE 8B



Intersection Capacity Worksheets:
2023 Existing

Intersection												
Int Delay, s/veh	5.5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↑	↗	↖	↑					↑	↗	
Traffic Vol, veh/h	0	537	160	112	698	0	0	0	0	38	1	168
Future Vol, veh/h	0	537	160	112	698	0	0	0	0	38	1	168
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	300	250	-	-	-	-	-	-	-	0
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	95	95	95	89	89	89	92	92	92	87	87	87
Heavy Vehicles, %	3	3	3	1	1	1	2	2	2	17	4	17
Mvmt Flow	0	565	168	126	784	0	0	0	0	44	1	193
Major/Minor	Major1	Major2					Minor2					
Conflicting Flow All	-	0	0	733	0	0				1685	1769	784
Stage 1	-	-	-	-	-	-				1036	1036	-
Stage 2	-	-	-	-	-	-				649	733	-
Critical Hdwy	-	-	-	4.11	-	-				6.57	6.54	6.37
Critical Hdwy Stg 1	-	-	-	-	-	-				5.57	5.54	-
Critical Hdwy Stg 2	-	-	-	-	-	-				5.57	5.54	-
Follow-up Hdwy	-	-	-	2.209	-	-				3.653	4.036	3.453
Pot Cap-1 Maneuver	0	-	-	876	-	0				95	82	370
Stage 1	0	-	-	-	-	0				321	306	-
Stage 2	0	-	-	-	-	0				493	423	-
Platoon blocked, %	-	-	-	-	-	-						
Mov Cap-1 Maneuver	-	-	-	876	-	-				81	0	370
Mov Cap-2 Maneuver	-	-	-	-	-	-				81	0	-
Stage 1	-	-	-	-	-	-				321	0	-
Stage 2	-	-	-	-	-	-				422	0	-
Approach	EB	WB					SB					
HCM Control Delay, s	0		1.4					38				
HCM LOS								E				
Minor Lane/Major Mvmt	EBT	EBR	WBL	WBT	SBLn1	SBLn2						
Capacity (veh/h)	-	-	876	-	81	370						
HCM Lane V/C Ratio	-	-	0.144	-	0.553	0.522						
HCM Control Delay (s)	-	-	9.8	-	94.4	24.9						
HCM Lane LOS	-	-	A	-	F	C						
HCM 95th %tile Q(veh)	-	-	0.5	-	2.4	2.9						

Intersection

Int Delay, s/veh 81.1

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖	↑		↑	↗	↖	↖	↖	↗			
Traffic Vol, veh/h	198	377	0	0	636	49	174	0	44	0	0	0
Future Vol, veh/h	198	377	0	0	636	49	174	0	44	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	250	-	-	-	-	250	-	-	175	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	95	95	95	88	88	88	87	87	87	92	92	92
Heavy Vehicles, %	5	5	5	3	3	3	3	3	3	2	2	2
Mvmt Flow	208	397	0	0	723	56	200	0	51	0	0	0

Major/Minor	Major1	Major2		Minor1		
Conflicting Flow All	779	0	-	-	0	1564 1592 397
Stage 1	-	-	-	-	-	813 813 -
Stage 2	-	-	-	-	-	751 779 -
Critical Hdwy	4.15	-	-	-	-	6.43 6.53 6.23
Critical Hdwy Stg 1	-	-	-	-	-	5.43 5.53 -
Critical Hdwy Stg 2	-	-	-	-	-	5.43 5.53 -
Follow-up Hdwy	2.245	-	-	-	-	3.527 4.027 3.327
Pot Cap-1 Maneuver	825	-	0 0	-	-	~122 107 650
Stage 1	-	-	0 0	-	-	434 390 -
Stage 2	-	-	0 0	-	-	464 405 -
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	825	-	-	-	-	~91 0 650
Mov Cap-2 Maneuver	-	-	-	-	-	~91 0 -
Stage 1	-	-	-	-	-	325 0 -
Stage 2	-	-	-	-	-	464 0 -

Approach	EB	WB	NB
HCM Control Delay, s	3.7	0	\$ 519.8
HCM LOS		F	
Minor Lane/Major Mvmt	NBLn1 NBLn2	EBL EBT	WBT WBR
Capacity (veh/h)	91 650	825	- -
HCM Lane V/C Ratio	2.198 0.078	0.253	- -
HCM Control Delay (s)	\$ 648.4 11	10.8	- -
HCM Lane LOS	F B	B	- -
HCM 95th %tile Q(veh)	17.8 0.3	1	- -

Notes

~: Volume exceeds capacity \$: Delay exceeds 300s +: Computation Not Defined *: All major volume in platoon

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑
Traffic Volume (vph)	131	218	72	199	341	40	193	515	80	34	430	151
Future Volume (vph)	131	218	72	199	341	40	193	515	80	34	430	151
Turn Type	pm+pt	NA	Perm									
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases			4		8		2		2	6		6
Detector Phase	7	4	4	3	8	8	5	2	2	1	6	6
Switch Phase												
Minimum Initial (s)	3.0	4.0	4.0	3.0	5.0	5.0	4.0	8.0	8.0	4.0	8.0	8.0
Minimum Split (s)	9.5	32.0	32.0	9.5	32.0	32.0	9.5	40.0	40.0	9.5	29.0	29.0
Total Split (s)	19.0	18.0	18.0	19.0	18.0	18.0	23.0	65.0	65.0	18.0	60.0	60.0
Total Split (%)	15.8%	15.0%	15.0%	15.8%	15.0%	15.0%	19.2%	54.2%	54.2%	15.0%	50.0%	50.0%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	4.0	4.0	3.0	4.0	4.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	6.0	6.0	5.0	6.0	6.0
Lead/Lag	Lead	Lag	Lag									
Lead-Lag Optimize?	Yes											
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max						

Intersection Summary

Cycle Length: 120

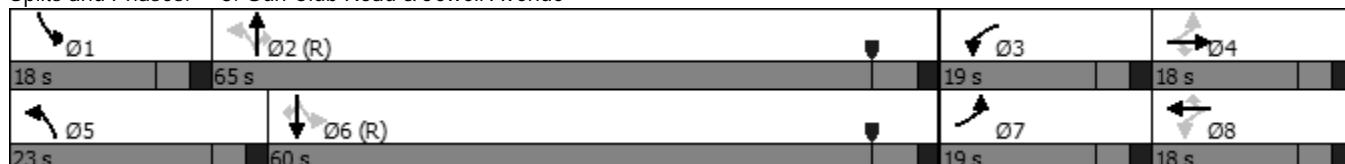
Actuated Cycle Length: 120

Offset: 108.5 (90%), Referenced to phase 2:NBTL and 6:SBTL, Start of Yellow

Natural Cycle: 95

Control Type: Actuated-Coordinated

Splits and Phases: 3: Gun Club Road & Jewell Avenue





Lane Group	EBL	EBT	EBC	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	138	229	76	229	392	46	297	792	123	39	494	174
v/c Ratio	0.68	1.13	0.27	0.89	1.50	0.14	0.58	0.73	0.13	0.14	0.56	0.21
Control Delay	52.7	150.4	4.9	72.2	282.0	0.9	13.6	24.7	2.7	9.6	25.6	3.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	52.7	150.4	4.9	72.2	282.0	0.9	13.6	24.7	2.7	9.6	25.6	3.4
Queue Length 50th (ft)	86	~213	0	150	~424	0	89	463	0	10	263	0
Queue Length 95th (ft)	144	#374	16	#267	#637	0	88	369	8	22	376	36
Internal Link Dist (ft)		2523			1479			2542			790	
Turn Bay Length (ft)	220		330	300			485		360	200		325
Base Capacity (vph)	258	203	277	265	261	323	555	1083	972	374	883	840
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.53	1.13	0.27	0.86	1.50	0.14	0.54	0.73	0.13	0.10	0.56	0.21

Intersection Summary

- ~ Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

HCM 6th Signalized Intersection Summary
11/22/2023

3: Gun Club Road & Jewell Avenue
2023 Existing - AM Peak Hour

Movement	EBL	EBT	EBC	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑
Traffic Volume (veh/h)	131	218	72	199	341	40	193	515	80	34	430	151
Future Volume (veh/h)	131	218	72	199	341	40	193	515	80	34	430	151
Initial Q (Q _b), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No		No		No		No		No		No	
Adj Sat Flow, veh/h/ln	1811	1811	1811	1870	1870	1870	1870	1870	1870	1841	1841	1841
Adj Flow Rate, veh/h	138	229	76	229	392	46	297	792	123	39	494	174
Peak Hour Factor	0.95	0.95	0.95	0.87	0.87	0.87	0.65	0.65	0.65	0.87	0.87	0.87
Percent Heavy Veh, %	6	6	6	2	2	2	2	2	2	4	4	4
Cap, veh/h	208	196	166	268	260	220	479	1077	913	261	925	784
Arrive On Green	0.09	0.11	0.11	0.12	0.14	0.14	0.10	0.58	0.58	0.02	0.50	0.50
Sat Flow, veh/h	1725	1811	1535	1781	1870	1585	1781	1870	1585	1753	1841	1560
Grp Volume(v), veh/h	138	229	76	229	392	46	297	792	123	39	494	174
Grp Sat Flow(s), veh/h/ln	1725	1811	1535	1781	1870	1585	1781	1870	1585	1753	1841	1560
Q Serve(g_s), s	8.4	13.0	5.6	13.6	16.7	3.1	9.2	37.4	4.3	1.3	21.9	7.5
Cycle Q Clear(g_c), s	8.4	13.0	5.6	13.6	16.7	3.1	9.2	37.4	4.3	1.3	21.9	7.5
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	208	196	166	268	260	220	479	1077	913	261	925	784
V/C Ratio(X)	0.66	1.17	0.46	0.86	1.51	0.21	0.62	0.74	0.13	0.15	0.53	0.22
Avail Cap(c_a), veh/h	261	196	166	268	260	220	573	1077	913	409	925	784
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	43.0	53.5	50.2	41.5	51.7	45.8	14.5	18.7	11.7	17.4	20.3	16.7
Incr Delay (d2), s/veh	2.2	116.6	2.0	21.8	247.3	0.5	1.5	4.5	0.3	0.3	2.2	0.7
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	3.6	12.1	2.2	7.4	25.5	1.2	3.3	15.3	1.5	0.5	9.1	2.7
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	45.3	170.1	52.1	63.3	299.0	46.3	15.9	23.2	12.0	17.6	22.5	17.4
LnGrp LOS	D	F	D	E	F	D	B	C	B	B	C	B
Approach Vol, veh/h		443			667			1212		707		
Approach Delay, s/veh		111.0			200.6			20.3		21.0		
Approach LOS		F			F			C		C		
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	7.9	75.1	19.0	18.0	16.7	66.3	15.3	21.7				
Change Period (Y+Rc), s	5.0	6.0	5.0	5.0	5.0	6.0	5.0	5.0				
Max Green Setting (Gmax), s	13.0	59.0	14.0	13.0	18.0	54.0	14.0	13.0				
Max Q Clear Time (g_c+l1), s	3.3	39.4	15.6	15.0	11.2	23.9	10.4	18.7				
Green Ext Time (p_c), s	0.0	11.8	0.0	0.0	0.5	9.7	0.0	0.0				

Intersection Summary

HCM 6th Ctrl Delay 73.4
HCM 6th LOS E

Notes

User approved pedestrian interval to be less than phase max green.

Intersection												
Int Delay, s/veh	4.9											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↑	↗	↖	↑					↑	↗	
Traffic Vol, veh/h	0	679	283	93	574	0	0	0	0	34	3	231
Future Vol, veh/h	0	679	283	93	574	0	0	0	0	34	3	231
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	300	250	-	-	-	-	-	-	-	0
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	95	95	95	96	96	96	92	92	92	86	86	86
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	6	6	6
Mvmt Flow	0	715	298	97	598	0	0	0	0	40	3	269
Major/Minor	Major1	Major2					Minor2					
Conflicting Flow All	-	0	0	1013	0	0				1656	1805	598
Stage 1	-	-	-	-	-	-				792	792	-
Stage 2	-	-	-	-	-	-				864	1013	-
Critical Hdwy	-	-	-	4.12	-	-				6.46	6.56	6.26
Critical Hdwy Stg 1	-	-	-	-	-	-				5.46	5.56	-
Critical Hdwy Stg 2	-	-	-	-	-	-				5.46	5.56	-
Follow-up Hdwy	-	-	-	2.218	-	-				3.554	4.054	3.354
Pot Cap-1 Maneuver	0	-	-	684	-	0				105	77	495
Stage 1	0	-	-	-	-	0				439	395	-
Stage 2	0	-	-	-	-	0				406	311	-
Platoon blocked, %	-	-	-	-	-	-						
Mov Cap-1 Maneuver	-	-	-	684	-	-				90	0	495
Mov Cap-2 Maneuver	-	-	-	-	-	-				90	0	-
Stage 1	-	-	-	-	-	-				439	0	-
Stage 2	-	-	-	-	-	-				348	0	-
Approach	EB	WB					SB					
HCM Control Delay, s	0		1.6					28.4				
HCM LOS								D				
Minor Lane/Major Mvmt	EBT	EBR	WBL	WBT	SBLn1	SBLn2						
Capacity (veh/h)	-	-	684	-	90	495						
HCM Lane V/C Ratio	-	-	0.142	-	0.478	0.543						
HCM Control Delay (s)	-	-	11.1	-	77.2	20.6						
HCM Lane LOS	-	-	B	-	F	C						
HCM 95th %tile Q(veh)	-	-	0.5	-	2	3.2						

Intersection

Int Delay, s/veh 31

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖	↑		↑	↗	↖	↖	↖	↗			
Traffic Vol, veh/h	172	541	0	0	559	74	108	0	59	0	0	0
Future Vol, veh/h	172	541	0	0	559	74	108	0	59	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	250	-	-	-	-	250	-	-	175	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	93	93	93	90	90	90	80	80	80	92	92	92
Heavy Vehicles, %	4	4	4	7	7	7	5	5	5	2	2	2
Mvmt Flow	185	582	0	0	621	82	135	0	74	0	0	0

Major/Minor	Major1	Major2		Minor1		
Conflicting Flow All	703	0	-	-	0	1614 1655 582
Stage 1	-	-	-	-	-	952 952 -
Stage 2	-	-	-	-	-	662 703 -
Critical Hdwy	4.14	-	-	-	-	6.45 6.55 6.25
Critical Hdwy Stg 1	-	-	-	-	-	5.45 5.55 -
Critical Hdwy Stg 2	-	-	-	-	-	5.45 5.55 -
Follow-up Hdwy	2.236	-	-	-	-	3.545 4.045 3.345
Pot Cap-1 Maneuver	885	-	0 0	-	-	~112 97 507
Stage 1	-	-	0 0	-	-	370 334 -
Stage 2	-	-	0 0	-	-	507 436 -
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	885	-	-	-	-	~89 0 507
Mov Cap-2 Maneuver	-	-	-	-	-	~89 0 -
Stage 1	-	-	-	-	-	293 0 -
Stage 2	-	-	-	-	-	507 0 -

Approach	EB	WB	NB
HCM Control Delay, s	2.4	0	240.5
HCM LOS		F	
<hr/>			
Minor Lane/Major Mvmt	NBLn1 NBLn2	EBL EBT WBT	WBR
Capacity (veh/h)	89 507	885 - -	-
HCM Lane V/C Ratio	1.517 0.145	0.209 - -	-
HCM Control Delay (s)	\$ 364.6 13.3	10.1 - -	-
HCM Lane LOS	F B B	- - -	-
HCM 95th %tile Q(veh)	10.5 0.5	0.8 - -	-

Notes

~: Volume exceeds capacity \$: Delay exceeds 300s +: Computation Not Defined *: All major volume in platoon

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑
Traffic Volume (vph)	158	296	146	184	313	34	156	526	240	15	687	164
Future Volume (vph)	158	296	146	184	313	34	156	526	240	15	687	164
Turn Type	pm+pt	NA	Perm									
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases			4		8		8	2		2	6	
Detector Phase	7	4	4	3	8	8	5	2	2	1	6	6
Switch Phase												
Minimum Initial (s)	3.0	4.0	4.0	3.0	5.0	5.0	4.0	8.0	8.0	4.0	8.0	8.0
Minimum Split (s)	9.5	32.0	32.0	9.5	32.0	32.0	9.5	40.0	40.0	9.5	29.0	29.0
Total Split (s)	21.0	27.0	27.0	14.0	20.0	20.0	12.0	67.0	67.0	12.0	67.0	67.0
Total Split (%)	17.5%	22.5%	22.5%	11.7%	16.7%	16.7%	10.0%	55.8%	55.8%	10.0%	55.8%	55.8%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	4.0	4.0	3.0	4.0	4.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	6.0	6.0	5.0	6.0	6.0
Lead/Lag	Lead	Lag	Lag									
Lead-Lag Optimize?	Yes											
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max						

Intersection Summary

Cycle Length: 120

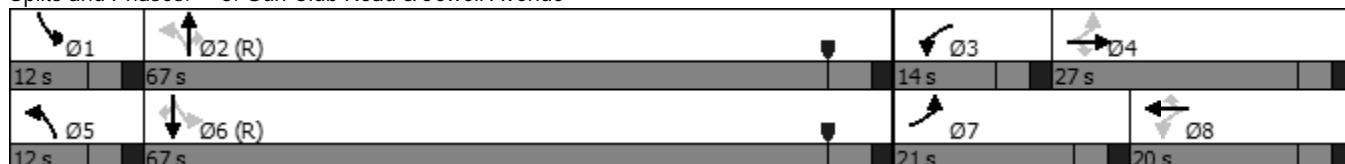
Actuated Cycle Length: 120

Offset: 108.5 (90%), Referenced to phase 2:NBTL and 6:SBTL, Start of Yellow

Natural Cycle: 95

Control Type: Actuated-Coordinated

Splits and Phases: 3: Gun Club Road & Jewell Avenue





Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	168	315	155	190	323	35	173	584	267	16	723	173
v/c Ratio	0.74	0.94	0.38	0.99	1.09	0.09	0.68	0.58	0.27	0.04	0.76	0.19
Control Delay	52.8	85.6	9.3	102.8	126.4	0.5	25.2	20.5	2.6	9.3	29.9	2.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	52.8	85.6	9.3	102.8	126.4	0.5	25.2	20.5	2.6	9.3	29.9	2.8
Queue Length 50th (ft)	101	243	0	116	-284	0	54	248	0	5	433	0
Queue Length 95th (ft)	163	#420	58	#273	#520	0	#98	445	42	13	595	35
Internal Link Dist (ft)		2523			1479			2542			790	
Turn Bay Length (ft)	220		330	300			485		360	200		325
Base Capacity (vph)	286	334	411	191	296	389	256	1012	975	424	956	897
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.59	0.94	0.38	0.99	1.09	0.09	0.68	0.58	0.27	0.04	0.76	0.19

Intersection Summary

- ~ Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

HCM 6th Signalized Intersection Summary
11/22/2023

3: Gun Club Road & Jewell Avenue
2023 Existing - PM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑
Traffic Volume (veh/h)	158	296	146	184	313	34	156	526	240	15	687	164
Future Volume (veh/h)	158	296	146	184	313	34	156	526	240	15	687	164
Initial Q (Q _b), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No		No		No		No		No		No	
Adj Sat Flow, veh/h/ln	1841	1841	1841	1841	1841	1841	1796	1796	1796	1885	1885	1885
Adj Flow Rate, veh/h	168	315	155	190	323	35	173	584	267	16	723	173
Peak Hour Factor	0.94	0.94	0.94	0.97	0.97	0.97	0.90	0.90	0.90	0.95	0.95	0.95
Percent Heavy Veh, %	4	4	4	4	4	4	7	7	7	1	1	1
Cap, veh/h	227	337	286	205	301	255	281	993	842	305	958	812
Arrive On Green	0.10	0.18	0.18	0.08	0.16	0.16	0.06	0.55	0.55	0.01	0.51	0.51
Sat Flow, veh/h	1753	1841	1560	1753	1841	1560	1711	1796	1522	1795	1885	1598
Grp Volume(v), veh/h	168	315	155	190	323	35	173	584	267	16	723	173
Grp Sat Flow(s), veh/h/ln	1753	1841	1560	1753	1841	1560	1711	1796	1522	1795	1885	1598
Q Serve(g_s), s	9.4	20.2	10.8	9.0	19.6	2.3	5.6	25.8	11.4	0.5	36.7	7.2
Cycle Q Clear(g_c), s	9.4	20.2	10.8	9.0	19.6	2.3	5.6	25.8	11.4	0.5	36.7	7.2
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	227	337	286	205	301	255	281	993	842	305	958	812
V/C Ratio(X)	0.74	0.93	0.54	0.93	1.07	0.14	0.62	0.59	0.32	0.05	0.75	0.21
Avail Cap(c_a), veh/h	294	337	286	205	301	255	281	993	842	385	958	812
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	38.1	48.3	44.4	43.3	50.2	43.0	20.4	17.8	14.5	15.6	23.5	16.3
Incr Delay (d2), s/veh	4.6	32.3	2.1	42.4	73.2	0.2	4.0	2.6	1.0	0.1	5.5	0.6
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	4.2	12.0	4.2	3.7	14.9	0.9	2.2	10.1	3.9	0.2	16.0	2.6
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	42.7	80.5	46.5	85.6	123.4	43.2	24.4	20.3	15.5	15.7	29.0	16.9
LnGrp LOS	D	F	D	F	F	D	C	C	B	B	C	B
Approach Vol, veh/h		638			548			1024			912	
Approach Delay, s/veh		62.3			105.2			19.8			26.5	
Approach LOS		E			F			B			C	

Intersection Summary

HCM 6th Ctrl Delay 45.4

HCM 6th LOS D

Notes

User approved pedestrian interval to be less than phase max green.

***Intersection Capacity Worksheets:
2023 Existing
with Improvements***





Lane Group	EBT	EBR	WBL	WBT	SBT	SBR
Lane Configurations	↑	↗	↖	↑	↖	↗
Traffic Volume (vph)	537	160	112	698	1	168
Future Volume (vph)	537	160	112	698	1	168
Turn Type	NA	Perm	pm+pt	NA	NA	Perm
Protected Phases	4			3	8	6
Permitted Phases				4	8	6
Detector Phase	4	4	3	8	6	6
Switch Phase						
Minimum Initial (s)	8.0	8.0	4.0	8.0	8.0	8.0
Minimum Split (s)	25.0	25.0	10.0	25.0	27.0	27.0
Total Split (s)	50.0	50.0	13.0	63.0	27.0	27.0
Total Split (%)	55.6%	55.6%	14.4%	70.0%	30.0%	30.0%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	6.0
Lead/Lag	Lag	Lag	Lead			
Lead-Lag Optimize?	Yes	Yes	Yes			
Recall Mode	C-Max	C-Max	None	C-Max	Max	Max

Intersection Summary

Cycle Length: 90

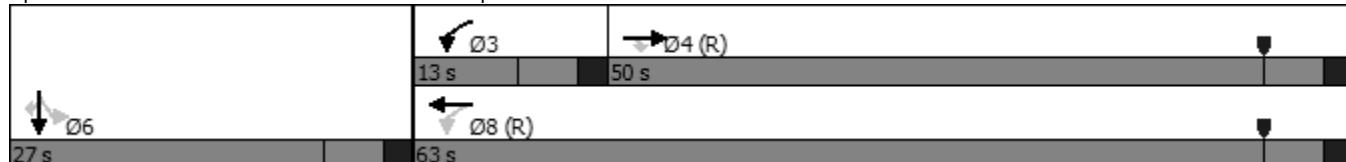
Actuated Cycle Length: 90

Offset: 68 (76%), Referenced to phase 4:EBT and 8:WBTL, Start of Yellow

Natural Cycle: 65

Control Type: Actuated-Coordinated

Splits and Phases: 1: E-470 Southbound Ramps & Jewell Avenue





Lane Group	EBT	EBR	WBL	WBT	SBT	SBR
Lane Group Flow (vph)	565	168	126	784	45	193
v/c Ratio	0.62	0.20	0.31	0.66	0.12	0.41
Control Delay	20.7	2.7	6.1	7.6	28.4	7.4
Queue Delay	0.0	0.0	0.0	0.2	0.0	0.0
Total Delay	20.7	2.7	6.1	7.8	28.4	7.4
Queue Length 50th (ft)	227	0	18	112	20	0
Queue Length 95th (ft)	336	31	m25	143	47	48
Internal Link Dist (ft)	679			530	532	
Turn Bay Length (ft)		300	250			
Base Capacity (vph)	906	855	413	1191	361	469
Starvation Cap Reductn	0	0	0	60	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.62	0.20	0.31	0.69	0.12	0.41

Intersection Summary

m Volume for 95th percentile queue is metered by upstream signal.

HCM 6th Signalized Intersection Summary
11/22/2023

1: E-470 Southbound Ramps & Jewell Avenue
2023 Existing (with Improvements) - AM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	0	537	160	112	698	0	0	0	0	38	1	168
Future Volume (veh/h)	0	537	160	112	698	0	0	0	0	38	1	168
Initial Q (Q _b), veh	0	0	0	0	0	0				0	0	0
Ped-Bike Adj(A_pbT)	1.00			1.00	1.00					1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00				1.00	1.00	1.00
Work Zone On Approach		No			No						No	
Adj Sat Flow, veh/h/ln	0	1856	1856	1885	1885	0				1648	1841	1648
Adj Flow Rate, veh/h	0	565	168	126	784	0				44	1	193
Peak Hour Factor	0.95	0.95	0.95	0.89	0.89	0.89				0.87	0.87	0.87
Percent Heavy Veh, %	0	3	3	1	1	0				17	4	17
Cap, veh/h	0	955	809	394	1194	0				400	9	326
Arrive On Green	0.00	0.51	0.51	0.10	1.00	0.00				0.23	0.23	0.23
Sat Flow, veh/h	0	1856	1572	1795	1885	0				1716	39	1397
Grp Volume(v), veh/h	0	565	168	126	784	0				45	0	193
Grp Sat Flow(s), veh/h/ln	0	1856	1572	1795	1885	0				1755	0	1397
Q Serve(g_s), s	0.0	19.1	5.2	2.9	0.0	0.0				1.8	0.0	11.1
Cycle Q Clear(g_c), s	0.0	19.1	5.2	2.9	0.0	0.0				1.8	0.0	11.1
Prop In Lane	0.00		1.00	1.00		0.00				0.98		1.00
Lane Grp Cap(c), veh/h	0	955	809	394	1194	0				409	0	326
V/C Ratio(X)	0.00	0.59	0.21	0.32	0.66	0.00				0.11	0.00	0.59
Avail Cap(c_a), veh/h	0	955	809	440	1194	0				409	0	326
HCM Platoon Ratio	1.00	1.00	1.00	2.00	2.00	1.00				1.00	1.00	1.00
Upstream Filter(l)	0.00	1.00	1.00	0.50	0.50	0.00				1.00	0.00	1.00
Uniform Delay (d), s/veh	0.0	15.3	11.9	10.5	0.0	0.0				27.1	0.0	30.7
Incr Delay (d2), s/veh	0.0	2.7	0.6	0.2	1.4	0.0				0.5	0.0	7.7
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0				0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	0.0	7.7	1.7	0.9	0.5	0.0				0.8	0.0	4.3
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	0.0	17.9	12.5	10.8	1.4	0.0				27.7	0.0	38.4
LnGrp LOS	A	B	B	B	A	A				C	A	D
Approach Vol, veh/h		733			910						238	
Approach Delay, s/veh		16.7			2.7						36.4	
Approach LOS		B			A						D	
Timer - Assigned Phs		3	4		6		8					
Phs Duration (G+Y+R _c), s		10.7	52.3		27.0		63.0					
Change Period (Y+R _c), s		6.0	6.0		6.0		6.0					
Max Green Setting (Gmax), s		7.0	44.0		21.0		57.0					
Max Q Clear Time (g _{c+l1}), s		4.9	21.1		13.1		2.0					
Green Ext Time (p _c), s		0.1	3.9		0.5		6.1					
Intersection Summary												
HCM 6th Ctrl Delay			12.4									
HCM 6th LOS			B									



Lane Group	EBL	EBT	WBT	WBR	NBT	NBR
Lane Configurations	↑ ↗	↑ ↘	↑ ↙	↗ ↖	↖ ↘	↖ ↖
Traffic Volume (vph)	198	377	636	49	0	44
Future Volume (vph)	198	377	636	49	0	44
Turn Type	pm+pt	NA	NA	Perm	NA	Perm
Protected Phases	7	4	8		2	
Permitted Phases	4			8		2
Detector Phase	7	4	8	8	2	2
Switch Phase						
Minimum Initial (s)	3.0	8.0	8.0	8.0	8.0	8.0
Minimum Split (s)	9.5	25.0	25.0	25.0	27.0	27.0
Total Split (s)	13.0	63.0	50.0	50.0	27.0	27.0
Total Split (%)	14.4%	70.0%	55.6%	55.6%	30.0%	30.0%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	6.0
Lead/Lag	Lead		Lag	Lag		
Lead-Lag Optimize?	Yes		Yes	Yes		
Recall Mode	None	C-Max	C-Max	C-Max	Max	Max

Intersection Summary

Cycle Length: 90

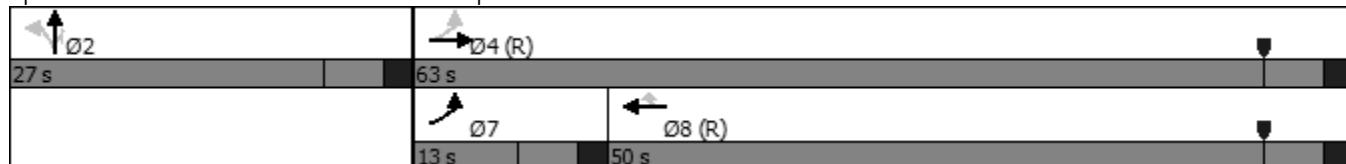
Actuated Cycle Length: 90

Offset: 60 (67%), Referenced to phase 4:EBTL and 8:WBT, Start of Yellow

Natural Cycle: 80

Control Type: Actuated-Coordinated

Splits and Phases: 2: E-470 Northbound Ramps & Jewell Avenue





Lane Group	EBL	EBT	WBT	WBR	NBT	NBR
Lane Group Flow (vph)	208	397	723	56	200	51
v/c Ratio	0.73	0.35	0.80	0.07	0.49	0.11
Control Delay	40.0	6.9	27.8	0.5	34.7	0.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	40.0	6.9	27.8	0.5	34.7	0.5
Queue Length 50th (ft)	79	60	331	0	99	0
Queue Length 95th (ft)	#175	109	471	3	160	0
Internal Link Dist (ft)		530	2523		717	
Turn Bay Length (ft)	250			250		175
Base Capacity (vph)	283	1146	902	822	408	449
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.73	0.35	0.80	0.07	0.49	0.11

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM 6th Signalized Intersection Summary
11/22/2023

2: E-470 Northbound Ramps & Jewell Avenue
2023 Existing (with Improvements) - AM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑ ↗	↑ ↘			↑ ↗	↑ ↘		↑ ↗	↑ ↘			
Traffic Volume (veh/h)	198	377	0	0	636	49	174	0	44	0	0	0
Future Volume (veh/h)	198	377	0	0	636	49	174	0	44	0	0	0
Initial Q (Q _b), veh	0	0	0	0	0	0	0	0	0			
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00			
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Work Zone On Approach		No			No			No				
Adj Sat Flow, veh/h/ln	1826	1826	0	0	1856	1856	1856	1856	1856			
Adj Flow Rate, veh/h	208	397	0	0	723	56	200	0	51			
Peak Hour Factor	0.95	0.95	0.95	0.88	0.88	0.88	0.87	0.87	0.87			
Percent Heavy Veh, %	5	5	0	0	3	3	3	3	3			
Cap, veh/h	325	1156	0	0	907	769	412	0	367			
Arrive On Green	0.16	1.00	0.00	0.00	0.49	0.49	0.23	0.00	0.23			
Sat Flow, veh/h	1739	1826	0	0	1856	1572	1767	0	1572			
Grp Volume(v), veh/h	208	397	0	0	723	56	200	0	51			
Grp Sat Flow(s), veh/h/ln	1739	1826	0	0	1856	1572	1767	0	1572			
Q Serve(g_s), s	5.3	0.0	0.0	0.0	29.4	1.7	8.8	0.0	2.3			
Cycle Q Clear(g_c), s	5.3	0.0	0.0	0.0	29.4	1.7	8.8	0.0	2.3			
Prop In Lane	1.00		0.00	0.00		1.00	1.00		1.00			
Lane Grp Cap(c), veh/h	325	1156	0	0	907	769	412	0	367			
V/C Ratio(X)	0.64	0.34	0.00	0.00	0.80	0.07	0.49	0.00	0.14			
Avail Cap(c_a), veh/h	325	1156	0	0	907	769	412	0	367			
HCM Platoon Ratio	2.00	2.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Upstream Filter(l)	0.74	0.74	0.00	0.00	0.66	0.66	1.00	0.00	1.00			
Uniform Delay (d), s/veh	14.9	0.0	0.0	0.0	19.3	12.2	29.8	0.0	27.3			
Incr Delay (d2), s/veh	3.1	0.6	0.0	0.0	4.9	0.1	4.0	0.0	0.8			
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0			
%ile BackOfQ(50%), veh/ln	1.7	0.2	0.0	0.0	12.1	0.6	4.1	0.0	0.9			
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	18.0	0.6	0.0	0.0	24.1	12.3	33.9	0.0	28.1			
LnGrp LOS	B	A	A	A	C	B	C	A	C			
Approach Vol, veh/h		605			779			251				
Approach Delay, s/veh		6.6			23.3			32.7				
Approach LOS		A			C			C				
Timer - Assigned Phs		2		4			7	8				
Phs Duration (G+Y+R _c), s		27.0		63.0			13.0	50.0				
Change Period (Y+R _c), s		6.0		6.0			6.0	6.0				
Max Green Setting (Gmax), s		21.0		57.0			7.0	44.0				
Max Q Clear Time (g_c+l1), s		10.8		2.0			7.3	31.4				
Green Ext Time (p_c), s		0.9		2.4			0.0	3.9				
Intersection Summary												
HCM 6th Ctrl Delay			18.6									
HCM 6th LOS			B									

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑
Traffic Volume (vph)	131	218	72	199	341	40	193	515	80	34	430	151
Future Volume (vph)	131	218	72	199	341	40	193	515	80	34	430	151
Turn Type	pm+pt	NA	pm+ov	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases	7	4	5	3	8		5	2		1	6	
Permitted Phases				4	8		8	2		2	6	6
Detector Phase	7	4	5	3	8	8	5	2	2	1	6	6
Switch Phase												
Minimum Initial (s)	3.0	4.0	4.0	3.0	5.0	5.0	4.0	8.0	8.0	4.0	8.0	8.0
Minimum Split (s)	9.5	32.0	9.5	9.5	32.0	32.0	9.5	40.0	40.0	9.5	29.0	29.0
Total Split (s)	13.0	28.0	23.0	21.0	36.0	36.0	23.0	53.0	53.0	18.0	48.0	48.0
Total Split (%)	10.8%	23.3%	19.2%	17.5%	30.0%	30.0%	19.2%	44.2%	44.2%	15.0%	40.0%	40.0%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	4.0	4.0	3.0	4.0	4.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	6.0	6.0	5.0	6.0	6.0
Lead/Lag	Lead	Lag	Lead	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes											
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max						

Intersection Summary

Cycle Length: 120

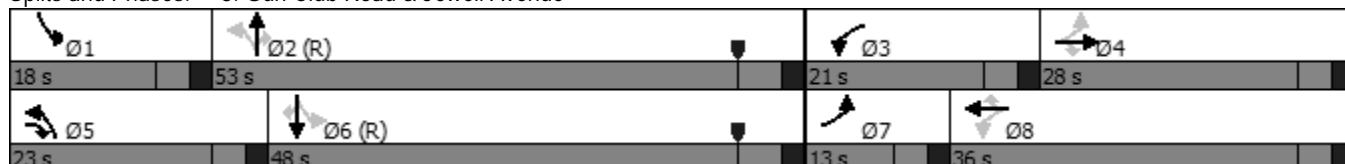
Actuated Cycle Length: 120

Offset: 108.5 (90%), Referenced to phase 2:NBTL and 6:SBTL, Start of Yellow

Natural Cycle: 95

Control Type: Actuated-Coordinated

Splits and Phases: 3: Gun Club Road & Jewell Avenue



	↙	→	↘	↖	←	↗	↑	↗	↘	↓	↖	
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	138	229	76	229	392	46	297	792	123	39	494	174
v/c Ratio	0.80	0.68	0.12	0.70	0.88	0.10	0.70	0.85	0.14	0.21	0.68	0.24
Control Delay	62.4	56.1	1.6	41.6	66.1	0.4	23.2	38.2	1.8	16.2	37.7	4.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	62.4	56.1	1.6	41.6	66.1	0.4	23.2	38.2	1.8	16.2	37.7	4.8
Queue Length 50th (ft)	73	162	0	128	287	0	119	580	0	13	331	0
Queue Length 95th (ft)	#155	255	11	189	#412	0	118	463	0	29	451	43
Internal Link Dist (ft)		2523			1479			2542			790	
Turn Bay Length (ft)	220		330	300			485		360	200		325
Base Capacity (vph)	178	350	653	353	481	496	453	937	873	279	722	719
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.78	0.65	0.12	0.65	0.81	0.09	0.66	0.85	0.14	0.14	0.68	0.24

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM 6th Signalized Intersection Summary
11/22/2023

3: Gun Club Road & Jewell Avenue
2023 Existing (with Improvements) - AM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑ ↗	↑ ↘	↗ ↙	↑ ↗	↑ ↘	↗ ↙	↑ ↗	↑ ↘	↗ ↙	↑ ↗	↑ ↘	↗ ↙
Traffic Volume (veh/h)	131	218	72	199	341	40	193	515	80	34	430	151
Future Volume (veh/h)	131	218	72	199	341	40	193	515	80	34	430	151
Initial Q (Q _b), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00			1.00	1.00		1.00	1.00		1.00	1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No			No			No		
Adj Sat Flow, veh/h/ln	1811	1811	1811	1870	1870	1870	1870	1870	1870	1841	1841	1841
Adj Flow Rate, veh/h	138	229	76	229	392	46	297	792	123	39	494	174
Peak Hour Factor	0.95	0.95	0.95	0.87	0.87	0.87	0.65	0.65	0.65	0.87	0.87	0.87
Percent Heavy Veh, %	6	6	6	2	2	2	2	2	2	4	4	4
Cap, veh/h	198	321	442	333	428	363	415	945	801	188	771	653
Arrive On Green	0.07	0.18	0.18	0.12	0.23	0.23	0.11	0.51	0.51	0.02	0.42	0.42
Sat Flow, veh/h	1725	1811	1535	1781	1870	1585	1781	1870	1585	1753	1841	1560
Grp Volume(v), veh/h	138	229	76	229	392	46	297	792	123	39	494	174
Grp Sat Flow(s), veh/h/ln	1725	1811	1535	1781	1870	1585	1781	1870	1585	1753	1841	1560
Q Serve(g_s), s	7.9	14.3	4.5	12.2	24.5	2.8	10.9	43.6	5.0	1.5	25.6	8.8
Cycle Q Clear(g_c), s	7.9	14.3	4.5	12.2	24.5	2.8	10.9	43.6	5.0	1.5	25.6	8.8
Prop In Lane	1.00			1.00	1.00		1.00	1.00		1.00	1.00	1.00
Lane Grp Cap(c), veh/h	198	321	442	333	428	363	415	945	801	188	771	653
V/C Ratio(X)	0.70	0.71	0.17	0.69	0.92	0.13	0.72	0.84	0.15	0.21	0.64	0.27
Avail Cap(c_a), veh/h	198	347	464	360	483	409	485	945	801	335	771	653
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	0.96	0.96	0.96	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	38.9	46.5	32.0	34.0	45.1	36.7	20.0	25.5	15.9	24.0	27.7	22.8
Incr Delay (d2), s/veh	8.5	6.0	0.2	3.8	20.8	0.2	4.1	8.8	0.4	0.5	4.1	1.0
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	3.7	6.8	1.6	5.4	13.5	1.1	4.4	19.4	1.8	0.6	11.3	3.3
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	47.4	52.4	32.2	37.8	65.9	36.9	24.1	34.3	16.3	24.5	31.8	23.8
LnGrp LOS	D	D	C	D	E	D	C	C	B	C	C	C
Approach Vol, veh/h					667				1212			707
Approach Delay, s/veh	47.4				54.2			30.0			29.4	
Approach LOS		D			D			C			C	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+R _c), s	7.9	66.6	19.2	26.3	18.3	56.2	13.0	32.5				
Change Period (Y+R _c), s	5.0	6.0	5.0	5.0	5.0	6.0	5.0	5.0				
Max Green Setting (Gmax), s	13.0	47.0	16.0	23.0	18.0	42.0	8.0	31.0				
Max Q Clear Time (g_c+l1), s	3.5	45.6	14.2	16.3	12.9	27.6	9.9	26.5				
Green Ext Time (p_c), s	0.0	1.1	0.0	0.7	0.4	6.5	0.0	0.9				

Intersection Summary

HCM 6th Ctrl Delay 37.7
HCM 6th LOS D

Notes

User approved pedestrian interval to be less than phase max green.



Lane Group	EBT	EBR	WBL	WBT	SBT	SBR
Lane Configurations	↑	↗	↖	↑	↖	↗
Traffic Volume (vph)	679	283	93	574	3	231
Future Volume (vph)	679	283	93	574	3	231
Turn Type	NA	Perm	pm+pt	NA	NA	Perm
Protected Phases	4			3	8	6
Permitted Phases				4	8	6
Detector Phase	4	4	3	8	6	6
Switch Phase						
Minimum Initial (s)	8.0	8.0	4.0	8.0	8.0	8.0
Minimum Split (s)	25.0	25.0	12.5	25.0	27.0	27.0
Total Split (s)	50.5	50.5	12.5	63.0	27.0	27.0
Total Split (%)	56.1%	56.1%	13.9%	70.0%	30.0%	30.0%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	6.0
Lead/Lag	Lag	Lag	Lead			
Lead-Lag Optimize?	Yes	Yes	Yes			
Recall Mode	C-Max	C-Max	None	C-Max	Max	Max

Intersection Summary

Cycle Length: 90

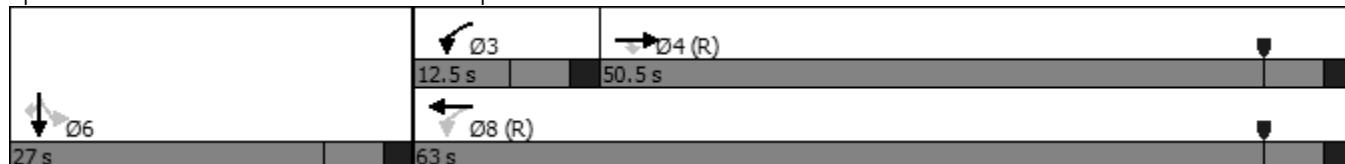
Actuated Cycle Length: 90

Offset: 83 (92%), Referenced to phase 4:EBT and 8:WBTL, Start of Yellow

Natural Cycle: 80

Control Type: Actuated-Coordinated

Splits and Phases: 1: E-470 Southbound Ramps & Jewell Avenue





Lane Group	EBT	EBR	WBL	WBT	SBT	SBR
Lane Group Flow (vph)	715	298	97	598	43	269
v/c Ratio	0.73	0.31	0.32	0.51	0.11	0.48
Control Delay	23.4	2.5	9.5	10.1	28.1	7.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	23.4	2.5	9.5	10.1	28.1	7.0
Queue Length 50th (ft)	319	0	18	117	19	0
Queue Length 95th (ft)	472	40	m38	236	44	51
Internal Link Dist (ft)	679			530	532	
Turn Bay Length (ft)		300	250			
Base Capacity (vph)	973	969	307	1179	399	561
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.73	0.31	0.32	0.51	0.11	0.48

Intersection Summary

m Volume for 95th percentile queue is metered by upstream signal.

HCM 6th Signalized Intersection Summary
11/22/2023

1: E-470 Southbound Ramps & Jewell Avenue
2023 Existing (with Improvements) - PM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	0	679	283	93	574	0	0	0	0	34	3	231
Future Volume (veh/h)	0	679	283	93	574	0	0	0	0	34	3	231
Initial Q (Q _b), veh	0	0	0	0	0	0				0	0	0
Ped-Bike Adj(A_pbT)	1.00			1.00	1.00					1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00				1.00	1.00	1.00
Work Zone On Approach		No			No						No	
Adj Sat Flow, veh/h/ln	0	1870	1870	1870	1870	0				1811	1811	1811
Adj Flow Rate, veh/h	0	715	298	97	598	0				40	3	269
Peak Hour Factor	0.95	0.95	0.95	0.96	0.96	0.96				0.86	0.86	0.86
Percent Heavy Veh, %	0	2	2	2	2	0				6	6	6
Cap, veh/h	0	980	831	284	1185	0				376	28	358
Arrive On Green	0.00	0.52	0.52	0.09	1.00	0.00				0.23	0.23	0.23
Sat Flow, veh/h	0	1870	1585	1781	1870	0				1610	121	1535
Grp Volume(v), veh/h	0	715	298	97	598	0				43	0	269
Grp Sat Flow(s), veh/h/ln	0	1870	1585	1781	1870	0				1731	0	1535
Q Serve(g_s), s	0.0	26.5	9.9	2.2	0.0	0.0				1.8	0.0	14.7
Cycle Q Clear(g_c), s	0.0	26.5	9.9	2.2	0.0	0.0				1.8	0.0	14.7
Prop In Lane	0.00		1.00	1.00		0.00				0.93		1.00
Lane Grp Cap(c), veh/h	0	980	831	284	1185	0				404	0	358
V/C Ratio(X)	0.00	0.73	0.36	0.34	0.50	0.00				0.11	0.00	0.75
Avail Cap(c_a), veh/h	0	980	831	336	1185	0				404	0	358
HCM Platoon Ratio	1.00	1.00	1.00	2.00	2.00	1.00				1.00	1.00	1.00
Upstream Filter(l)	0.00	1.00	1.00	0.59	0.59	0.00				1.00	0.00	1.00
Uniform Delay (d), s/veh	0.0	16.5	12.5	12.9	0.0	0.0				27.1	0.0	32.1
Incr Delay (d2), s/veh	0.0	4.8	1.2	0.4	0.9	0.0				0.5	0.0	13.5
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0				0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	0.0	11.0	3.3	0.7	0.3	0.0				0.8	0.0	6.7
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	0.0	21.3	13.8	13.3	0.9	0.0				27.7	0.0	45.6
LnGrp LOS	A	C	B	B	A	A				C	A	D
Approach Vol, veh/h		1013			695						312	
Approach Delay, s/veh		19.1			2.6						43.1	
Approach LOS		B			A						D	
Timer - Assigned Phs		3	4		6		8					
Phs Duration (G+Y+R _c), s		9.8	53.2		27.0		63.0					
Change Period (Y+R _c), s		6.0	6.0		6.0		6.0					
Max Green Setting (Gmax), s		6.5	44.5		21.0		57.0					
Max Q Clear Time (g _{c+l1}), s		4.2	28.5		16.7		2.0					
Green Ext Time (p _c), s		0.0	5.1		0.5		4.0					
Intersection Summary												
HCM 6th Ctrl Delay		17.1										
HCM 6th LOS			B									



Lane Group	EBL	EBT	WBT	WBR	NBT	NBR
Lane Configurations	↑ ↗	↑ ↘	↑ ↗	↗ ↘	↖ ↗	↖ ↘
Traffic Volume (vph)	172	541	559	74	0	59
Future Volume (vph)	172	541	559	74	0	59
Turn Type	pm+pt	NA	NA	Perm	NA	Perm
Protected Phases	7	4	8		2	
Permitted Phases	4			8		2
Detector Phase	7	4	8	8	2	2
Switch Phase						
Minimum Initial (s)	3.0	8.0	8.0	8.0	8.0	8.0
Minimum Split (s)	9.5	25.0	25.0	25.0	27.0	27.0
Total Split (s)	15.0	63.0	48.0	48.0	27.0	27.0
Total Split (%)	16.7%	70.0%	53.3%	53.3%	30.0%	30.0%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	6.0
Lead/Lag	Lead		Lag	Lag		
Lead-Lag Optimize?	Yes		Yes	Yes		
Recall Mode	None	C-Max	C-Max	C-Max	Max	Max

Intersection Summary

Cycle Length: 90

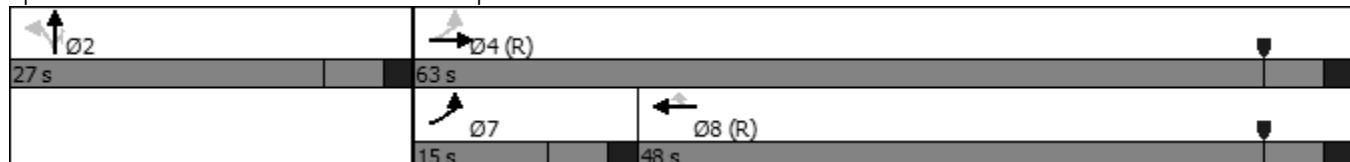
Actuated Cycle Length: 90

Offset: 0 (0%), Referenced to phase 4:EBTL and 8:WBT, Start of Yellow

Natural Cycle: 75

Control Type: Actuated-Coordinated

Splits and Phases: 2: E-470 Northbound Ramps & Jewell Avenue





Lane Group	EBL	EBT	WBT	WBR	NBT	NBR
Lane Group Flow (vph)	185	582	621	82	135	74
v/c Ratio	0.50	0.50	0.74	0.11	0.34	0.17
Control Delay	13.8	3.3	26.0	1.9	31.6	3.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	13.8	3.3	26.0	1.9	31.6	3.4
Queue Length 50th (ft)	12	29	279	0	64	0
Queue Length 95th (ft)	m47	48	417	15	102	9
Internal Link Dist (ft)		530	2523		717	
Turn Bay Length (ft)	250		250		175	
Base Capacity (vph)	377	1157	839	771	401	442
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.49	0.50	0.74	0.11	0.34	0.17

Intersection Summary

m Volume for 95th percentile queue is metered by upstream signal.

HCM 6th Signalized Intersection Summary
11/22/2023

2: E-470 Northbound Ramps & Jewell Avenue
2023 Existing (with Improvements) - PM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑ ↗	↑ ↘			↑ ↗	↑ ↘		↑ ↗	↑ ↘			
Traffic Volume (veh/h)	172	541	0	0	559	74	108	0	59	0	0	0
Future Volume (veh/h)	172	541	0	0	559	74	108	0	59	0	0	0
Initial Q (Q _b), veh	0	0	0	0	0	0	0	0	0			
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00			
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Work Zone On Approach		No			No			No				
Adj Sat Flow, veh/h/ln	1841	1841	0	0	1796	1796	1826	1826	1826			
Adj Flow Rate, veh/h	185	582	0	0	621	82	135	0	74			
Peak Hour Factor	0.93	0.93	0.93	0.90	0.90	0.90	0.80	0.80	0.80			
Percent Heavy Veh, %	4	4	0	0	7	7	5	5	5			
Cap, veh/h	374	1166	0	0	886	751	406	0	361			
Arrive On Green	0.15	1.00	0.00	0.00	0.49	0.49	0.23	0.00	0.23			
Sat Flow, veh/h	1753	1841	0	0	1796	1522	1739	0	1547			
Grp Volume(v), veh/h	185	582	0	0	621	82	135	0	74			
Grp Sat Flow(s), veh/h/ln	1753	1841	0	0	1796	1522	1739	0	1547			
Q Serve(g_s), s	4.6	0.0	0.0	0.0	24.1	2.6	5.8	0.0	3.5			
Cycle Q Clear(g_c), s	4.6	0.0	0.0	0.0	24.1	2.6	5.8	0.0	3.5			
Prop In Lane	1.00		0.00	0.00		1.00	1.00		1.00			
Lane Grp Cap(c), veh/h	374	1166	0	0	886	751	406	0	361			
V/C Ratio(X)	0.49	0.50	0.00	0.00	0.70	0.11	0.33	0.00	0.20			
Avail Cap(c_a), veh/h	420	1166	0	0	886	751	406	0	361			
HCM Platoon Ratio	2.00	2.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Upstream Filter(l)	0.60	0.60	0.00	0.00	0.74	0.74	1.00	0.00	1.00			
Uniform Delay (d), s/veh	12.4	0.0	0.0	0.0	17.7	12.2	28.7	0.0	27.8			
Incr Delay (d2), s/veh	0.6	0.9	0.0	0.0	3.4	0.2	2.2	0.0	1.3			
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0			
%ile BackOfQ(50%), veh/ln	1.3	0.3	0.0	0.0	9.5	0.8	2.6	0.0	1.4			
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	13.0	0.9	0.0	0.0	21.1	12.4	30.9	0.0	29.1			
LnGrp LOS	B	A	A	A	C	B	C	A	C			
Approach Vol, veh/h	767				703				209			
Approach Delay, s/veh	3.8				20.1				30.2			
Approach LOS	A				C				C			
Timer - Assigned Phs	2		4			7		8				
Phs Duration (G+Y+R _c), s	27.0		63.0			12.6		50.4				
Change Period (Y+R _c), s	6.0		6.0			6.0		6.0				
Max Green Setting (Gmax), s	21.0		57.0			9.0		42.0				
Max Q Clear Time (g_c+l1), s	7.8		2.0			6.6		26.1				
Green Ext Time (p_c), s	0.8		3.9			0.1		3.6				
Intersection Summary												
HCM 6th Ctrl Delay			13.9									
HCM 6th LOS			B									

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑
Traffic Volume (vph)	158	296	146	184	313	34	156	526	240	15	687	164
Future Volume (vph)	158	296	146	184	313	34	156	526	240	15	687	164
Turn Type	pm+pt	NA	pm+ov	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases	7	4	5	3	8		5	2		1	6	
Permitted Phases				4	8		8	2		2	6	6
Detector Phase	7	4	5	3	8	8	5	2	2	1	6	6
Switch Phase												
Minimum Initial (s)	3.0	4.0	4.0	3.0	5.0	5.0	4.0	8.0	8.0	4.0	8.0	8.0
Minimum Split (s)	9.5	32.0	9.5	9.5	32.0	32.0	9.5	40.0	40.0	9.5	29.0	29.0
Total Split (s)	14.0	35.0	12.0	16.0	37.0	37.0	12.0	57.0	57.0	12.0	57.0	57.0
Total Split (%)	11.7%	29.2%	10.0%	13.3%	30.8%	30.8%	10.0%	47.5%	47.5%	10.0%	47.5%	47.5%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	4.0	4.0	3.0	4.0	4.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	6.0	6.0	5.0	6.0	6.0
Lead/Lag	Lead	Lag	Lead	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes											
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max						

Intersection Summary

Cycle Length: 120

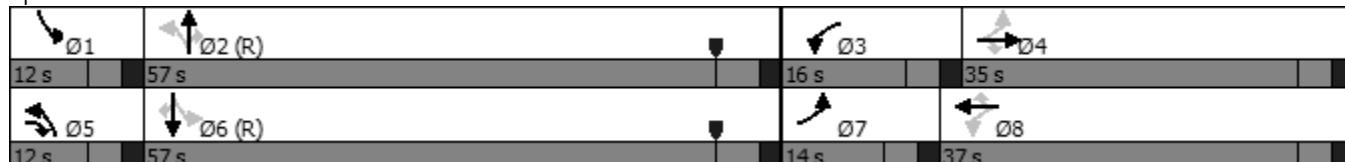
Actuated Cycle Length: 120

Offset: 108.5 (90%), Referenced to phase 2:NBTL and 6:SBTL, Start of Yellow

Natural Cycle: 95

Control Type: Actuated-Coordinated

Splits and Phases: 3: Gun Club Road & Jewell Avenue





Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	168	315	155	190	323	35	173	584	267	16	723	173
v/c Ratio	0.76	0.83	0.25	0.81	0.79	0.08	0.74	0.62	0.29	0.04	0.89	0.22
Control Delay	51.8	63.3	7.9	55.3	58.1	0.4	43.4	25.9	3.3	13.1	46.5	3.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	51.8	63.3	7.9	55.3	58.1	0.4	43.4	25.9	3.3	13.1	46.5	3.8
Queue Length 50th (ft)	93	233	13	107	235	0	73	283	0	5	514	0
Queue Length 95th (ft)	#151	326	60	#159	327	0	#243	532	50	17	#756	42
Internal Link Dist (ft)		2523			1479			2542			790	
Turn Bay Length (ft)	220		330	300			485		360	200		325
Base Capacity (vph)	228	456	625	245	487	500	234	947	929	379	812	789
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.74	0.69	0.25	0.78	0.66	0.07	0.74	0.62	0.29	0.04	0.89	0.22

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM 6th Signalized Intersection Summary
11/22/2023

3: Gun Club Road & Jewell Avenue
2023 Existing (with Improvements) - PM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑
Traffic Volume (veh/h)	158	296	146	184	313	34	156	526	240	15	687	164
Future Volume (veh/h)	158	296	146	184	313	34	156	526	240	15	687	164
Initial Q (Q _b), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No											
Adj Sat Flow, veh/h/ln	1841	1841	1841	1841	1841	1841	1796	1796	1796	1885	1885	1885
Adj Flow Rate, veh/h	168	315	155	190	323	35	173	584	267	16	723	173
Peak Hour Factor	0.94	0.94	0.94	0.97	0.97	0.97	0.90	0.90	0.90	0.95	0.95	0.95
Percent Heavy Veh, %	4	4	4	4	4	4	7	7	7	1	1	1
Cap, veh/h	235	358	394	246	389	329	254	943	799	278	906	768
Arrive On Green	0.08	0.19	0.19	0.09	0.21	0.21	0.06	0.53	0.53	0.01	0.48	0.48
Sat Flow, veh/h	1753	1841	1560	1753	1841	1560	1711	1796	1522	1795	1885	1598
Grp Volume(v), veh/h	168	315	155	190	323	35	173	584	267	16	723	173
Grp Sat Flow(s), veh/h/ln	1753	1841	1560	1753	1841	1560	1711	1796	1522	1795	1885	1598
Q Serve(g_s), s	9.0	20.0	9.9	10.4	20.1	2.2	6.0	27.5	12.1	0.5	38.8	7.6
Cycle Q Clear(g_c), s	9.0	20.0	9.9	10.4	20.1	2.2	6.0	27.5	12.1	0.5	38.8	7.6
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	235	358	394	246	389	329	254	943	799	278	906	768
V/C Ratio(X)	0.71	0.88	0.39	0.77	0.83	0.11	0.68	0.62	0.33	0.06	0.80	0.23
Avail Cap(c_a), veh/h	235	460	481	246	491	416	254	943	799	358	906	768
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	0.88	0.88	0.88	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	37.6	47.0	37.2	36.3	45.3	38.2	23.0	20.1	16.4	17.6	26.3	18.2
Incr Delay (d2), s/veh	7.6	13.2	0.6	12.7	9.4	0.1	7.3	3.0	1.1	0.1	7.3	0.7
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	4.3	10.2	3.7	5.2	9.9	0.8	2.6	11.0	4.2	0.2	17.4	2.8
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	45.1	60.1	37.7	49.0	54.7	38.3	30.2	23.1	17.5	17.7	33.5	18.8
LnGrp LOS	D	E	D	D	D	D	C	C	B	B	C	B
Approach Vol, veh/h		638			548			1024			912	
Approach Delay, s/veh		50.7			51.7			22.9			30.5	
Approach LOS		D			D			C			C	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	6.7	69.0	16.0	28.3	12.0	63.7	14.0	30.3				
Change Period (Y+Rc), s	5.0	6.0	5.0	5.0	5.0	6.0	5.0	5.0				
Max Green Setting (Gmax), s	7.0	51.0	11.0	30.0	7.0	51.0	9.0	32.0				
Max Q Clear Time (g_c+l1), s	2.5	29.5	12.4	22.0	8.0	40.8	11.0	22.1				
Green Ext Time (p_c), s	0.0	10.7	0.0	1.4	0.0	6.8	0.0	1.3				

Intersection Summary

HCM 6th Ctrl Delay 35.8
HCM 6th LOS D

Notes

User approved pedestrian interval to be less than phase max green.

***Intersection Capacity Worksheets:
2027 Background***



Lane Group	EBT	EBR	WBL	WBT	SBT	SBR
Lane Configurations	↑	↗	↖	↑	↖	↗
Traffic Volume (vph)	665	180	135	805	1	190
Future Volume (vph)	665	180	135	805	1	190
Turn Type	NA	Perm	pm+pt	NA	NA	Perm
Protected Phases	4			3	8	6
Permitted Phases				4	8	6
Detector Phase	4	4	3	8	6	6
Switch Phase						
Minimum Initial (s)	8.0	8.0	4.0	8.0	8.0	8.0
Minimum Split (s)	25.0	25.0	10.0	25.0	27.0	27.0
Total Split (s)	50.0	50.0	13.0	63.0	27.0	27.0
Total Split (%)	55.6%	55.6%	14.4%	70.0%	30.0%	30.0%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	6.0
Lead/Lag	Lag	Lag	Lead			
Lead-Lag Optimize?	Yes	Yes	Yes			
Recall Mode	C-Max	C-Max	None	C-Max	Max	Max

Intersection Summary

Cycle Length: 90

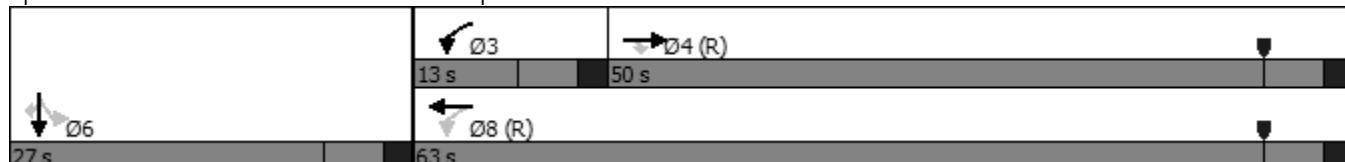
Actuated Cycle Length: 90

Offset: 68 (76%), Referenced to phase 4:EBT and 8:WBTL, Start of Yellow

Natural Cycle: 75

Control Type: Actuated-Coordinated

Splits and Phases: 1: E-470 Southbound Ramps & Jewell Avenue





Lane Group	EBT	EBR	WBL	WBT	SBT	SBR
Lane Group Flow (vph)	700	189	152	904	168	218
v/c Ratio	0.77	0.22	0.49	0.76	0.47	0.48
Control Delay	26.2	2.7	11.2	7.9	34.6	12.5
Queue Delay	0.0	0.0	0.0	0.5	0.0	0.0
Total Delay	26.2	2.7	11.2	8.4	34.6	12.5
Queue Length 50th (ft)	314	0	21	125	82	23
Queue Length 95th (ft)	466	33	m26	m141	139	81
Internal Link Dist (ft)	679			530	532	
Turn Bay Length (ft)		300	250			
Base Capacity (vph)	904	865	312	1191	361	450
Starvation Cap Reductn	0	0	0	61	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.77	0.22	0.49	0.80	0.47	0.48

Intersection Summary

m Volume for 95th percentile queue is metered by upstream signal.

HCM 6th Signalized Intersection Summary
11/22/2023

1: E-470 Southbound Ramps & Jewell Avenue
2027 Background - AM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	0	665	180	135	805	0	0	0	0	145	1	190
Future Volume (veh/h)	0	665	180	135	805	0	0	0	0	145	1	190
Initial Q (Q _b), veh	0	0	0	0	0	0				0	0	0
Ped-Bike Adj(A_pbT)	1.00			1.00	1.00					1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00				1.00	1.00	1.00
Work Zone On Approach		No			No						No	
Adj Sat Flow, veh/h/ln	0	1856	1856	1885	1885	0				1648	1841	1648
Adj Flow Rate, veh/h	0	700	189	152	904	0				167	1	218
Peak Hour Factor	0.95	0.95	0.95	0.89	0.89	0.89				0.87	0.87	0.87
Percent Heavy Veh, %	0	3	3	1	1	0				17	4	17
Cap, veh/h	0	939	796	319	1194	0				407	2	326
Arrive On Green	0.00	0.51	0.51	0.12	1.00	0.00				0.23	0.23	0.23
Sat Flow, veh/h	0	1856	1572	1795	1885	0				1743	10	1397
Grp Volume(v), veh/h	0	700	189	152	904	0				168	0	218
Grp Sat Flow(s), veh/h/ln	0	1856	1572	1795	1885	0				1754	0	1397
Q Serve(g_s), s	0.0	26.9	6.1	3.5	0.0	0.0				7.3	0.0	12.8
Cycle Q Clear(g_c), s	0.0	26.9	6.1	3.5	0.0	0.0				7.3	0.0	12.8
Prop In Lane	0.00		1.00	1.00		0.00				0.99		1.00
Lane Grp Cap(c), veh/h	0	939	796	319	1194	0				409	0	326
V/C Ratio(X)	0.00	0.75	0.24	0.48	0.76	0.00				0.41	0.00	0.67
Avail Cap(c_a), veh/h	0	939	796	350	1194	0				409	0	326
HCM Platoon Ratio	1.00	1.00	1.00	2.00	2.00	1.00				1.00	1.00	1.00
Upstream Filter(l)	0.00	1.00	1.00	0.18	0.18	0.00				1.00	0.00	1.00
Uniform Delay (d), s/veh	0.0	17.6	12.5	13.4	0.0	0.0				29.3	0.0	31.3
Incr Delay (d2), s/veh	0.0	5.4	0.7	0.2	0.8	0.0				3.0	0.0	10.4
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0				0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	0.0	11.3	2.0	1.0	0.3	0.0				3.4	0.0	5.1
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	0.0	23.0	13.2	13.6	0.8	0.0				32.3	0.0	41.8
LnGrp LOS	A	C	B	B	A	A				C	A	D
Approach Vol, veh/h		889			1056					386		
Approach Delay, s/veh		20.9			2.7					37.6		
Approach LOS		C			A					D		
Timer - Assigned Phs		3	4		6		8					
Phs Duration (G+Y+R _c), s		11.5	51.5		27.0		63.0					
Change Period (Y+R _c), s		6.0	6.0		6.0		6.0					
Max Green Setting (Gmax), s		7.0	44.0		21.0		57.0					
Max Q Clear Time (g _{c+l1}), s		5.5	28.9		14.8		2.0					
Green Ext Time (p _c), s		0.1	4.5		0.9		7.9					
Intersection Summary												
HCM 6th Ctrl Delay			15.4									
HCM 6th LOS			B									



Lane Group	EBL	EBT	WBT	WBR	NBT	NBR
Lane Configurations	↑ ↗	↑ ↗	↑ ↗	↑ ↗	↑ ↗	↑ ↗
Traffic Volume (vph)	225	585	745	70	0	125
Future Volume (vph)	225	585	745	70	0	125
Turn Type	pm+pt	NA	NA	Perm	NA	Perm
Protected Phases	7	4	8		2	
Permitted Phases	4			8		2
Detector Phase	7	4	8	8	2	2
Switch Phase						
Minimum Initial (s)	3.0	8.0	8.0	8.0	8.0	8.0
Minimum Split (s)	9.5	25.0	25.0	25.0	27.0	27.0
Total Split (s)	15.0	64.0	49.0	49.0	26.0	26.0
Total Split (%)	16.7%	71.1%	54.4%	54.4%	28.9%	28.9%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	6.0
Lead/Lag	Lead		Lag	Lag		
Lead-Lag Optimize?	Yes		Yes	Yes		
Recall Mode	None	C-Max	C-Max	C-Max	Max	Max

Intersection Summary

Cycle Length: 90

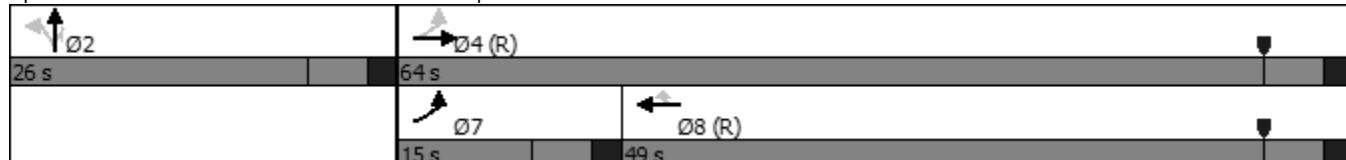
Actuated Cycle Length: 90

Offset: 60 (67%), Referenced to phase 4:EBTL and 8:WBT, Start of Yellow

Natural Cycle: 90

Control Type: Actuated-Coordinated

Splits and Phases: 2: E-470 Northbound Ramps & Jewell Avenue





Lane Group	EBL	EBT	WBT	WBR	NBT	NBR
Lane Group Flow (vph)	237	616	847	80	224	144
v/c Ratio	0.94	0.53	0.96	0.10	0.58	0.31
Control Delay	65.8	10.0	46.6	1.7	38.0	7.3
Queue Delay	0.0	0.1	0.0	0.0	0.0	0.0
Total Delay	65.8	10.2	46.6	1.7	38.0	7.3
Queue Length 50th (ft)	118	135	446	0	114	0
Queue Length 95th (ft)	m#184	250	#683	13	181	42
Internal Link Dist (ft)		530	2523		717	
Turn Bay Length (ft)	250			250		175
Base Capacity (vph)	252	1166	881	806	389	460
Starvation Cap Reductn	0	88	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.94	0.57	0.96	0.10	0.58	0.31

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

HCM 6th Signalized Intersection Summary
11/22/2023

2: E-470 Northbound Ramps & Jewell Avenue
2027 Background - AM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑			↑	↑		↑	↑	0	0	0
Traffic Volume (veh/h)	225	585	0	0	745	70	195	0	125	0	0	0
Future Volume (veh/h)	225	585	0	0	745	70	195	0	125	0	0	0
Initial Q (Q _b), veh	0	0	0	0	0	0	0	0	0			
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00			
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Work Zone On Approach		No			No			No				
Adj Sat Flow, veh/h/ln	1826	1826	0	0	1856	1856	1856	1856	1856			
Adj Flow Rate, veh/h	237	616	0	0	847	80	224	0	144			
Peak Hour Factor	0.95	0.95	0.95	0.88	0.88	0.88	0.87	0.87	0.87			
Percent Heavy Veh, %	5	5	0	0	3	3	3	3	3			
Cap, veh/h	271	1177	0	0	905	767	393	0	349			
Arrive On Green	0.18	1.00	0.00	0.00	0.49	0.49	0.22	0.00	0.22			
Sat Flow, veh/h	1739	1826	0	0	1856	1572	1767	0	1572			
Grp Volume(v), veh/h	237	616	0	0	847	80	224	0	144			
Grp Sat Flow(s), veh/h/ln	1739	1826	0	0	1856	1572	1767	0	1572			
Q Serve(g_s), s	6.1	0.0	0.0	0.0	38.7	2.5	10.2	0.0	7.1			
Cycle Q Clear(g_c), s	6.1	0.0	0.0	0.0	38.7	2.5	10.2	0.0	7.1			
Prop In Lane	1.00		0.00	0.00		1.00	1.00		1.00			
Lane Grp Cap(c), veh/h	271	1177	0	0	905	767	393	0	349			
V/C Ratio(X)	0.88	0.52	0.00	0.00	0.94	0.10	0.57	0.00	0.41			
Avail Cap(c_a), veh/h	288	1177	0	0	905	767	393	0	349			
HCM Platoon Ratio	2.00	2.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Upstream Filter(l)	0.54	0.54	0.00	0.00	0.62	0.62	1.00	0.00	1.00			
Uniform Delay (d), s/veh	17.7	0.0	0.0	0.0	21.7	12.5	31.2	0.0	30.0			
Incr Delay (d2), s/veh	14.6	0.9	0.0	0.0	12.6	0.2	5.9	0.0	3.6			
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0			
%ile BackOfQ(50%), veh/ln	3.1	0.3	0.0	0.0	17.6	0.8	4.9	0.0	3.0			
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	32.3	0.9	0.0	0.0	34.3	12.6	37.1	0.0	33.5			
LnGrp LOS	C	A	A	A	C	B	D	A	C			
Approach Vol, veh/h		853			927			368				
Approach Delay, s/veh		9.6			32.5			35.7				
Approach LOS		A			C			D				
Timer - Assigned Phs		2		4			7	8				
Phs Duration (G+Y+Rc), s		26.0		64.0			14.1	49.9				
Change Period (Y+Rc), s		6.0		6.0			6.0	6.0				
Max Green Setting (Gmax), s		20.0		58.0			9.0	43.0				
Max Q Clear Time (g_c+l1), s		12.2		2.0			8.1	40.7				
Green Ext Time (p_c), s		1.1		4.2			0.1	1.3				

Intersection Summary

HCM 6th Ctrl Delay 23.9
HCM 6th LOS C

Notes

User approved pedestrian interval to be less than phase max green.

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑↑	↑	↑	↑↑	↑	↑	↑↑	↑↑	↑	↑	↑↑	↑
Traffic Volume (vph)	150	245	315	300	385	45	260	655	105	40	560	170
Future Volume (vph)	150	245	315	300	385	45	260	655	105	40	560	170
Turn Type	Prot	NA	pm+ov	Prot	NA	Perm	Prot	NA	Perm	pm+pt	NA	Perm
Protected Phases	7	4	5	3	8		5	2		1	6	
Permitted Phases						8				2	6	6
Detector Phase	7	4	5	3	8	8	5	2	2	1	6	6
Switch Phase												
Minimum Initial (s)	3.0	4.0	4.0	3.0	5.0	5.0	4.0	8.0	8.0	4.0	8.0	8.0
Minimum Split (s)	8.0	32.0	9.0	8.0	32.0	32.0	9.0	40.0	40.0	9.0	29.0	29.0
Total Split (s)	12.0	32.0	24.0	25.0	45.0	45.0	24.0	54.0	54.0	9.0	39.0	39.0
Total Split (%)	10.0%	26.7%	20.0%	20.8%	37.5%	37.5%	20.0%	45.0%	45.0%	7.5%	32.5%	32.5%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	4.0	4.0	3.0	4.0	4.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	6.0	6.0	5.0	6.0	6.0
Lead/Lag	Lead	Lag	Lead	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes											
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max						

Intersection Summary

Cycle Length: 120

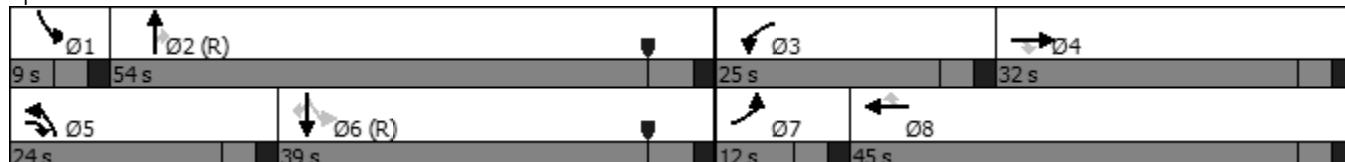
Actuated Cycle Length: 120

Offset: 108.5 (90%), Referenced to phase 2:NBT and 6:SBTL, Start of Yellow

Natural Cycle: 90

Control Type: Actuated-Coordinated

Splits and Phases: 3: Gun Club Road & Jewell Avenue





Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	158	258	332	345	443	52	400	1008	162	46	644	195
v/c Ratio	0.85	0.68	0.48	0.82	0.86	0.09	0.78	0.62	0.20	0.20	0.55	0.30
Control Delay	92.0	53.0	18.3	66.9	57.0	0.3	54.3	40.5	14.5	19.1	35.9	5.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	92.0	53.0	18.3	66.9	57.0	0.3	54.3	40.5	14.5	19.1	35.9	5.5
Queue Length 50th (ft)	63	184	115	136	324	0	154	408	27	16	216	0
Queue Length 95th (ft)	#122	273	194	173	402	0	148	336	47	38	287	47
Internal Link Dist (ft)		2523			1479			2465			790	
Turn Bay Length (ft)	220		330	300			485		360	200		325
Base Capacity (vph)	192	410	700	572	621	637	554	1629	816	227	1180	660
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.82	0.63	0.47	0.60	0.71	0.08	0.72	0.62	0.20	0.20	0.55	0.30

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM 6th Signalized Intersection Summary
11/22/2023

3: Gun Club Road & Jewell Avenue
2027 Background - AM Peak Hour

Movement	EBL	EBT	EBC	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑↑	↑	↑↑	↑↑	↑	↑↑	↑↑	↑↑	↑	↑	↑↑	↑↑
Traffic Volume (veh/h)	150	245	315	300	385	45	260	655	105	40	560	170
Future Volume (veh/h)	150	245	315	300	385	45	260	655	105	40	560	170
Initial Q (Q _b), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00			1.00	1.00		1.00	1.00		1.00	1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No			No			No		
Adj Sat Flow, veh/h/ln	1811	1811	1811	1870	1870	1870	1870	1870	1870	1841	1841	1841
Adj Flow Rate, veh/h	158	258	332	345	443	52	400	1008	162	46	644	195
Peak Hour Factor	0.95	0.95	0.95	0.87	0.87	0.87	0.65	0.65	0.65	0.87	0.87	0.87
Percent Heavy Veh, %	6	6	6	2	2	2	2	2	2	4	4	4
Cap, veh/h	195	373	522	398	491	416	463	1698	758	234	1294	577
Arrive On Green	0.06	0.21	0.21	0.12	0.26	0.26	0.13	0.48	0.48	0.03	0.37	0.37
Sat Flow, veh/h	3346	1811	1535	3456	1870	1585	3456	3554	1585	1753	3497	1560
Grp Volume(v), veh/h	158	258	332	345	443	52	400	1008	162	46	644	195
Grp Sat Flow(s), veh/h/ln	1673	1811	1535	1728	1870	1585	1728	1777	1585	1753	1749	1560
Q Serve(g_s), s	5.6	15.8	21.9	11.8	27.5	3.0	13.6	24.8	7.1	2.0	17.1	10.8
Cycle Q Clear(g_c), s	5.6	15.8	21.9	11.8	27.5	3.0	13.6	24.8	7.1	2.0	17.1	10.8
Prop In Lane	1.00			1.00	1.00		1.00	1.00		1.00	1.00	1.00
Lane Grp Cap(c), veh/h	195	373	522	398	491	416	463	1698	758	234	1294	577
V/C Ratio(X)	0.81	0.69	0.64	0.87	0.90	0.12	0.86	0.59	0.21	0.20	0.50	0.34
Avail Cap(c_a), veh/h	195	407	551	576	623	528	547	1698	758	246	1294	577
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	0.87	0.87	0.87	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	55.8	44.1	33.4	52.2	42.7	33.7	50.9	22.8	18.2	23.1	29.2	27.2
Incr Delay (d2), s/veh	18.2	3.9	2.0	7.0	14.0	0.1	11.9	1.5	0.6	0.4	1.4	1.6
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	2.8	7.3	8.1	5.4	14.1	1.1	6.4	9.8	2.6	0.8	7.0	4.1
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	74.1	48.0	35.3	59.1	56.7	33.9	62.8	24.4	18.9	23.5	30.6	28.8
LnGrp LOS	E	D	D	E	E	C	E	C	B	C	C	C
Approach Vol, veh/h		748			840			1570			885	
Approach Delay, s/veh		47.9			56.3			33.6			29.8	
Approach LOS		D			E			C			C	

Intersection Summary

HCM 6th Ctrl Delay 40.1
HCM 6th LOS D

Notes

User approved pedestrian interval to be less than phase max green.

Intersection

Int Delay, s/veh 0.3

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	0	5	50	1020	1060	100
Future Vol, veh/h	0	5	50	1020	1060	100
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	200	-	-	200
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	95	95	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	5	54	1074	1116	109

Major/Minor	Minor2	Major1	Major2		
Conflicting Flow All	-	558	1225	0	-
Stage 1	-	-	-	-	-
Stage 2	-	-	-	-	-
Critical Hdwy	-	6.94	4.14	-	-
Critical Hdwy Stg 1	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-
Follow-up Hdwy	-	3.32	2.22	-	-
Pot Cap-1 Maneuver	0	473	565	-	-
Stage 1	0	-	-	-	-
Stage 2	0	-	-	-	-
Platoon blocked, %				-	-
Mov Cap-1 Maneuver	-	473	565	-	-
Mov Cap-2 Maneuver	-	-	-	-	-
Stage 1	-	-	-	-	-
Stage 2	-	-	-	-	-

Approach	EB	NB	SB	
HCM Control Delay, s	12.7	0.6	0	
HCM LOS	B			

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	565	-	473	-	-
HCM Lane V/C Ratio	0.096	-	0.011	-	-
HCM Control Delay (s)	12	-	12.7	-	-
HCM Lane LOS	B	-	B	-	-
HCM 95th %tile Q(veh)	0.3	-	0	-	-

Lane Group	EBL	EBT	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑ ↗	↗ ↘	↖ ↗	↑ ↗	↗ ↗	↖ ↗	↑↑ ↗	↗ ↗	↖ ↗	↑↑ ↗	↗ ↗
Traffic Volume (vph)	50	5	55	5	45	100	975	30	75	865	125
Future Volume (vph)	50	5	55	5	45	100	975	30	75	865	125
Turn Type	pm+pt	NA	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases	7	4	3	8		5	2		1	6	
Permitted Phases					8	2		2	6		6
Detector Phase	7	4	3	8	8	5	2	2	1	6	6
Switch Phase											
Minimum Initial (s)	5.0	8.0	5.0	8.0	8.0	5.0	8.0	8.0	5.0	8.0	8.0
Minimum Split (s)	11.0	33.0	11.0	33.0	33.0	11.0	30.0	30.0	11.0	30.0	30.0
Total Split (s)	20.0	42.0	20.0	42.0	42.0	20.0	38.0	38.0	20.0	38.0	38.0
Total Split (%)	16.7%	35.0%	16.7%	35.0%	35.0%	16.7%	31.7%	31.7%	16.7%	31.7%	31.7%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0
Lead/Lag	Lead	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes										
Recall Mode	None	None	None	None	None	None	C-Max	C-Max	None	C-Max	C-Max

Intersection Summary

Cycle Length: 120

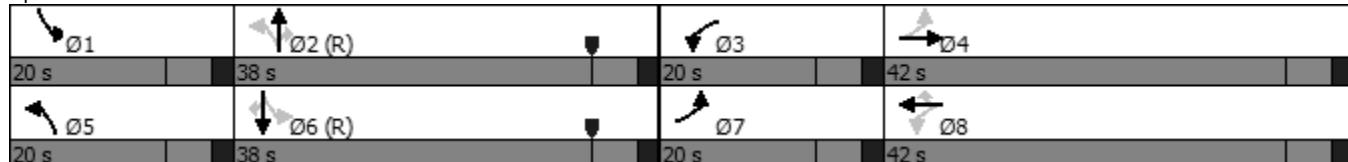
Actuated Cycle Length: 120

Offset: 0 (0%), Referenced to phase 2:NBTL and 6:SBTL, Start of Yellow

Natural Cycle: 85

Control Type: Actuated-Coordinated

Splits and Phases: 102: Gun Club Road & Yale Avenue





Lane Group	EBL	EBT	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	54	16	60	5	49	109	1026	33	82	911	136
v/c Ratio	0.30	0.13	0.25	0.04	0.21	0.26	0.44	0.03	0.21	0.40	0.13
Control Delay	48.4	34.0	42.9	52.8	2.0	7.2	13.7	0.1	5.8	16.3	6.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	48.4	34.0	42.9	52.8	2.0	7.2	13.7	0.1	5.8	16.3	6.1
Queue Length 50th (ft)	41	4	39	4	0	24	223	0	26	218	23
Queue Length 95th (ft)	70	27	77	17	0	45	318	0	m33	377	59
Internal Link Dist (ft)		816		718			2631			1641	
Turn Bay Length (ft)	150		350		10	300		300	300		300
Base Capacity (vph)	256	509	279	558	570	509	2358	1100	474	2254	1057
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.21	0.03	0.22	0.01	0.09	0.21	0.44	0.03	0.17	0.40	0.13

Intersection Summary

m Volume for 95th percentile queue is metered by upstream signal.

HCM 6th Signalized Intersection Summary
11/22/2023

102: Gun Club Road & Yale Avenue
2027 Background - AM Peak Hour

Movement	EBL	EBT	EBC	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑	↑	↑	↑	↑	↑	↑↑	↑	↑	↑↑	↑
Traffic Volume (veh/h)	50	5	10	55	5	45	100	975	30	75	865	125
Future Volume (veh/h)	50	5	10	55	5	45	100	975	30	75	865	125
Initial Q (Q _b), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00			1.00	1.00		1.00	1.00		1.00	1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	54	5	11	60	5	0	109	1026	33	82	911	136
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.95	0.92	0.92	0.95	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	214	32	70	207	122		422	2338	1043	406	2333	1040
Arrive On Green	0.04	0.06	0.06	0.04	0.07	0.00	0.04	0.66	0.66	0.04	0.66	0.66
Sat Flow, veh/h	1781	520	1144	1781	1870	1585	1781	3554	1585	1781	3554	1585
Grp Volume(v), veh/h	54	0	16	60	5	0	109	1026	33	82	911	136
Grp Sat Flow(s), veh/h/ln	1781	0	1664	1781	1870	1585	1781	1777	1585	1781	1777	1585
Q Serve(g_s), s	3.4	0.0	1.1	3.8	0.3	0.0	2.4	16.7	0.9	1.8	14.2	3.9
Cycle Q Clear(g_c), s	3.4	0.0	1.1	3.8	0.3	0.0	2.4	16.7	0.9	1.8	14.2	3.9
Prop In Lane	1.00			1.00			1.00	1.00		1.00	1.00	1.00
Lane Grp Cap(c), veh/h	214	0	102	207	122		422	2338	1043	406	2333	1040
V/C Ratio(X)	0.25	0.00	0.16	0.29	0.04		0.26	0.44	0.03	0.20	0.39	0.13
Avail Cap(c_a), veh/h	354	0	499	341	561		558	2338	1043	544	2333	1040
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	0.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	50.2	0.0	53.4	50.0	52.6	0.0	6.9	9.9	7.2	7.2	9.5	7.7
Incr Delay (d2), s/veh	0.6	0.0	0.7	0.8	0.1	0.0	0.3	0.6	0.1	0.2	0.5	0.3
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	1.5	0.0	0.5	1.7	0.1	0.0	0.7	5.5	0.3	0.6	4.7	1.3
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	50.8	0.0	54.1	50.7	52.7	0.0	7.2	10.5	7.2	7.4	10.0	8.0
LnGrp LOS	D	A	D	D	D		A	B	A	A	B	A
Approach Vol, veh/h		70			65			1168			1129	
Approach Delay, s/veh		51.6			50.9			10.1			9.6	
Approach LOS		D			D			B			A	

Intersection Summary

HCM 6th Ctrl Delay

12.1

HCM 6th LOS

B

Notes

Unsignalized Delay for [WBR] is excluded from calculations of the approach delay and intersection delay.

Intersection

Int Delay, s/veh 0.2

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↑	↗	↑↑	↖	↖	↑↑
Traffic Vol, veh/h	5	15	1005	15	20	1155
Future Vol, veh/h	5	15	1005	15	20	1155
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	350	0	-	350	350	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	5	16	1092	16	22	1255

Major/Minor	Minor1	Major1	Major2		
Conflicting Flow All	1764	546	0	0	1108
Stage 1	1092	-	-	-	-
Stage 2	672	-	-	-	-
Critical Hdwy	6.84	6.94	-	-	4.14
Critical Hdwy Stg 1	5.84	-	-	-	-
Critical Hdwy Stg 2	5.84	-	-	-	-
Follow-up Hdwy	3.52	3.32	-	-	2.22
Pot Cap-1 Maneuver	*270	*674	-	-	*1009
Stage 1	*636	-	-	-	-
Stage 2	*589	-	-	-	-
Platoon blocked, %	1	1	-	-	1
Mov Cap-1 Maneuver	*264	*674	-	-	*1009
Mov Cap-2 Maneuver	*264	-	-	-	-
Stage 1	*636	-	-	-	-
Stage 2	*576	-	-	-	-

Approach	WB	NB	SB
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HCM Control Delay, s 12.6 0 0.1

HCM LOS B

Minor Lane/Major Mvmt	NBT	NBR	WBLn1	WBLn2	SBL	SBT
Capacity (veh/h)	-	-	264	674	* 1009	-
HCM Lane V/C Ratio	-	-	0.021	0.024	0.022	-
HCM Control Delay (s)	-	-	18.9	10.5	8.6	-
HCM Lane LOS	-	-	C	B	A	-
HCM 95th %tile Q(veh)	-	-	0.1	0.1	0.1	-

Notes

~: Volume exceeds capacity \$: Delay exceeds 300s +: Computation Not Defined *: All major volume in platoon



Lane Group	EBT	EBR	WBL	WBT	SBT	SBR
Lane Configurations	↑	↗	↖	↑	↖	↗
Traffic Volume (vph)	790	320	170	700	5	260
Future Volume (vph)	790	320	170	700	5	260
Turn Type	NA	Perm	pm+pt	NA	NA	Perm
Protected Phases	4			3	8	6
Permitted Phases				4	8	6
Detector Phase	4	4	3	8	6	6
Switch Phase						
Minimum Initial (s)	8.0	8.0	4.0	8.0	8.0	8.0
Minimum Split (s)	25.0	25.0	12.5	25.0	27.0	27.0
Total Split (s)	50.5	50.5	12.5	63.0	27.0	27.0
Total Split (%)	56.1%	56.1%	13.9%	70.0%	30.0%	30.0%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	6.0
Lead/Lag	Lag	Lag	Lead			
Lead-Lag Optimize?	Yes	Yes	Yes			
Recall Mode	C-Max	C-Max	None	C-Max	Max	Max

Intersection Summary

Cycle Length: 90

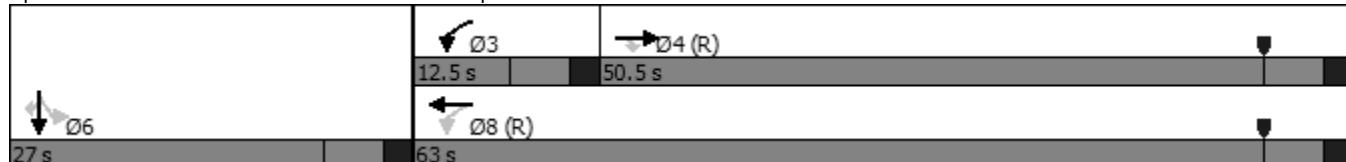
Actuated Cycle Length: 90

Offset: 83 (92%), Referenced to phase 4:EBT and 8:WBTL, Start of Yellow

Natural Cycle: 90

Control Type: Actuated-Coordinated

Splits and Phases: 1: E-470 Southbound Ramps & Jewell Avenue





Lane Group	EBT	EBR	WBL	WBT	SBT	SBR
Lane Group Flow (vph)	832	337	177	729	70	302
v/c Ratio	0.90	0.35	0.84	0.62	0.18	0.56
Control Delay	36.1	2.6	43.3	13.0	29.1	11.6
Queue Delay	0.0	0.0	0.0	0.3	0.0	0.0
Total Delay	36.1	2.6	43.3	13.3	29.1	11.6
Queue Length 50th (ft)	413	0	82	219	32	27
Queue Length 95th (ft)	#667	41	m92	m226	64	90
Internal Link Dist (ft)	679			530	532	
Turn Bay Length (ft)		300	250			
Base Capacity (vph)	921	953	211	1179	399	541
Starvation Cap Reductn	0	0	0	98	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.90	0.35	0.84	0.67	0.18	0.56

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

HCM 6th Signalized Intersection Summary
11/22/2023

1: E-470 Southbound Ramps & Jewell Avenue
2027 Background - PM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	0	790	320	170	700	0	0	0	0	55	5	260
Future Volume (veh/h)	0	790	320	170	700	0	0	0	0	55	5	260
Initial Q (Q _b), veh	0	0	0	0	0	0				0	0	0
Ped-Bike Adj(A_pbT)	1.00			1.00			1.00			1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00				1.00	1.00	1.00
Work Zone On Approach		No			No						No	
Adj Sat Flow, veh/h/ln	0	1870	1870	1870	1870	0				1811	1811	1811
Adj Flow Rate, veh/h	0	832	337	177	729	0				64	6	302
Peak Hour Factor	0.95	0.95	0.95	0.96	0.96	0.96				0.86	0.86	0.86
Percent Heavy Veh, %	0	2	2	2	2	0				6	6	6
Cap, veh/h	0	931	789	248	1185	0				369	35	358
Arrive On Green	0.00	0.50	0.50	0.14	1.00	0.00				0.23	0.23	0.23
Sat Flow, veh/h	0	1870	1585	1781	1870	0				1583	148	1535
Grp Volume(v), veh/h	0	832	337	177	729	0				70	0	302
Grp Sat Flow(s), veh/h/ln	0	1870	1585	1781	1870	0				1732	0	1535
Q Serve(g_s), s	0.0	36.2	12.2	4.2	0.0	0.0				2.9	0.0	16.9
Cycle Q Clear(g_c), s	0.0	36.2	12.2	4.2	0.0	0.0				2.9	0.0	16.9
Prop In Lane	0.00		1.00	1.00		0.00				0.91		1.00
Lane Grp Cap(c), veh/h	0	931	789	248	1185	0				404	0	358
V/C Ratio(X)	0.00	0.89	0.43	0.71	0.62	0.00				0.17	0.00	0.84
Avail Cap(c_a), veh/h	0	931	789	255	1185	0				404	0	358
HCM Platoon Ratio	1.00	1.00	1.00	2.00	2.00	1.00				1.00	1.00	1.00
Upstream Filter(l)	0.00	1.00	1.00	0.09	0.09	0.00				1.00	0.00	1.00
Uniform Delay (d), s/veh	0.0	20.4	14.4	18.0	0.0	0.0				27.6	0.0	32.9
Incr Delay (d2), s/veh	0.0	12.8	1.7	0.8	0.2	0.0				0.9	0.0	20.9
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0				0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	0.0	16.8	4.2	1.6	0.1	0.0				1.3	0.0	8.2
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	0.0	33.2	16.1	18.9	0.2	0.0				28.5	0.0	53.8
LnGrp LOS	A	C	B	B	A	A				C	A	D
Approach Vol, veh/h		1169			906					372		
Approach Delay, s/veh		28.3			3.9					49.0		
Approach LOS		C			A					D		
Timer - Assigned Phs		3	4		6		8					
Phs Duration (G+Y+R _c), s		12.2	50.8		27.0		63.0					
Change Period (Y+R _c), s		6.0	6.0		6.0		6.0					
Max Green Setting (Gmax), s		6.5	44.5		21.0		57.0					
Max Q Clear Time (g _{c+l1}), s		6.2	38.2		18.9		2.0					
Green Ext Time (p _c), s		0.0	3.4		0.3		5.4					
Intersection Summary												
HCM 6th Ctrl Delay			22.4									
HCM 6th LOS			C									



Lane Group	EBL	EBT	WBT	WBR	NBT	NBR
Lane Configurations	↑ ↗	↑ ↘	↑ ↗	↗ ↘	↖ ↗	↖ ↘
Traffic Volume (vph)	195	650	750	170	0	80
Future Volume (vph)	195	650	750	170	0	80
Turn Type	pm+pt	NA	NA	Perm	NA	Perm
Protected Phases	7	4	8		2	
Permitted Phases	4			8		2
Detector Phase	7	4	8	8	2	2
Switch Phase						
Minimum Initial (s)	3.0	8.0	8.0	8.0	8.0	8.0
Minimum Split (s)	9.5	25.0	25.0	25.0	27.0	27.0
Total Split (s)	15.0	63.0	48.0	48.0	27.0	27.0
Total Split (%)	16.7%	70.0%	53.3%	53.3%	30.0%	30.0%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	6.0
Lead/Lag	Lead		Lag	Lag		
Lead-Lag Optimize?	Yes		Yes	Yes		
Recall Mode	None	C-Max	C-Max	C-Max	Max	Max

Intersection Summary

Cycle Length: 90

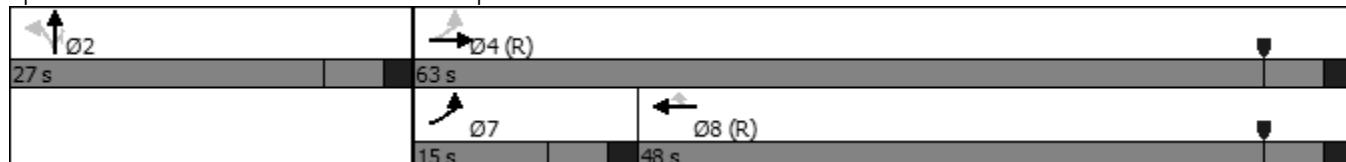
Actuated Cycle Length: 90

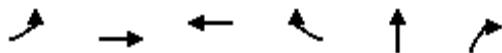
Offset: 0 (0%), Referenced to phase 4:EBTL and 8:WBT, Start of Yellow

Natural Cycle: 90

Control Type: Actuated-Coordinated

Splits and Phases: 2: E-470 Northbound Ramps & Jewell Avenue





Lane Group	EBL	EBT	WBT	WBR	NBT	NBR
Lane Group Flow (vph)	210	699	833	189	150	100
v/c Ratio	0.83	0.60	1.00	0.24	0.37	0.23
Control Delay	51.6	3.9	58.1	3.1	32.2	6.5
Queue Delay	0.0	0.2	0.0	0.0	0.0	0.0
Total Delay	51.6	4.1	58.1	3.1	32.2	6.5
Queue Length 50th (ft)	86	43	~463	1	72	0
Queue Length 95th (ft)	m106	m59	#724	35	112	25
Internal Link Dist (ft)		530	2523		717	
Turn Bay Length (ft)	250			250		175
Base Capacity (vph)	254	1157	829	804	401	442
Starvation Cap Reductn	0	64	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.83	0.64	1.00	0.24	0.37	0.23

Intersection Summary

- ~ Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.
- m Volume for 95th percentile queue is metered by upstream signal.

HCM 6th Signalized Intersection Summary
11/22/2023

2: E-470 Northbound Ramps & Jewell Avenue
2027 Background - PM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑ ↗	↑ ↘			↑ ↗	↑ ↘		↑ ↗	↑ ↘			
Traffic Volume (veh/h)	195	650	0	0	750	170	120	0	80	0	0	0
Future Volume (veh/h)	195	650	0	0	750	170	120	0	80	0	0	0
Initial Q (Q _b), veh	0	0	0	0	0	0	0	0	0			
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00			
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Work Zone On Approach		No			No			No				
Adj Sat Flow, veh/h/ln	1841	1841	0	0	1796	1796	1826	1826	1826			
Adj Flow Rate, veh/h	210	699	0	0	833	189	150	0	100			
Peak Hour Factor	0.93	0.93	0.93	0.90	0.90	0.90	0.80	0.80	0.80			
Percent Heavy Veh, %	4	4	0	0	7	7	5	5	5			
Cap, veh/h	245	1166	0	0	871	738	406	0	361			
Arrive On Green	0.16	1.00	0.00	0.00	0.48	0.48	0.23	0.00	0.23			
Sat Flow, veh/h	1753	1841	0	0	1796	1522	1739	0	1547			
Grp Volume(v), veh/h	210	699	0	0	833	189	150	0	100			
Grp Sat Flow(s), veh/h/ln	1753	1841	0	0	1796	1522	1739	0	1547			
Q Serve(g_s), s	5.3	0.0	0.0	0.0	40.1	6.6	6.5	0.0	4.8			
Cycle Q Clear(g_c), s	5.3	0.0	0.0	0.0	40.1	6.6	6.5	0.0	4.8			
Prop In Lane	1.00		0.00	0.00		1.00	1.00		1.00			
Lane Grp Cap(c), veh/h	245	1166	0	0	871	738	406	0	361			
V/C Ratio(X)	0.86	0.60	0.00	0.00	0.96	0.26	0.37	0.00	0.28			
Avail Cap(c_a), veh/h	277	1166	0	0	871	738	406	0	361			
HCM Platoon Ratio	2.00	2.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Upstream Filter(l)	0.31	0.31	0.00	0.00	0.65	0.65	1.00	0.00	1.00			
Uniform Delay (d), s/veh	18.3	0.0	0.0	0.0	22.3	13.6	28.9	0.0	28.3			
Incr Delay (d2), s/veh	7.6	0.7	0.0	0.0	16.2	0.5	2.6	0.0	1.9			
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0			
%ile BackOfQ(50%), veh/ln	2.3	0.2	0.0	0.0	18.4	2.1	3.0	0.0	1.9			
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	25.9	0.7	0.0	0.0	38.4	14.2	31.5	0.0	30.2			
LnGrp LOS	C	A	A	A	D	B	C	A	C			
Approach Vol, veh/h	909				1022				250			
Approach Delay, s/veh	6.5				34.0				31.0			
Approach LOS	A				C				C			
Timer - Assigned Phs	2		4			7		8				
Phs Duration (G+Y+R _c), s	27.0		63.0			13.4		49.6				
Change Period (Y+R _c), s	6.0		6.0			6.0		6.0				
Max Green Setting (Gmax), s	21.0		57.0			9.0		42.0				
Max Q Clear Time (g_c+l1), s	8.5		2.0			7.3		42.1				
Green Ext Time (p_c), s	0.9		5.1			0.1		0.0				
Intersection Summary												
HCM 6th Ctrl Delay			22.2									
HCM 6th LOS			C									

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑↑	↑	↑	↑↑	↑	↑	↑↑	↑↑	↑	↑	↑↑	↑
Traffic Volume (vph)	180	335	215	235	350	40	385	705	335	15	885	185
Future Volume (vph)	180	335	215	235	350	40	385	705	335	15	885	185
Turn Type	Prot	NA	pm+ov	Prot	NA	Perm	Prot	NA	Perm	pm+pt	NA	Perm
Protected Phases	7	4	5	3	8		5	2		1	6	
Permitted Phases						8			2	6		6
Detector Phase	7	4	5	3	8	8	5	2	2	1	6	6
Switch Phase												
Minimum Initial (s)	3.0	4.0	4.0	3.0	5.0	5.0	4.0	8.0	8.0	4.0	8.0	8.0
Minimum Split (s)	8.0	32.0	9.0	8.0	32.0	32.0	9.0	40.0	40.0	9.0	29.0	29.0
Total Split (s)	15.0	37.0	25.0	16.0	38.0	38.0	25.0	58.0	58.0	9.0	42.0	42.0
Total Split (%)	12.5%	30.8%	20.8%	13.3%	31.7%	31.7%	20.8%	48.3%	48.3%	7.5%	35.0%	35.0%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	4.0	4.0	3.0	4.0	4.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	6.0	6.0	5.0	6.0	6.0
Lead/Lag	Lead	Lag	Lead	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes											
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max						

Intersection Summary

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBTL, Start of Yellow

Natural Cycle: 90

Control Type: Actuated-Coordinated

Splits and Phases: 3: Gun Club Road & Jewell Avenue





Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	191	356	229	242	361	41	405	742	353	16	932	195
v/c Ratio	0.78	0.85	0.33	0.85	0.82	0.08	0.80	0.42	0.37	0.05	0.73	0.28
Control Delay	76.0	63.2	15.5	80.2	58.6	0.3	61.3	20.2	3.3	15.9	39.3	5.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	76.0	63.2	15.5	80.2	58.6	0.3	61.3	20.2	3.3	15.9	39.3	5.4
Queue Length 50th (ft)	75	263	74	95	262	0	155	172	0	5	342	0
Queue Length 95th (ft)	#117	364	125	#157	365	0	211	273	55	17	#453	54
Internal Link Dist (ft)		2523			1479			2465			790	
Turn Bay Length (ft)	220		330	300			485		360	200		325
Base Capacity (vph)	280	487	721	308	502	545	547	1763	956	329	1273	695
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.68	0.73	0.32	0.79	0.72	0.08	0.74	0.42	0.37	0.05	0.73	0.28

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM 6th Signalized Intersection Summary
11/22/2023

3: Gun Club Road & Jewell Avenue
2027 Background - PM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑↑	↑	↑	↑↑	↑	↑	↑↑	↑↑	↑	↑	↑↑	↑
Traffic Volume (veh/h)	180	335	215	235	350	40	385	705	335	15	885	185
Future Volume (veh/h)	180	335	215	235	350	40	385	705	335	15	885	185
Initial Q (Q _b), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No			No			No		
Adj Sat Flow, veh/h/ln	1841	1841	1841	1841	1841	1796	1796	1796	1885	1885	1885	1885
Adj Flow Rate, veh/h	191	356	229	242	361	41	405	742	353	16	932	195
Peak Hour Factor	0.94	0.94	0.94	0.97	0.97	0.97	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, %	4	4	4	4	4	4	7	7	7	1	1	1
Cap, veh/h	244	401	559	295	429	363	466	1729	771	276	1360	607
Arrive On Green	0.07	0.22	0.22	0.09	0.23	0.23	0.14	0.51	0.51	0.01	0.38	0.38
Sat Flow, veh/h	3401	1841	1560	3401	1841	1560	3319	3413	1522	1795	3582	1598
Grp Volume(v), veh/h	191	356	229	242	361	41	405	742	353	16	932	195
Grp Sat Flow(s), veh/h/ln	1700	1841	1560	1700	1841	1560	1659	1706	1522	1795	1791	1598
Q Serve(g_s), s	6.6	22.5	13.2	8.4	22.5	2.5	14.3	16.5	17.9	0.7	26.2	10.3
Cycle Q Clear(g_c), s	6.6	22.5	13.2	8.4	22.5	2.5	14.3	16.5	17.9	0.7	26.2	10.3
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	244	401	559	295	429	363	466	1729	771	276	1360	607
V/C Ratio(X)	0.78	0.89	0.41	0.82	0.84	0.11	0.87	0.43	0.46	0.06	0.69	0.32
Avail Cap(c_a), veh/h	283	491	635	312	506	429	553	1729	771	312	1360	607
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	0.81	0.81	0.81	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	54.8	45.5	28.9	53.9	43.9	36.3	50.5	18.7	19.0	22.3	31.2	26.3
Incr Delay (d2), s/veh	7.9	12.9	0.4	14.1	10.7	0.1	12.3	0.8	2.0	0.1	2.8	1.4
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	3.0	11.4	4.8	4.1	11.2	0.9	6.5	6.1	6.3	0.3	11.0	4.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	62.7	58.4	29.3	67.9	54.6	36.4	62.8	19.5	21.0	22.4	34.0	27.7
LnGrp LOS	E	E	C	E	D	D	E	B	C	C	C	C
Approach Vol, veh/h		776			644			1500			1143	
Approach Delay, s/veh		50.9			58.5			31.5			32.8	
Approach LOS		D			E			C			C	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	6.7	66.8	15.4	31.2	21.9	51.6	13.6	33.0				
Change Period (Y+Rc), s	5.0	6.0	5.0	5.0	5.0	6.0	5.0	5.0				
Max Green Setting (Gmax), s	4.0	52.0	11.0	32.0	20.0	36.0	10.0	33.0				
Max Q Clear Time (g_c+l1), s	2.7	19.9	10.4	24.5	16.3	28.2	8.6	24.5				
Green Ext Time (p_c), s	0.0	17.0	0.0	1.7	0.5	6.1	0.0	1.3				

Intersection Summary

HCM 6th Ctrl Delay 39.8

HCM 6th LOS D

Notes

User approved pedestrian interval to be less than phase max green.

Intersection

Int Delay, s/veh 0.1

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	0	20	5	1415	1310	15
Future Vol, veh/h	0	20	5	1415	1310	15
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	200	-	-	200
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	95	95	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	22	5	1489	1379	16

Major/Minor	Minor2	Major1	Major2		
Conflicting Flow All	-	690	1395	0	-
Stage 1	-	-	-	-	-
Stage 2	-	-	-	-	-
Critical Hdwy	-	6.94	4.14	-	-
Critical Hdwy Stg 1	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-
Follow-up Hdwy	-	3.32	2.22	-	-
Pot Cap-1 Maneuver	0	388	486	-	-
Stage 1	0	-	-	-	-
Stage 2	0	-	-	-	-
Platoon blocked, %				-	-
Mov Cap-1 Maneuver	-	388	486	-	-
Mov Cap-2 Maneuver	-	-	-	-	-
Stage 1	-	-	-	-	-
Stage 2	-	-	-	-	-

Approach	EB	NB	SB	
HCM Control Delay, s	14.8	0	0	
HCM LOS	B			

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	486	-	388	-	-
HCM Lane V/C Ratio	0.011	-	0.056	-	-
HCM Control Delay (s)	12.5	-	14.8	-	-
HCM Lane LOS	B	-	B	-	-
HCM 95th %tile Q(veh)	0	-	0.2	-	-

Lane Group	EBL	EBT	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑ ↗	↑ ↘	↑ ↗	↑ ↘	↗ ↗	↑ ↗	↑↑ ↗	↗ ↗	↑ ↗	↑↑ ↗	↗ ↗
Traffic Volume (vph)	305	5	95	5	75	15	1040	50	105	1205	20
Future Volume (vph)	305	5	95	5	75	15	1040	50	105	1205	20
Turn Type	pm+pt	NA	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases	7	4	3	8		5	2		1	6	
Permitted Phases					8	2		2	6		6
Detector Phase	7	4	3	8	8	5	2	2	1	6	6
Switch Phase											
Minimum Initial (s)	5.0	8.0	5.0	8.0	8.0	5.0	8.0	8.0	5.0	8.0	8.0
Minimum Split (s)	11.0	33.0	11.0	29.0	29.0	11.0	30.0	30.0	11.0	30.0	30.0
Total Split (s)	16.0	33.0	12.0	29.0	29.0	11.0	34.0	34.0	11.0	34.0	34.0
Total Split (%)	17.8%	36.7%	13.3%	32.2%	32.2%	12.2%	37.8%	37.8%	12.2%	37.8%	37.8%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0
Lead/Lag	Lead	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes										
Recall Mode	None	None	None	None	None	None	C-Max	C-Max	None	C-Max	C-Max

Intersection Summary

Cycle Length: 90

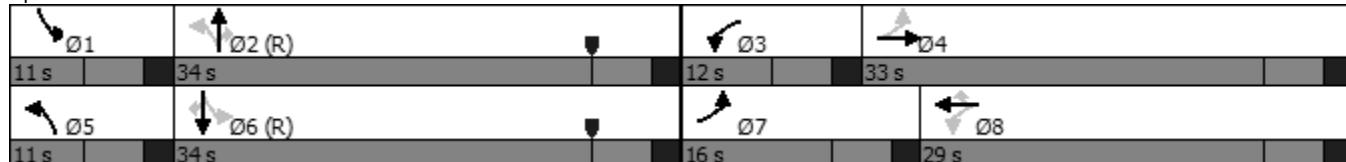
Actuated Cycle Length: 90

Offset: 0 (0%), Referenced to phase 2:NBTL and 6:SBTL, Start of Yellow

Natural Cycle: 85

Control Type: Actuated-Coordinated

Splits and Phases: 102: Gun Club Road & Yale Avenue





Lane Group	EBL	EBT	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	332	76	103	5	82	16	1095	54	114	1268	22
v/c Ratio	1.19	0.28	0.49	0.03	0.27	0.06	0.66	0.06	0.38	0.59	0.02
Control Delay	145.9	13.3	35.9	38.0	2.1	8.4	21.8	0.1	11.4	14.3	0.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	145.9	13.3	35.9	38.0	2.1	8.4	21.8	0.1	11.4	14.3	0.1
Queue Length 50th (ft)	~193	3	46	3	0	3	256	0	25	217	0
Queue Length 95th (ft)	#318	42	88	13	0	11	351	0	48	393	0
Internal Link Dist (ft)		816		718			2631			1641	
Turn Bay Length (ft)	150		350		10	300		300	300		300
Base Capacity (vph)	279	530	212	476	540	254	1670	843	297	2135	1027
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	1.19	0.14	0.49	0.01	0.15	0.06	0.66	0.06	0.38	0.59	0.02

Intersection Summary

- ~ Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

HCM 6th Signalized Intersection Summary
11/22/2023

102: Gun Club Road & Yale Avenue
2027 Background - PM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑	↑	↑	↑	↑	↑	↑↑	↑	↑	↑↑	↑
Traffic Volume (veh/h)	305	5	65	95	5	75	15	1040	50	105	1205	20
Future Volume (veh/h)	305	5	65	95	5	75	15	1040	50	105	1205	20
Initial Q (Q _b), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00			1.00	1.00		1.00	1.00	1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	332	5	71	103	5	0	16	1095	54	114	1268	22
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.95	0.92	0.92	0.95	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	399	14	198	315	165		218	1712	764	296	1833	818
Arrive On Green	0.11	0.13	0.13	0.07	0.09	0.00	0.02	0.48	0.48	0.05	0.52	0.52
Sat Flow, veh/h	1781	105	1496	1781	1870	1585	1781	3554	1585	1781	3554	1585
Grp Volume(v), veh/h	332	0	76	103	5	0	16	1095	54	114	1268	22
Grp Sat Flow(s), veh/h/ln	1781	0	1601	1781	1870	1585	1781	1777	1585	1781	1777	1585
Q Serve(g_s), s	10.0	0.0	3.9	4.7	0.2	0.0	0.4	20.8	1.6	2.9	24.2	0.6
Cycle Q Clear(g_c), s	10.0	0.0	3.9	4.7	0.2	0.0	0.4	20.8	1.6	2.9	24.2	0.6
Prop In Lane	1.00		0.93	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	399	0	212	315	165		218	1712	764	296	1833	818
V/C Ratio(X)	0.83	0.00	0.36	0.33	0.03		0.07	0.64	0.07	0.38	0.69	0.03
Avail Cap(c_a), veh/h	399	0	480	315	478		285	1712	764	302	1833	818
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	0.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	35.9	0.0	35.6	34.1	37.5	0.0	13.9	17.5	12.5	13.6	16.4	10.7
Incr Delay (d2), s/veh	14.0	0.0	1.0	0.6	0.1	0.0	0.1	1.8	0.2	0.8	2.2	0.1
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	3.8	0.0	1.6	2.0	0.1	0.0	0.1	7.5	0.6	1.0	8.5	0.2
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	49.9	0.0	36.6	34.7	37.6	0.0	14.1	19.3	12.7	14.4	18.6	10.8
LnGrp LOS	D	A	D	C	D		B	B	B	B	B	B
Approach Vol, veh/h		408			108			1165			1404	
Approach Delay, s/veh		47.4			34.9			18.9			18.1	
Approach LOS		D			C			B			B	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	10.7	49.4	12.0	17.9	7.6	52.4	16.0	13.9				
Change Period (Y+Rc), s	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0				
Max Green Setting (Gmax), s	5.0	28.0	6.0	27.0	5.0	28.0	10.0	23.0				
Max Q Clear Time (g_c+l1), s	4.9	22.8	6.7	5.9	2.4	26.2	12.0	2.2				
Green Ext Time (p_c), s	0.0	3.0	0.0	0.3	0.0	1.3	0.0	0.0				
Intersection Summary												
HCM 6th Ctrl Delay			22.9									
HCM 6th LOS			C									
Notes												
User approved pedestrian interval to be less than phase max green.												
Unsignalized Delay for [WBR] is excluded from calculations of the approach delay and intersection delay.												

Intersection

Int Delay, s/veh 3.4

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↑	↗	↑↑	↖	↖	↑↑
Traffic Vol, veh/h	25	25	1400	15	35	1300
Future Vol, veh/h	25	25	1400	15	35	1300
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	350	0	-	350	350	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	27	27	1522	16	38	1413

Major/Minor	Minor1	Major1	Major2		
Conflicting Flow All	2305	761	0	0	1538
Stage 1	1522	-	-	-	-
Stage 2	783	-	-	-	-
Critical Hdwy	6.84	6.94	-	-	4.14
Critical Hdwy Stg 1	5.84	-	-	-	-
Critical Hdwy Stg 2	5.84	-	-	-	-
Follow-up Hdwy	3.52	3.32	-	-	2.22
Pot Cap-1 Maneuver	32	348	-	-	428
Stage 1	166	-	-	-	-
Stage 2	411	-	-	-	-
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	29	348	-	-	428
Mov Cap-2 Maneuver	29	-	-	-	-
Stage 1	166	-	-	-	-
Stage 2	374	-	-	-	-

Approach WB NB SB

HCM Control Delay, s 180.2 0 0.4

HCM LOS F

Minor Lane/Major Mvmt	NBT	NBR	WBLn1	WBLn2	SBL	SBT
Capacity (veh/h)	-	-	29	348	428	-
HCM Lane V/C Ratio	-	-	0.937	0.078	0.089	-
HCM Control Delay (s)	-	\$ 344.2	16.2	14.2	-	-
HCM Lane LOS	-	-	F	C	B	-
HCM 95th %tile Q(veh)	-	-	3.1	0.3	0.3	-

***Intersection Capacity Worksheets:
2050 Background***



Lane Group	EBT	EBR	WBL	WBT	SBT	SBR
Lane Configurations	↑↑↑	↗	↖↑↑	↑↑↑	↖	↗
Traffic Volume (vph)	1655	260	320	2640	5	250
Future Volume (vph)	1655	260	320	2640	5	250
Turn Type	NA	Perm	pm+pt	NA	NA	Perm
Protected Phases	4			3	8	6
Permitted Phases				4	8	6
Detector Phase	4	4	3	8	6	6
Switch Phase						
Minimum Initial (s)	8.0	8.0	4.0	8.0	8.0	8.0
Minimum Split (s)	25.0	25.0	10.0	25.0	27.0	27.0
Total Split (s)	48.0	48.0	14.0	62.0	28.0	28.0
Total Split (%)	53.3%	53.3%	15.6%	68.9%	31.1%	31.1%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	6.0
Lead/Lag	Lag	Lag	Lead			
Lead-Lag Optimize?	Yes	Yes	Yes			
Recall Mode	C-Max	C-Max	None	C-Max	Max	Max

Intersection Summary

Cycle Length: 90

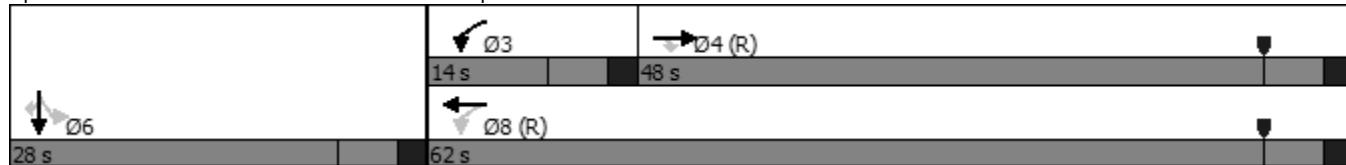
Actuated Cycle Length: 90

Offset: 68 (76%), Referenced to phase 4:EBT and 8:WBTL, Start of Yellow

Natural Cycle: 75

Control Type: Actuated-Coordinated

Splits and Phases: 1: E-470 Southbound Ramps & Jewell Avenue





Lane Group	EBT	EBR	WBL	WBT	SBT	SBR
Lane Group Flow (vph)	1689	265	327	2694	282	255
v/c Ratio	0.72	0.30	0.71	0.85	0.66	0.55
Control Delay	21.4	2.9	21.4	22.4	39.3	21.8
Queue Delay	0.0	0.0	0.0	0.2	0.0	0.0
Total Delay	21.4	2.9	21.4	22.7	39.3	21.8
Queue Length 50th (ft)	271	0	76	441	145	70
Queue Length 95th (ft)	327	40	m75	m429	232	148
Internal Link Dist (ft)	679			530	532	
Turn Bay Length (ft)		300	250			
Base Capacity (vph)	2357	875	465	3164	425	461
Starvation Cap Reductn	0	0	0	81	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.72	0.30	0.70	0.87	0.66	0.55

Intersection Summary

m Volume for 95th percentile queue is metered by upstream signal.

HCM 6th Signalized Intersection Summary
11/22/2023

1: E-470 Southbound Ramps & Jewell Avenue
2050 Background - AM Peak Hour



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	0	1655	260	320	2640	0	0	0	0	260	5	250
Future Volume (veh/h)	0	1655	260	320	2640	0	0	0	0	260	5	250
Initial Q (Q _b), veh	0	0	0	0	0	0				0	0	0
Ped-Bike Adj(A_pbT)	1.00			1.00			1.00			1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00				1.00	1.00	1.00
Work Zone On Approach		No			No						No	
Adj Sat Flow, veh/h/ln	0	1856	1856	1870	1870	0				1841	1841	1841
Adj Flow Rate, veh/h	0	1689	265	327	2694	0				277	5	0
Peak Hour Factor	0.92	0.98	0.98	0.98	0.98	0.92				0.94	0.98	0.98
Percent Heavy Veh, %	0	3	3	2	2	0				4	4	4
Cap, veh/h	0	2458	763	502	3177	0				421	8	
Arrive On Green	0.00	0.49	0.49	0.14	1.00	0.00				0.24	0.24	0.00
Sat Flow, veh/h	0	5233	1572	3456	5274	0				1723	31	1560
Grp Volume(v), veh/h	0	1689	265	327	2694	0				282	0	0
Grp Sat Flow(s), veh/h/ln	0	1689	1572	1728	1702	0				1755	0	1560
Q Serve(g_s), s	0.0	23.2	9.4	4.1	0.0	0.0				13.0	0.0	0.0
Cycle Q Clear(g_c), s	0.0	23.2	9.4	4.1	0.0	0.0				13.0	0.0	0.0
Prop In Lane	0.00		1.00	1.00		0.00				0.98		1.00
Lane Grp Cap(c), veh/h	0	2458	763	502	3177	0				429	0	
V/C Ratio(X)	0.00	0.69	0.35	0.65	0.85	0.00				0.66	0.00	
Avail Cap(c_a), veh/h	0	2458	763	567	3177	0				429	0	
HCM Platoon Ratio	1.00	1.00	1.00	2.00	2.00	1.00				1.00	1.00	1.00
Upstream Filter(l)	0.00	1.00	1.00	0.09	0.09	0.00				1.00	0.00	0.00
Uniform Delay (d), s/veh	0.0	17.9	14.3	15.8	0.0	0.0				30.6	0.0	0.0
Incr Delay (d2), s/veh	0.0	1.6	1.2	0.2	0.3	0.0				7.7	0.0	0.0
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0				0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	0.0	8.2	3.2	1.4	0.1	0.0				6.3	0.0	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	0.0	19.5	15.6	16.0	0.3	0.0				38.3	0.0	0.0
LnGrp LOS	A	B	B	B	A	A				D	A	
Approach Vol, veh/h		1954			3021					282		
Approach Delay, s/veh		18.9			2.0					38.3		
Approach LOS		B			A					D		
Timer - Assigned Phs		3	4		6		8					
Phs Duration (G+Y+R _c), s		12.3	49.7		28.0		62.0					
Change Period (Y+R _c), s		6.0	6.0		6.0		6.0					
Max Green Setting (Gmax), s		8.0	42.0		22.0		56.0					
Max Q Clear Time (g _{c+l1}), s		6.1	25.2		15.0		2.0					
Green Ext Time (p _c), s		0.2	11.1		0.9		39.5					
Intersection Summary												
HCM 6th Ctrl Delay		10.2										
HCM 6th LOS		B										
Notes												

Unsignalized Delay for [SBR] is excluded from calculations of the approach delay and intersection delay.



Lane Group	EBL	EBT	WBT	WBR	NBT	NBR
Lane Configurations	↑↑	↑↑↑	↑↑↑	↑	↑	↑
Traffic Volume (vph)	280	1635	2610	450	0	250
Future Volume (vph)	280	1635	2610	450	0	250
Turn Type	pm+pt	NA	NA	Perm	NA	Perm
Protected Phases	7	4	8		2	
Permitted Phases	4			8		2
Detector Phase	7	4	8	8	2	2
Switch Phase						
Minimum Initial (s)	3.0	8.0	8.0	8.0	8.0	8.0
Minimum Split (s)	9.5	25.0	25.0	25.0	27.0	27.0
Total Split (s)	11.0	63.0	52.0	52.0	27.0	27.0
Total Split (%)	12.2%	70.0%	57.8%	57.8%	30.0%	30.0%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	6.0
Lead/Lag	Lead		Lag	Lag		
Lead-Lag Optimize?	Yes		Yes	Yes		
Recall Mode	None	C-Max	C-Max	C-Max	Max	Max

Intersection Summary

Cycle Length: 90

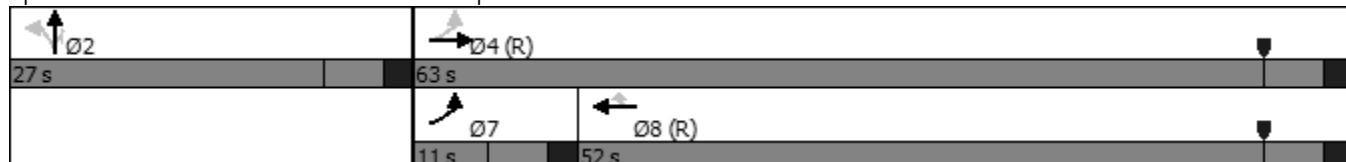
Actuated Cycle Length: 90

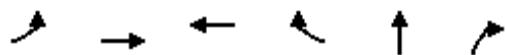
Offset: 0 (0%), Referenced to phase 4:EBTL and 8:WBT, Start of Yellow

Natural Cycle: 90

Control Type: Actuated-Coordinated

Splits and Phases: 2: E-470 Northbound Ramps & Jewell Avenue





Lane Group	EBL	EBT	WBT	WBR	NBT	NBR
Lane Group Flow (vph)	286	1668	2663	459	357	255
v/c Ratio	0.85	0.54	1.04	0.46	0.86	0.56
Control Delay	44.7	2.6	54.0	3.4	55.5	22.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	44.7	2.6	54.0	3.4	55.5	22.6
Queue Length 50th (ft)	35	28	~607	10	196	71
Queue Length 95th (ft)	m#91	41	#702	57	#350	149
Internal Link Dist (ft)		530	2523		717	
Turn Bay Length (ft)	250				175	
Base Capacity (vph)	338	3098	2549	1001	413	452
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.85	0.54	1.04	0.46	0.86	0.56

Intersection Summary

- ~ Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.
- m Volume for 95th percentile queue is metered by upstream signal.

HCM 6th Signalized Intersection Summary
11/22/2023

2: E-470 Northbound Ramps & Jewell Avenue
2050 Background - AM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	280	1635	0	0	2610	450	350	0	250	0	0	0
Future Volume (veh/h)	280	1635	0	0	2610	450	350	0	250	0	0	0
Initial Q (Q _b), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00			
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Work Zone On Approach		No			No			No				
Adj Sat Flow, veh/h/ln	1811	1811	0	0	1841	1841	1870	1870	1870			
Adj Flow Rate, veh/h	286	1668	0	0	2663	459	357	0	0			
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98			
Percent Heavy Veh, %	6	6	0	0	4	4	2	2	2			
Cap, veh/h	346	3131	0	0	2568	797	416	0				
Arrive On Green	0.11	1.00	0.00	0.00	0.51	0.51	0.23	0.00	0.00			
Sat Flow, veh/h	3346	5107	0	0	5191	1560	1781	0	1585			
Grp Volume(v), veh/h	286	1668	0	0	2663	459	357	0	0			
Grp Sat Flow(s), veh/h/ln	1673	1648	0	0	1675	1560	1781	0	1585			
Q Serve(g_s), s	3.6	0.0	0.0	0.0	46.0	18.3	17.3	0.0	0.0			
Cycle Q Clear(g_c), s	3.6	0.0	0.0	0.0	46.0	18.3	17.3	0.0	0.0			
Prop In Lane	1.00		0.00	0.00		1.00	1.00		1.00			
Lane Grp Cap(c), veh/h	346	3131	0	0	2568	797	416	0				
V/C Ratio(X)	0.83	0.53	0.00	0.00	1.04	0.58	0.86	0.00				
Avail Cap(c_a), veh/h	346	3131	0	0	2568	797	416	0				
HCM Platoon Ratio	2.00	2.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Upstream Filter(l)	0.63	0.63	0.00	0.00	0.09	0.09	1.00	0.00	0.00			
Uniform Delay (d), s/veh	19.9	0.0	0.0	0.0	22.0	15.2	33.1	0.0	0.0			
Incr Delay (d2), s/veh	10.1	0.4	0.0	0.0	18.2	0.3	20.0	0.0	0.0			
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0			
%ile BackOfQ(50%), veh/ln	1.8	0.1	0.0	0.0	19.5	5.7	9.6	0.0	0.0			
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	30.0	0.4	0.0	0.0	40.2	15.5	53.1	0.0	0.0			
LnGrp LOS	C	A	A	A	F	B	D	A				
Approach Vol, veh/h		1954			3122			357				
Approach Delay, s/veh		4.7			36.6			53.1				
Approach LOS		A			D			D				
Timer - Assigned Phs	2		4			7		8				
Phs Duration (G+Y+Rc), s	27.0		63.0			11.0		52.0				
Change Period (Y+Rc), s	6.0		6.0			6.0		6.0				
Max Green Setting (Gmax), s	21.0		57.0			5.0		46.0				
Max Q Clear Time (g_c+l1), s	19.3		2.0			5.6		48.0				
Green Ext Time (p_c), s	0.4		17.9			0.0		0.0				
Intersection Summary												
HCM 6th Ctrl Delay			26.2									
HCM 6th LOS			C									
Notes												
User approved pedestrian interval to be less than phase max green.												
Unsignalized Delay for [NBR] is excluded from calculations of the approach delay and intersection delay.												

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑↑	↑↑↑	↑	↑↑	↑↑↑	↑	↑↑	↑↑↑	↑	↑↑	↑↑↑	↑
Traffic Volume (vph)	365	1110	410	480	2355	505	395	1580	330	285	1020	310
Future Volume (vph)	365	1110	410	480	2355	505	395	1580	330	285	1020	310
Turn Type	Prot	NA	Free	Prot	NA	pm+ov	Prot	NA	Free	Prot	NA	pm+ov
Protected Phases	7	4		3	8	1	5	2		1	6	7
Permitted Phases			Free			8			Free			6
Detector Phase	7	4		3	8	1	5	2		1	6	7
Switch Phase												
Minimum Initial (s)	3.0	8.0		3.0	8.0	4.0	5.0	8.0		4.0	8.0	3.0
Minimum Split (s)	9.5	40.0		9.5	40.0	9.5	9.5	40.0		9.5	29.0	9.5
Total Split (s)	14.2	48.8		34.2	68.8	16.0	24.2	51.0		16.0	42.8	14.2
Total Split (%)	9.5%	32.5%		22.8%	45.9%	10.7%	16.1%	34.0%		10.7%	28.5%	9.5%
Yellow Time (s)	3.0	4.0		3.0	4.0	3.0	3.5	4.0		3.0	4.0	3.0
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	1.0	2.0		2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Total Lost Time (s)	5.0	6.0		5.0	6.0	5.0	4.5	6.0		5.0	6.0	5.0
Lead/Lag	Lead	Lag		Lead	Lag	Lead	Lead	Lag		Lead	Lag	Lead
Lead-Lag Optimize?	Yes	Yes		Yes	Yes	Yes	Yes	Yes		Yes	Yes	Yes
Recall Mode	None	None		None	None	None	None	C-Max		None	C-Max	None

Intersection Summary

Cycle Length: 150

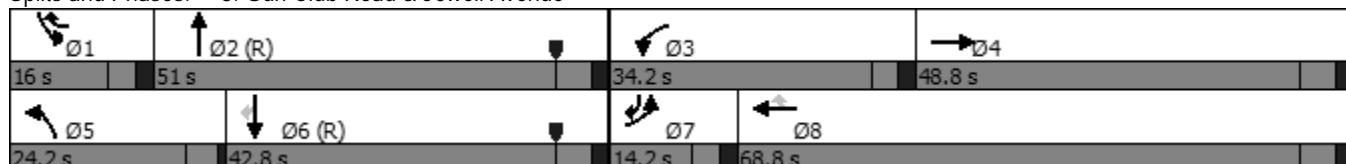
Actuated Cycle Length: 150

Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBT, Start of Yellow

Natural Cycle: 150

Control Type: Actuated-Coordinated

Splits and Phases: 3: Gun Club Road & Jewell Avenue





Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	372	1133	418	490	2403	515	403	1612	337	291	1041	316
v/c Ratio	1.84	0.73	0.27	0.90	1.15	0.59	0.91	1.07	0.21	1.21	0.87	0.52
Control Delay	432.9	49.3	0.4	81.7	113.7	22.6	89.6	92.9	0.3	183.2	63.2	26.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	432.9	49.3	0.4	81.7	113.7	22.6	89.6	92.9	0.3	183.2	63.2	26.0
Queue Length 50th (ft)	~282	362	0	245	~1009	277	203	~636	0	~178	361	146
Queue Length 95th (ft)	#389	438	0	300	#1095	396	#298	#732	0	#277	420	244
Internal Link Dist (ft)			2523			1479			1470			790
Turn Bay Length (ft)	420		330	300		565	485		300	200		325
Base Capacity (vph)	202	1556	1524	655	2088	868	446	1510	1568	240	1195	608
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	1.84	0.73	0.27	0.75	1.15	0.59	0.90	1.07	0.21	1.21	0.87	0.52

Intersection Summary

- ~ Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

HCM 6th Signalized Intersection Summary
11/22/2023

3: Gun Club Road & Jewell Avenue
2050 Background - AM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑↑	↑↑↑	↑	↑↑	↑↑↑	↑	↑↑	↑↑↑	↑	↑↑	↑↑↑	↑
Traffic Volume (veh/h)	365	1110	410	480	2355	505	395	1580	330	285	1020	310
Future Volume (veh/h)	365	1110	410	480	2355	505	395	1580	330	285	1020	310
Initial Q (Q _b), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No		No		No		No		No		No	
Adj Sat Flow, veh/h/ln	1811	1811	1811	1841	1841	1841	1856	1856	1856	1796	1796	1796
Adj Flow Rate, veh/h	372	1133	0	490	2403	515	403	1612	0	291	1041	316
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Percent Heavy Veh, %	6	6	6	4	4	4	3	3	3	7	7	7
Cap, veh/h	205	1603		530	2104	767	444	1520		243	1211	469
Arrive On Green	0.06	0.32	0.00	0.16	0.42	0.42	0.13	0.30	0.00	0.07	0.25	0.25
Sat Flow, veh/h	3346	4944	1535	3401	5025	1560	3428	5066	1572	3319	4904	1522
Grp Volume(v), veh/h	372	1133	0	490	2403	515	403	1612	0	291	1041	316
Grp Sat Flow(s), veh/h/ln	1673	1648	1535	1700	1675	1560	1714	1689	1572	1659	1635	1522
Q Serve(g_s), s	9.2	30.1	0.0	21.3	62.8	37.6	17.4	45.0	0.0	11.0	30.4	27.2
Cycle Q Clear(g_c), s	9.2	30.1	0.0	21.3	62.8	37.6	17.4	45.0	0.0	11.0	30.4	27.2
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	205	1603		530	2104	767	444	1520		243	1211	469
V/C Ratio(X)	1.81	0.71		0.92	1.14	0.67	0.91	1.06		1.20	0.86	0.67
Avail Cap(c_a), veh/h	205	1603		662	2104	767	450	1520		243	1211	469
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	0.82	0.82	0.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	70.4	44.4	0.0	62.4	43.6	28.9	64.4	52.5	0.0	69.5	54.0	45.3
Incr Delay (d2), s/veh	381.1	1.9	0.0	15.1	70.2	4.0	21.7	41.1	0.0	121.0	8.1	7.5
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	14.7	12.3	0.0	10.2	38.6	14.4	8.7	24.0	0.0	8.6	13.0	11.1
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	451.5	46.3	0.0	77.5	113.8	32.9	86.1	93.6	0.0	190.5	62.0	52.8
LnGrp LOS	F	D		E	F	C	F	F		F	E	D
Approach Vol, veh/h		1505			3408			2015		1648		
Approach Delay, s/veh		146.5			96.4			92.1		82.9		
Approach LOS		F			F			F		F		
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	16.0	51.0	28.4	54.6	23.9	43.1	14.2	68.8				
Change Period (Y+Rc), s	5.0	6.0	5.0	6.0	4.5	6.0	5.0	6.0				
Max Green Setting (Gmax), s	11.0	45.0	29.2	42.8	19.7	36.8	9.2	62.8				
Max Q Clear Time (g_c+l1), s	13.0	47.0	23.3	32.1	19.4	32.4	11.2	64.8				
Green Ext Time (p_c), s	0.0	0.0	0.1	8.3	0.1	3.8	0.0	0.0				

Intersection Summary

HCM 6th Ctrl Delay 101.6
HCM 6th LOS F

Notes

User approved pedestrian interval to be less than phase max green.

Unsignalized Delay for [NBR, EBR] is excluded from calculations of the approach delay and intersection delay.

Intersection

Int Delay, s/veh 0.2

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	0	5	50	2305	1755	100
Future Vol, veh/h	0	5	50	2305	1755	100
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	250	-	-	250
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	98	98	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	5	54	2352	1791	109

Major/Minor	Minor2	Major1	Major2		
Conflicting Flow All	-	896	1900	0	-
Stage 1	-	-	-	-	-
Stage 2	-	-	-	-	-
Critical Hdwy	-	7.14	5.34	-	-
Critical Hdwy Stg 1	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-
Follow-up Hdwy	-	3.92	3.12	-	-
Pot Cap-1 Maneuver	0	*513	*645	-	-
Stage 1	0	-	-	-	-
Stage 2	0	-	-	-	-
Platoon blocked, %	1	1	-	-	-
Mov Cap-1 Maneuver	-	*513	*645	-	-
Mov Cap-2 Maneuver	-	-	-	-	-
Stage 1	-	-	-	-	-
Stage 2	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	12.1	0.3	0
HCM LOS	B		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	* 645	-	513	-	-
HCM Lane V/C Ratio	0.084	-	0.011	-	-
HCM Control Delay (s)	11.1	-	12.1	-	-
HCM Lane LOS	B	-	B	-	-
HCM 95th %tile Q(veh)	0.3	-	0	-	-

Notes

~: Volume exceeds capacity \$: Delay exceeds 300s +: Computation Not Defined *: All major volume in platoon

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑	↑, ↓	↑	↑	↑, ↑, ↑	↑	↑, ↓	↑, ↑, ↑	↑
Traffic Volume (vph)	50	5	340	5	100	2095	150	300	1335	125
Future Volume (vph)	50	5	340	5	100	2095	150	300	1335	125
Turn Type	pm+pt	NA	pm+pt	NA	pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases	7	4	3	8	5	2		1	6	
Permitted Phases						2		2	6	
Detector Phase	7	4	3	8	5	2	2	1	6	6
Switch Phase										
Minimum Initial (s)	5.0	8.0	5.0	8.0	5.0	8.0	8.0	5.0	8.0	8.0
Minimum Split (s)	11.0	33.0	11.0	33.0	11.0	30.0	30.0	11.0	30.0	30.0
Total Split (s)	11.0	33.0	11.0	33.0	15.0	59.0	59.0	17.0	61.0	61.0
Total Split (%)	9.2%	27.5%	9.2%	27.5%	12.5%	49.2%	49.2%	14.2%	50.8%	50.8%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0
Lead/Lag	Lead	Lag	Lead	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None	None	Max	None	C-Max	C-Max	None	C-Max	C-Max

Intersection Summary

Cycle Length: 120

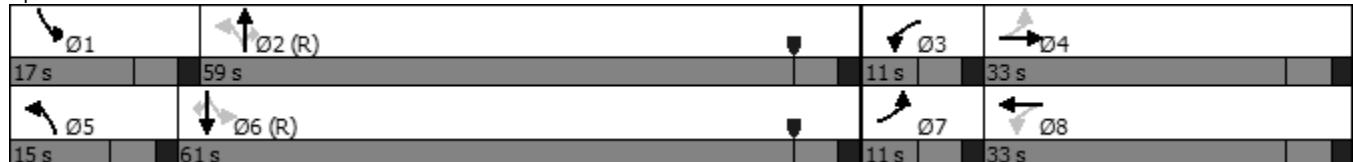
Actuated Cycle Length: 120

Offset: 53 (44%), Referenced to phase 2:NBT and 6:SBTL, Start of Yellow

Natural Cycle: 95

Control Type: Actuated-Coordinated

Splits and Phases: 102: Gun Club Road & Yale Avenue





Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	54	16	370	233	109	2138	163	326	1362	136
v/c Ratio	0.31	0.07	0.43	0.43	0.49	0.94	0.21	0.76	0.58	0.17
Control Delay	43.1	23.3	36.2	10.5	19.8	41.8	5.7	34.4	24.8	3.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	43.1	23.3	36.2	10.5	19.8	41.8	5.7	34.4	24.8	3.6
Queue Length 50th (ft)	42	4	112	18	35	572	12	72	277	0
Queue Length 95th (ft)	61	22	155	89	62	#694	52	123	327	35
Internal Link Dist (ft)		816		845		4562			1641	
Turn Bay Length (ft)	150				250		250	250		250
Base Capacity (vph)	176	384	865	540	233	2267	781	443	2364	808
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.31	0.04	0.43	0.43	0.47	0.94	0.21	0.74	0.58	0.17

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM 6th Signalized Intersection Summary
11/22/2023

102: Gun Club Road & Yale Avenue
2050 Background - AM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑		↑↑	↑		↑	↑↑↑	↑	↑↑	↑↑↑	↑
Traffic Volume (veh/h)	50	5	10	340	5	210	100	2095	150	300	1335	125
Future Volume (veh/h)	50	5	10	340	5	210	100	2095	150	300	1335	125
Initial Q (Q _b), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00			1.00	1.00		1.00	1.00	1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	54	5	0	370	5	0	109	2138	163	326	1362	136
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.98	0.92	0.92	0.98	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	430	408		855	421		252	2405	747	387	2517	781
Arrive On Green	0.03	0.22	0.00	0.04	0.22	0.00	0.05	0.47	0.47	0.07	0.49	0.49
Sat Flow, veh/h	1781	1870	0	3456	1870	0	1781	5106	1585	3456	5106	1585
Grp Volume(v), veh/h	54	5	0	370	5	0	109	2138	163	326	1362	136
Grp Sat Flow(s), veh/h/ln	1781	1870	0	1728	1870	0	1781	1702	1585	1728	1702	1585
Q Serve(g_s), s	2.8	0.3	0.0	5.0	0.2	0.0	3.8	45.7	7.3	6.0	22.1	5.7
Cycle Q Clear(g_c), s	2.8	0.3	0.0	5.0	0.2	0.0	3.8	45.7	7.3	6.0	22.1	5.7
Prop In Lane	1.00			1.00			1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	430	408		855	421		252	2405	747	387	2517	781
V/C Ratio(X)	0.13	0.01		0.43	0.01		0.43	0.89	0.22	0.84	0.54	0.17
Avail Cap(c_a), veh/h	442	421		855	421		302	2405	747	465	2517	781
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	34.5	36.8	0.0	37.3	36.1	0.0	17.1	28.9	18.7	27.9	21.0	16.9
Incr Delay (d2), s/veh	0.1	0.0	0.0	0.3	0.1	0.0	1.2	5.4	0.7	11.4	0.8	0.5
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	1.2	0.1	0.0	2.2	0.1	0.0	1.5	17.9	2.8	2.8	8.2	2.2
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	34.7	36.8	0.0	37.6	36.2	0.0	18.3	34.3	19.4	39.3	21.9	17.4
LnGrp LOS	C	D		D	D		B	C	B	D	C	B
Approach Vol, veh/h		59			375			2410			1824	
Approach Delay, s/veh		34.8			37.6			32.5			24.7	
Approach LOS		C			D			C			C	

Intersection Summary

HCM 6th Ctrl Delay 29.9
HCM 6th LOS C

Notes

Unsignalized Delay for [EBR, WBR] is excluded from calculations of the approach delay and intersection delay.



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↑ ↗	↗ ↗	↑↑↑	↗	↖	↑↑↑
Traffic Volume (vph)	25	50	2255	50	80	1830
Future Volume (vph)	25	50	2255	50	80	1830
Turn Type	Prot	Free	NA	Perm	pm+pt	NA
Protected Phases	7		2		1	6
Permitted Phases		Free			2	6
Detector Phase	7		2	2	1	6
Switch Phase						
Minimum Initial (s)	5.0		8.0	8.0	5.0	8.0
Minimum Split (s)	11.0		28.0	28.0	11.0	28.0
Total Split (s)	11.0		66.0	66.0	13.0	79.0
Total Split (%)	12.2%		73.3%	73.3%	14.4%	87.8%
Yellow Time (s)	4.0		4.0	4.0	4.0	4.0
All-Red Time (s)	2.0		2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0		0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0		6.0	6.0	6.0	6.0
Lead/Lag		Lag	Lag	Lead		
Lead-Lag Optimize?		Yes	Yes	Yes		
Recall Mode	None	C-Max	C-Max	None	C-Max	

Intersection Summary

Cycle Length: 90

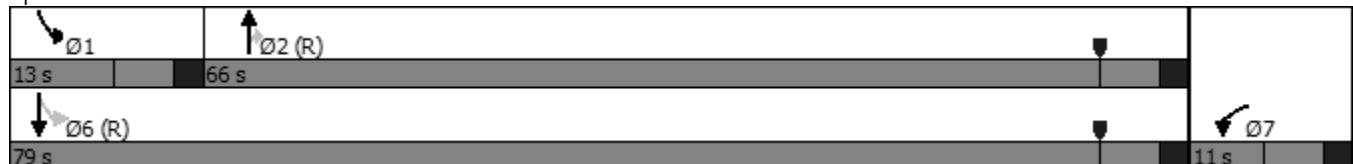
Actuated Cycle Length: 90

Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBTL, Start of Yellow

Natural Cycle: 60

Control Type: Actuated-Coordinated

Splits and Phases: 103: Gun Club Road & Addison Avenue





Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Group Flow (vph)	27	54	2301	54	87	1867
v/c Ratio	0.28	0.03	0.57	0.04	0.42	0.40
Control Delay	48.1	0.0	6.3	1.6	14.0	1.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	48.1	0.0	6.3	1.6	14.0	1.5
Queue Length 50th (ft)	15	0	126	0	0	0
Queue Length 95th (ft)	42	0	304	11	45	99
Internal Link Dist (ft)	1036		985			915
Turn Bay Length (ft)				300	300	
Base Capacity (vph)	98	1583	4009	1259	221	4701
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.28	0.03	0.57	0.04	0.39	0.40

Intersection Summary



Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	25	50	2255	50	80	1830
Future Volume (veh/h)	25	50	2255	50	80	1830
Initial Q (Q _b), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00	1.00		1.00	1.00	
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No		No		No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	27	0	2301	54	87	1867
Peak Hour Factor	0.92	0.92	0.98	0.92	0.92	0.98
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	49		3694	1147	243	4286
Arrive On Green	0.03	0.00	0.72	0.72	0.05	0.84
Sat Flow, veh/h	1781	1585	5274	1585	1781	5274
Grp Volume(v), veh/h	27	0	2301	54	87	1867
Grp Sat Flow(s), veh/h/ln	1781	1585	1702	1585	1781	1702
Q Serve(g_s), s	1.3	0.0	20.4	0.9	0.9	8.3
Cycle Q Clear(g_c), s	1.3	0.0	20.4	0.9	0.9	8.3
Prop In Lane	1.00	1.00		1.00	1.00	
Lane Grp Cap(c), veh/h	49		3694	1147	243	4286
V/C Ratio(X)	0.56		0.62	0.05	0.36	0.44
Avail Cap(c_a), veh/h	99		3694	1147	294	4286
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	0.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	43.2	0.0	6.3	3.6	8.5	1.8
Incr Delay (d2), s/veh	9.6	0.0	0.8	0.1	0.9	0.3
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	0.7	0.0	4.3	0.2	0.5	0.1
Unsig. Movement Delay, s/veh						
LnGrp Delay(d), s/veh	52.8	0.0	7.1	3.6	9.4	2.2
LnGrp LOS	D		A	A	A	
Approach Vol, veh/h	27		2355		1954	
Approach Delay, s/veh	52.8		7.0		2.5	
Approach LOS	D		A		A	
Timer - Assigned Phs	1	2		4		6
Phs Duration (G+Y+R _c), s	10.4	71.1		8.5	81.5	
Change Period (Y+R _c), s	6.0	6.0		6.0	6.0	
Max Green Setting (Gmax), s	7.0	60.0		5.0	73.0	
Max Q Clear Time (g_c+l1), s	2.9	22.4		3.3	10.3	
Green Ext Time (p_c), s	0.1	24.0		0.0	20.7	

Intersection Summary

HCM 6th Ctrl Delay	5.2
HCM 6th LOS	A

Notes

Unsignalized Delay for [WBR] is excluded from calculations of the approach delay and intersection delay.



Lane Group	EBT	EBR	WBL	WBT	SBT	SBR
Lane Configurations	↑↑↑	↑↓	↑↑↓	↑↑↑	↑↓	↑
Traffic Volume (vph)	2695	330	325	1865	5	355
Future Volume (vph)	2695	330	325	1865	5	355
Turn Type	NA	Perm	pm+pt	NA	NA	Perm
Protected Phases	4			3	8	6
Permitted Phases				4	8	6
Detector Phase	4	4	3	8	6	6
Switch Phase						
Minimum Initial (s)	8.0	8.0	4.0	8.0	8.0	8.0
Minimum Split (s)	25.0	25.0	12.5	25.0	27.0	27.0
Total Split (s)	61.0	61.0	14.0	75.0	35.0	35.0
Total Split (%)	55.5%	55.5%	12.7%	68.2%	31.8%	31.8%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	6.0
Lead/Lag	Lag	Lag	Lead			
Lead-Lag Optimize?	Yes	Yes	Yes			
Recall Mode	C-Max	C-Max	None	C-Max	None	None

Intersection Summary

Cycle Length: 110

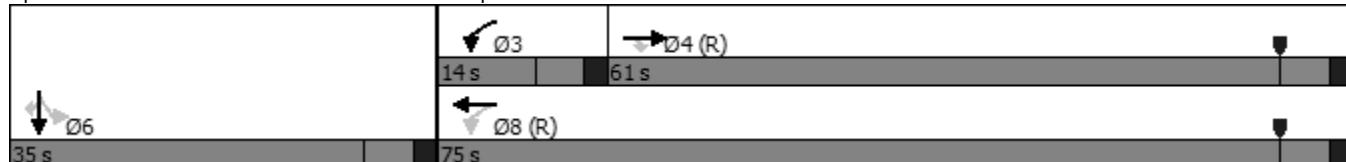
Actuated Cycle Length: 110

Offset: 0 (0%), Referenced to phase 4:EBT and 8:WBTL, Start of Yellow

Natural Cycle: 90

Control Type: Actuated-Coordinated

Splits and Phases: 1: E-470 Southbound Ramps & Jewell Avenue





Lane Group	EBT	EBR	WBL	WBT	SBT	SBR
Lane Group Flow (vph)	2750	337	332	1903	245	362
v/c Ratio	1.02	0.35	0.82	0.56	0.63	0.87
Control Delay	50.4	4.2	49.2	6.5	45.4	52.2
Queue Delay	0.0	0.0	0.0	0.1	0.0	0.0
Total Delay	50.4	4.2	49.2	6.6	45.4	52.2
Queue Length 50th (ft)	~797	18	96	100	152	186
Queue Length 95th (ft)	#889	67	m#121	165	232	#324
Internal Link Dist (ft)	679			530	532	
Turn Bay Length (ft)		300	250			
Base Capacity (vph)	2689	971	406	3375	450	467
Starvation Cap Reductn	0	0	0	220	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	1.02	0.35	0.82	0.60	0.54	0.78

Intersection Summary

- ~ Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.
- m Volume for 95th percentile queue is metered by upstream signal.

HCM 6th Signalized Intersection Summary
11/22/2023

1: E-470 Southbound Ramps & Jewell Avenue
2050 Background - PM Peak Hour



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	0	2695	330	325	1865	0	0	0	0	235	5	355
Future Volume (veh/h)	0	2695	330	325	1865	0	0	0	0	235	5	355
Initial Q (Q _b), veh	0	0	0	0	0	0				0	0	0
Ped-Bike Adj(A_pbT)	1.00			1.00	1.00					1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00				1.00	1.00	1.00
Work Zone On Approach		No			No						No	
Adj Sat Flow, veh/h/ln	0	1870	1870	1870	1870	0				1811	1811	1811
Adj Flow Rate, veh/h	0	2750	337	332	1903	0				240	5	0
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98				0.98	0.98	0.98
Percent Heavy Veh, %	0	2	2	2	2	0				6	6	6
Cap, veh/h	0	3078	956	385	3703	0				280	6	
Arrive On Green	0.00	0.60	0.60	0.14	1.00	0.00				0.17	0.17	0.00
Sat Flow, veh/h	0	5274	1585	3456	5274	0				1691	35	1535
Grp Volume(v), veh/h	0	2750	337	332	1903	0				245	0	0
Grp Sat Flow(s), veh/h/ln	0	1702	1585	1728	1702	0				1727	0	1535
Q Serve(g_s), s	0.0	51.0	11.8	5.4	0.0	0.0				15.2	0.0	0.0
Cycle Q Clear(g_c), s	0.0	51.0	11.8	5.4	0.0	0.0				15.2	0.0	0.0
Prop In Lane	0.00		1.00	1.00		0.00				0.98		1.00
Lane Grp Cap(c), veh/h	0	3078	956	385	3703	0				286	0	
V/C Ratio(X)	0.00	0.89	0.35	0.86	0.51	0.00				0.86	0.00	
Avail Cap(c_a), veh/h	0	3078	956	402	3703	0				455	0	
HCM Platoon Ratio	1.00	1.00	1.00	2.00	2.00	1.00				1.00	1.00	1.00
Upstream Filter(l)	0.00	1.00	1.00	0.34	0.34	0.00				1.00	0.00	0.00
Uniform Delay (d), s/veh	0.0	18.8	11.0	29.5	0.0	0.0				44.6	0.0	0.0
Incr Delay (d2), s/veh	0.0	4.5	1.0	6.6	0.2	0.0				9.1	0.0	0.0
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0				0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	0.0	18.3	3.9	4.3	0.1	0.0				7.2	0.0	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	0.0	23.3	12.0	36.0	0.2	0.0				53.7	0.0	0.0
LnGrp LOS	A	C	B	D	A	A				D	A	
Approach Vol, veh/h		3087			2235					245		
Approach Delay, s/veh		22.1			5.5					53.7		
Approach LOS		C			A					D		
Timer - Assigned Phs		3	4		6		8					
Phs Duration (G+Y+R _c), s		13.5	72.3		24.2		85.8					
Change Period (Y+R _c), s		6.0	6.0		6.0		6.0					
Max Green Setting (Gmax), s		8.0	55.0		29.0		69.0					
Max Q Clear Time (g _{c+l1}), s		7.4	53.0		17.2		2.0					
Green Ext Time (p _c), s		0.1	2.0		1.1		23.9					
Intersection Summary												
HCM 6th Ctrl Delay		16.8										
HCM 6th LOS			B									
Notes												

Unsignalized Delay for [SBR] is excluded from calculations of the approach delay and intersection delay.



Lane Group	EBL	EBT	WBT	WBR	NBT	NBR
Lane Configurations	↑↑	↑↑↑	↑↑↑	↑	↑	↑
Traffic Volume (vph)	275	2655	1915	310	0	370
Future Volume (vph)	275	2655	1915	310	0	370
Turn Type	pm+pt	NA	NA	Perm	NA	Perm
Protected Phases	7	4	8		2	
Permitted Phases	4			8		2
Detector Phase	7	4	8	8	2	2
Switch Phase						
Minimum Initial (s)	3.0	8.0	8.0	8.0	8.0	8.0
Minimum Split (s)	9.5	25.0	25.0	25.0	27.0	27.0
Total Split (s)	13.0	69.0	56.0	56.0	41.0	41.0
Total Split (%)	11.8%	62.7%	50.9%	50.9%	37.3%	37.3%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	6.0
Lead/Lag	Lead		Lag	Lag		
Lead-Lag Optimize?	Yes		Yes	Yes		
Recall Mode	None	C-Max	C-Max	C-Max	Max	Max

Intersection Summary

Cycle Length: 110

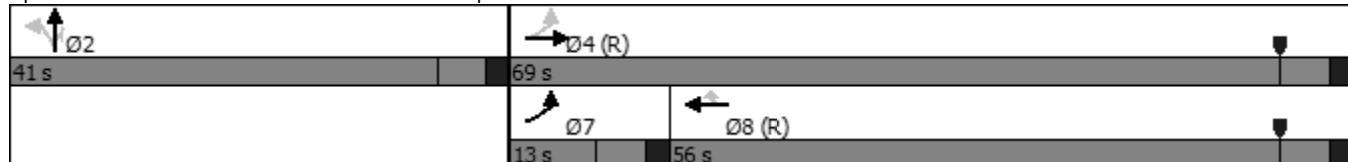
Actuated Cycle Length: 110

Offset: 0 (0%), Referenced to phase 4:EBTL and 8:WBT, Start of Yellow

Natural Cycle: 80

Control Type: Actuated-Coordinated

Splits and Phases: 2: E-470 Northbound Ramps & Jewell Avenue





Lane Group	EBL	EBT	WBT	WBR	NBT	NBR
Lane Group Flow (vph)	281	2709	1954	316	281	378
v/c Ratio	0.82	0.95	0.89	0.37	0.51	0.69
Control Delay	42.5	14.1	33.6	4.2	34.6	32.1
Queue Delay	0.0	1.5	0.0	0.0	0.0	0.0
Total Delay	42.5	15.6	33.6	4.2	34.6	32.1
Queue Length 50th (ft)	69	177	450	10	161	179
Queue Length 95th (ft)	m71	m174	523	60	246	292
Internal Link Dist (ft)		530	2523		717	
Turn Bay Length (ft)	250				175	
Base Capacity (vph)	342	2856	2203	844	546	550
Starvation Cap Reductn	0	60	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.82	0.97	0.89	0.37	0.51	0.69

Intersection Summary

m Volume for 95th percentile queue is metered by upstream signal.

HCM 6th Signalized Intersection Summary
11/22/2023

2: E-470 Northbound Ramps & Jewell Avenue
2050 Background - PM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑↑	↑↑↑			↑↑↑	↑		↑	↑	0	0	0
Traffic Volume (veh/h)	275	2655	0	0	1915	310	275	0	370	0	0	0
Future Volume (veh/h)	275	2655	0	0	1915	310	275	0	370	0	0	0
Initial Q (Q _b), veh	0	0	0	0	0	0	0	0	0			
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00			
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Work Zone On Approach		No			No			No				
Adj Sat Flow, veh/h/ln	1841	1841	0	0	1796	1796	1826	1826	1826			
Adj Flow Rate, veh/h	281	2709	0	0	1954	316	281	0	0			
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98			
Percent Heavy Veh, %	4	4	0	0	7	7	5	5	5			
Cap, veh/h	370	2878	0	0	2240	695	553	0				
Arrive On Green	0.12	1.00	0.00	0.00	0.46	0.46	0.32	0.00	0.00			
Sat Flow, veh/h	3401	5191	0	0	5065	1522	1739	0	1547			
Grp Volume(v), veh/h	281	2709	0	0	1954	316	281	0	0			
Grp Sat Flow(s), veh/h/ln	1700	1675	0	0	1635	1522	1739	0	1547			
Q Serve(g_s), s	4.7	0.0	0.0	0.0	39.6	15.7	14.5	0.0	0.0			
Cycle Q Clear(g_c), s	4.7	0.0	0.0	0.0	39.6	15.7	14.5	0.0	0.0			
Prop In Lane	1.00		0.00	0.00		1.00	1.00		1.00			
Lane Grp Cap(c), veh/h	370	2878	0	0	2240	695	553	0				
V/C Ratio(X)	0.76	0.94	0.00	0.00	0.87	0.45	0.51	0.00				
Avail Cap(c_a), veh/h	378	2878	0	0	2240	695	553	0				
HCM Platoon Ratio	2.00	2.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Upstream Filter(l)	0.09	0.09	0.00	0.00	0.17	0.17	1.00	0.00	0.00			
Uniform Delay (d), s/veh	23.1	0.0	0.0	0.0	27.0	20.5	30.5	0.0	0.0			
Incr Delay (d2), s/veh	0.8	0.9	0.0	0.0	0.9	0.4	3.3	0.0	0.0			
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0			
%ile BackOfQ(50%), veh/ln	1.5	0.2	0.0	0.0	14.3	5.3	6.5	0.0	0.0			
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	23.9	0.9	0.0	0.0	27.9	20.8	33.8	0.0	0.0			
LnGrp LOS	C	A	A	A	C	C	C	A				
Approach Vol, veh/h	2990				2270				281			
Approach Delay, s/veh	3.0				26.9				33.8			
Approach LOS	A				C				C			
Timer - Assigned Phs	2		4			7		8				
Phs Duration (G+Y+Rc), s	41.0		69.0			12.7		56.3				
Change Period (Y+Rc), s	6.0		6.0			6.0		6.0				
Max Green Setting (Gmax), s	35.0		63.0			7.0		50.0				
Max Q Clear Time (g_c+l1), s	16.5		2.0			6.7		41.6				
Green Ext Time (p_c), s	1.6		43.5			0.0		7.1				
Intersection Summary												
HCM 6th Ctrl Delay			14.4									
HCM 6th LOS			B									
Notes												
User approved pedestrian interval to be less than phase max green.												
Unsignalized Delay for [NBR] is excluded from calculations of the approach delay and intersection delay.												

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑↑	↑↑↑	↑	↑↑	↑↑↑	↑	↑↑	↑↑↑	↑	↑↑	↑↑↑	↑
Traffic Volume (vph)	495	2205	325	470	1705	470	270	1315	495	505	1545	250
Future Volume (vph)	495	2205	325	470	1705	470	270	1315	495	505	1545	250
Turn Type	Prot	NA	Free	Prot	NA	pm+ov	Prot	NA	Free	Prot	NA	pm+ov
Protected Phases	7	4		3	8	1	5	2		1	6	7
Permitted Phases			Free			8			Free			6
Detector Phase	7	4		3	8	1	5	2		1	6	7
Switch Phase												
Minimum Initial (s)	3.0	4.0		3.0	5.0	4.0	4.0	8.0		4.0	8.0	3.0
Minimum Split (s)	9.5	32.0		9.5	32.0	9.5	9.5	40.0		9.5	29.0	9.5
Total Split (s)	26.8	64.0		16.0	53.2	22.0	21.0	48.0		22.0	49.0	26.8
Total Split (%)	17.9%	42.7%		10.7%	35.5%	14.7%	14.0%	32.0%		14.7%	32.7%	17.9%
Yellow Time (s)	3.0	3.0		3.0	3.0	3.0	3.0	4.0		3.0	4.0	3.0
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	2.0	2.0		2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0		5.0	5.0	5.0	5.0	6.0		5.0	6.0	5.0
Lead/Lag	Lead	Lag		Lead	Lag	Lead	Lead	Lag		Lead	Lag	Lead
Lead-Lag Optimize?	Yes	Yes		Yes	Yes	Yes	Yes	Yes		Yes	Yes	Yes
Recall Mode	None	None		None	None	None	None	C-Max		None	C-Max	None

Intersection Summary

Cycle Length: 150

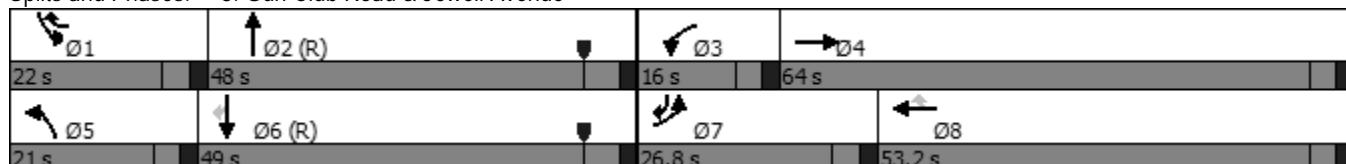
Actuated Cycle Length: 150

Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBT, Start of Yellow

Natural Cycle: 145

Control Type: Actuated-Coordinated

Splits and Phases: 3: Gun Club Road & Jewell Avenue





Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	505	2250	332	480	1740	480	276	1342	505	515	1577	255
v/c Ratio	1.03	1.15	0.21	1.95	1.09	0.62	0.82	0.99	0.33	1.31	1.06	0.32
Control Delay	110.5	114.1	0.3	475.8	97.1	27.5	84.9	75.2	0.6	207.7	91.0	20.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	110.5	114.1	0.3	475.8	97.1	27.5	84.9	75.2	0.6	207.7	91.0	20.6
Queue Length 50th (ft)	~272	~942	0	~372	~697	280	138	481	0	~332	~624	121
Queue Length 95th (ft)	#389	#1031	0	#488	#792	404	#203	#590	0	#451	#721	189
Internal Link Dist (ft)		2523			1479			1470			790	
Turn Bay Length (ft)	420		330	300		565	485		300	200		325
Base Capacity (vph)	489	1961	1553	246	1602	777	349	1357	1509	392	1489	786
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	1.03	1.15	0.21	1.95	1.09	0.62	0.79	0.99	0.33	1.31	1.06	0.32

Intersection Summary

- ~ Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

HCM 6th Signalized Intersection Summary
11/22/2023

3: Gun Club Road & Jewell Avenue
2050 Background - PM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑↑	↑↑↑	↑	↑↑	↑↑↑	↑	↑↑	↑↑↑	↑	↑↑	↑↑↑	↑
Traffic Volume (veh/h)	495	2205	325	470	1705	470	270	1315	495	505	1545	250
Future Volume (veh/h)	495	2205	325	470	1705	470	270	1315	495	505	1545	250
Initial Q (Q _b), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No			No			No		
Adj Sat Flow, veh/h/ln	1841	1841	1841	1841	1841	1841	1796	1796	1796	1885	1885	1885
Adj Flow Rate, veh/h	505	2250	0	480	1740	480	276	1342	0	515	1577	255
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Percent Heavy Veh, %	4	4	4	4	4	4	7	7	7	1	1	1
Cap, veh/h	494	1977		249	1615	678	320	1373		395	1528	707
Arrive On Green	0.15	0.39	0.00	0.07	0.32	0.32	0.10	0.28	0.00	0.11	0.30	0.30
Sat Flow, veh/h	3401	5025	1560	3401	5025	1560	3319	4904	1522	3483	5147	1598
Grp Volume(v), veh/h	505	2250	0	480	1740	480	276	1342	0	515	1577	255
Grp Sat Flow(s), veh/h/ln	1700	1675	1560	1700	1675	1560	1659	1635	1522	1742	1716	1598
Q Serve(g_s), s	21.8	59.0	0.0	11.0	48.2	37.7	12.3	40.7	0.0	17.0	44.5	15.9
Cycle Q Clear(g_c), s	21.8	59.0	0.0	11.0	48.2	37.7	12.3	40.7	0.0	17.0	44.5	15.9
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	494	1977		249	1615	678	320	1373		395	1528	707
V/C Ratio(X)	1.02	1.14		1.92	1.08	0.71	0.86	0.98		1.30	1.03	0.36
Avail Cap(c_a), veh/h	494	1977		249	1615	678	354	1373		395	1528	707
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	0.28	0.28	0.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	64.1	45.5	0.0	69.5	50.9	34.6	66.8	53.5	0.0	66.5	52.7	27.8
Incr Delay (d2), s/veh	27.1	64.3	0.0	430.6	46.5	3.4	18.0	19.5	0.0	154.4	31.6	1.4
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	11.1	35.3	0.0	19.6	26.6	14.5	5.9	18.5	0.0	15.8	22.8	6.2
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	91.2	109.8	0.0	500.1	97.4	38.0	84.8	73.0	0.0	220.9	84.4	29.2
LnGrp LOS	F	F		F	F	D	F	E		F	F	C
Approach Vol, veh/h		2755			2700			1618		2347		
Approach Delay, s/veh		106.4			158.4			75.0		108.3		
Approach LOS		F			F			E		F		
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	22.0	48.0	16.0	64.0	19.5	50.5	26.8	53.2				
Change Period (Y+Rc), s	5.0	6.0	5.0	5.0	5.0	6.0	5.0	5.0				
Max Green Setting (Gmax), s	17.0	42.0	11.0	59.0	16.0	43.0	21.8	48.2				
Max Q Clear Time (g_c+l1), s	19.0	42.7	13.0	61.0	14.3	46.5	23.8	50.2				
Green Ext Time (p_c), s	0.0	0.0	0.0	0.0	0.2	0.0	0.0	0.0				

Intersection Summary

HCM 6th Ctrl Delay 116.4
HCM 6th LOS F

Notes

User approved pedestrian interval to be less than phase max green.

Unsignalized Delay for [NBR, EBR] is excluded from calculations of the approach delay and intersection delay.

Intersection

Int Delay, s/veh 0.1

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations			↑	↑↑↑↑	↑↑↑↑	↑
Traffic Vol, veh/h	0	20	5	2025	2285	15
Future Vol, veh/h	0	20	5	2025	2285	15
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	250	-	-	250
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	98	98	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	22	5	2066	2332	16

Major/Minor	Minor2	Major1	Major2		
Conflicting Flow All	-	1166	2348	0	-
Stage 1	-	-	-	-	-
Stage 2	-	-	-	-	-
Critical Hdwy	-	7.14	5.34	-	-
Critical Hdwy Stg 1	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-
Follow-up Hdwy	-	3.92	3.12	-	-
Pot Cap-1 Maneuver	0	*399	*501	-	-
Stage 1	0	-	-	-	-
Stage 2	0	-	-	-	-
Platoon blocked, %	1	1	-	-	-
Mov Cap-1 Maneuver	-	*399	*501	-	-
Mov Cap-2 Maneuver	-	-	-	-	-
Stage 1	-	-	-	-	-
Stage 2	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	14.5	0	0
HCM LOS	B		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	* 501	-	399	-	-
HCM Lane V/C Ratio	0.011	-	0.054	-	-
HCM Control Delay (s)	12.3	-	14.5	-	-
HCM Lane LOS	B	-	B	-	-
HCM 95th %tile Q(veh)	0	-	0.2	-	-

Notes

~: Volume exceeds capacity \$: Delay exceeds 300s +: Computation Not Defined *: All major volume in platoon

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑	↑	↑	↑	↑↑↑	↑	↑	↑↑↑	↑
Traffic Volume (vph)	305	5	460	5	15	1405	325	450	1835	20
Future Volume (vph)	305	5	460	5	15	1405	325	450	1835	20
Turn Type	pm+pt	NA	pm+pt	NA	pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases	7	4	3	8	5	2		1	6	
Permitted Phases						2		2	6	
Detector Phase	7	4	3	8	5	2	2	1	6	6
Switch Phase										
Minimum Initial (s)	5.0	8.0	5.0	8.0	5.0	8.0	8.0	5.0	8.0	8.0
Minimum Split (s)	11.0	33.0	11.0	33.0	11.0	30.0	30.0	11.0	30.0	30.0
Total Split (s)	21.0	37.0	17.0	33.0	11.0	46.0	46.0	20.0	55.0	55.0
Total Split (%)	17.5%	30.8%	14.2%	27.5%	9.2%	38.3%	38.3%	16.7%	45.8%	45.8%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0
Lead/Lag	Lead	Lag	Lead	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes									
Recall Mode	None	None	None	Max	None	C-Max	C-Max	None	C-Max	C-Max

Intersection Summary

Cycle Length: 120

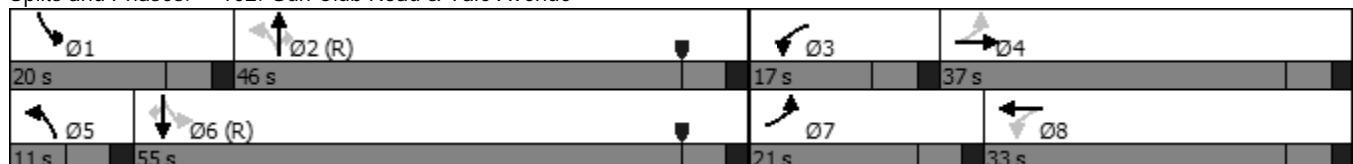
Actuated Cycle Length: 120

Offset: 0 (0%), Referenced to phase 2:NBTL and 6:SBTL, Start of Yellow

Natural Cycle: 105

Control Type: Actuated-Coordinated

Splits and Phases: 102: Gun Club Road & Yale Avenue





Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	332	76	500	353	16	1434	353	489	1872	22
v/c Ratio	1.03	0.19	0.54	0.72	0.12	0.85	0.47	0.93	0.79	0.03
Control Delay	91.1	10.1	29.7	31.1	18.1	42.9	6.2	56.3	31.4	0.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	91.1	10.1	29.7	31.1	18.1	42.9	6.2	56.3	31.4	0.1
Queue Length 50th (ft)	188	3	138	132	6	376	10	142	406	0
Queue Length 95th (ft)	#333	42	182	246	18	438	81	#244	561	0
Internal Link Dist (ft)		816		845		4562			1641	
Turn Bay Length (ft)	150				250		250	250		250
Base Capacity (vph)	322	466	931	489	137	1695	749	523	2356	806
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	1.03	0.16	0.54	0.72	0.12	0.85	0.47	0.93	0.79	0.03

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM 6th Signalized Intersection Summary
11/22/2023

102: Gun Club Road & Yale Avenue
2050 Background - PM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑	↑	↑↑	↑		↑	↑↑↑	↑	↑↑	↑↑↑	↑
Traffic Volume (veh/h)	305	5	65	460	5	320	15	1405	325	450	1835	20
Future Volume (veh/h)	305	5	65	460	5	320	15	1405	325	450	1835	20
Initial Q (Q _b), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00			1.00	1.00		1.00	1.00	1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	332	5	0	500	5	0	16	1434	353	489	1872	22
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.98	0.92	0.92	0.98	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	597	483		1053	421		116	1734	538	543	2210	686
Arrive On Green	0.13	0.26	0.00	0.09	0.22	0.00	0.02	0.34	0.34	0.11	0.43	0.43
Sat Flow, veh/h	1781	1870	0	3456	1870	0	1781	5106	1585	3456	5106	1585
Grp Volume(v), veh/h	332	5	0	500	5	0	16	1434	353	489	1872	22
Grp Sat Flow(s), veh/h/ln	1781	1870	0	1728	1870	0	1781	1702	1585	1728	1702	1585
Q Serve(g_s), s	15.0	0.2	0.0	11.0	0.2	0.0	0.7	30.9	22.7	11.0	39.4	1.0
Cycle Q Clear(g_c), s	15.0	0.2	0.0	11.0	0.2	0.0	0.7	30.9	22.7	11.0	39.4	1.0
Prop In Lane	1.00			0.00	1.00		0.00	1.00	1.00	1.00		1.00
Lane Grp Cap(c), veh/h	597	483		1053	421		116	1734	538	543	2210	686
V/C Ratio(X)	0.56	0.01		0.47	0.01		0.14	0.83	0.66	0.90	0.85	0.03
Avail Cap(c_a), veh/h	597	483		1053	421		159	1734	538	565	2210	686
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	31.2	33.1	0.0	32.8	36.1	0.0	28.6	36.4	33.7	28.9	30.5	19.6
Incr Delay (d2), s/veh	1.1	0.0	0.0	0.3	0.1	0.0	0.5	4.7	6.1	17.0	4.3	0.1
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	7.9	0.1	0.0	5.9	0.1	0.0	0.3	12.7	9.6	5.4	15.6	0.4
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	32.4	33.1	0.0	33.2	36.2	0.0	29.1	41.1	39.8	46.0	34.7	19.7
LnGrp LOS	C	C		C	D		C	D	D	D	C	B
Approach Vol, veh/h		337			505			1803			2383	
Approach Delay, s/veh		32.4			33.2			40.7			36.9	
Approach LOS		C			C			D			D	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	19.2	46.8	17.0	37.0	8.1	57.9	21.0	33.0				
Change Period (Y+Rc), s	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0				
Max Green Setting (Gmax), s	14.0	40.0	11.0	31.0	5.0	49.0	15.0	27.0				
Max Q Clear Time (g_c+l1), s	13.0	32.9	13.0	2.2	2.7	41.4	17.0	2.2				
Green Ext Time (p_c), s	0.2	5.0	0.0	0.0	0.0	5.9	0.0	0.0				
Intersection Summary												
HCM 6th Ctrl Delay		37.6										
HCM 6th LOS			D									
Notes												
Unsignalized Delay for [EBR, WBR] is excluded from calculations of the approach delay and intersection delay.												



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↑ ↘	↑ ↗	↑↑↑	↑ ↗	↖	↑↑↑
Traffic Volume (vph)	90	105	1975	50	130	2210
Future Volume (vph)	90	105	1975	50	130	2210
Turn Type	Prot	Free	NA	Perm	pm+pt	NA
Protected Phases	7		2		1	6
Permitted Phases		Free			2	6
Detector Phase	7		2	2	1	6
Switch Phase						
Minimum Initial (s)	5.0		8.0	8.0	5.0	8.0
Minimum Split (s)	11.0		28.0	28.0	11.0	28.0
Total Split (s)	16.0		58.0	58.0	16.0	74.0
Total Split (%)	17.8%		64.4%	64.4%	17.8%	82.2%
Yellow Time (s)	4.0		4.0	4.0	4.0	4.0
All-Red Time (s)	2.0		2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0		0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0		6.0	6.0	6.0	6.0
Lead/Lag		Lag	Lag	Lead		
Lead-Lag Optimize?		Yes	Yes	Yes		
Recall Mode	None	C-Max	C-Max	None	C-Max	

Intersection Summary

Cycle Length: 90

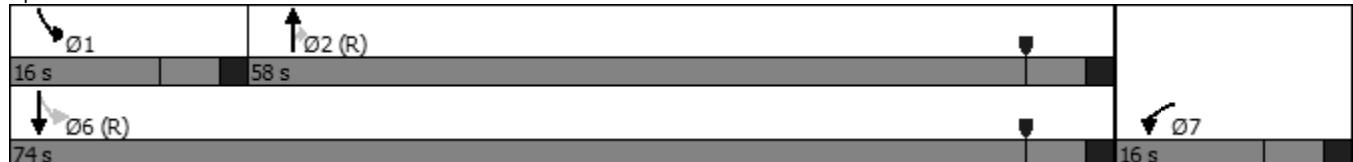
Actuated Cycle Length: 90

Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBTL, Start of Yellow

Natural Cycle: 55

Control Type: Actuated-Coordinated

Splits and Phases: 103: Gun Club Road & Addison Avenue





Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Group Flow (vph)	98	114	2015	54	141	2255
v/c Ratio	0.56	0.07	0.62	0.05	0.59	0.55
Control Delay	50.8	0.1	12.0	2.6	23.1	4.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	50.8	0.1	12.0	2.6	23.1	4.6
Queue Length 50th (ft)	53	0	253	0	27	164
Queue Length 95th (ft)	104	0	324	15	87	196
Internal Link Dist (ft)	1036		985			915
Turn Bay Length (ft)				300	300	
Base Capacity (vph)	196	1583	3247	1030	278	4106
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.50	0.07	0.62	0.05	0.51	0.55

Intersection Summary



Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↖	↗	↑↑↑	↖	↖	↑↑↑
Traffic Volume (veh/h)	90	105	1975	50	130	2210
Future Volume (veh/h)	90	105	1975	50	130	2210
Initial Q (Q _b), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00	1.00		1.00	1.00	
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No		No		No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	98	0	2015	54	141	2255
Peak Hour Factor	0.92	0.92	0.98	0.92	0.92	0.98
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	125		3451	1071	269	4066
Arrive On Green	0.07	0.00	0.68	0.68	0.05	0.80
Sat Flow, veh/h	1781	1585	5274	1585	1781	5274
Grp Volume(v), veh/h	98	0	2015	54	141	2255
Grp Sat Flow(s), veh/h/ln	1781	1585	1702	1585	1781	1702
Q Serve(g_s), s	4.9	0.0	19.0	1.0	1.9	14.5
Cycle Q Clear(g_c), s	4.9	0.0	19.0	1.0	1.9	14.5
Prop In Lane	1.00	1.00		1.00	1.00	
Lane Grp Cap(c), veh/h	125		3451	1071	269	4066
V/C Ratio(X)	0.78		0.58	0.05	0.52	0.55
Avail Cap(c_a), veh/h	198		3451	1071	371	4066
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	0.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	41.2	0.0	7.8	4.9	11.1	3.3
Incr Delay (d2), s/veh	10.1	0.0	0.7	0.1	1.6	0.5
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	2.5	0.0	4.8	0.3	1.0	1.7
Unsig. Movement Delay, s/veh						
LnGrp Delay(d), s/veh	51.3	0.0	8.5	5.0	12.7	3.9
LnGrp LOS	D		A	A	B	A
Approach Vol, veh/h	98		2069			2396
Approach Delay, s/veh	51.3		8.5			4.4
Approach LOS	D		A			A
Timer - Assigned Phs	1	2		4		6
Phs Duration (G+Y+R _c), s	10.9	66.8		12.3		77.7
Change Period (Y+R _c), s	6.0	6.0		6.0		6.0
Max Green Setting (Gmax), s	10.0	52.0		10.0		68.0
Max Q Clear Time (g_c+l1), s	3.9	21.0		6.9		16.5
Green Ext Time (p_c), s	0.2	17.9		0.1		27.5

Intersection Summary

HCM 6th Ctrl Delay	7.2
HCM 6th LOS	A

Notes

User approved pedestrian interval to be less than phase max green.

Unsignalized Delay for [WBR] is excluded from calculations of the approach delay and intersection delay.

***Intersection Capacity Worksheets:
2027 Background
+ Project***



Lane Group	EBT	EBR	WBL	WBT	SBT	SBR
Lane Configurations	↑	↗	↖	↑	↖	↗
Traffic Volume (vph)	706	180	150	816	1	190
Future Volume (vph)	706	180	150	816	1	190
Turn Type	NA	Perm	pm+pt	NA	NA	Perm
Protected Phases	4			3	8	6
Permitted Phases				4	8	6
Detector Phase	4	4	3	8	6	6
Switch Phase						
Minimum Initial (s)	8.0	8.0	4.0	8.0	8.0	8.0
Minimum Split (s)	25.0	25.0	10.0	25.0	27.0	27.0
Total Split (s)	50.0	50.0	13.0	63.0	27.0	27.0
Total Split (%)	55.6%	55.6%	14.4%	70.0%	30.0%	30.0%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	6.0
Lead/Lag	Lag	Lag	Lead			
Lead-Lag Optimize?	Yes	Yes	Yes			
Recall Mode	C-Max	C-Max	None	C-Max	Max	Max

Intersection Summary

Cycle Length: 90

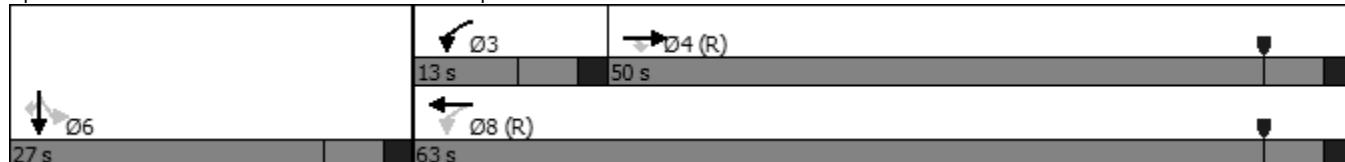
Actuated Cycle Length: 90

Offset: 68 (76%), Referenced to phase 4:EBT and 8:WBTL, Start of Yellow

Natural Cycle: 80

Control Type: Actuated-Coordinated

Splits and Phases: 1: E-470 Southbound Ramps & Jewell Avenue





Lane Group	EBT	EBR	WBL	WBT	SBT	SBR
Lane Group Flow (vph)	743	189	169	917	260	218
v/c Ratio	0.82	0.22	0.60	0.77	0.72	0.49
Control Delay	29.2	2.7	19.1	8.0	44.7	13.1
Queue Delay	0.0	0.0	0.0	0.5	0.0	0.0
Total Delay	29.2	2.7	19.1	8.5	44.7	13.1
Queue Length 50th (ft)	346	0	32	127	137	25
Queue Length 95th (ft)	#565	33	m45	m136	#229	83
Internal Link Dist (ft)	679			530	532	
Turn Bay Length (ft)		300	250			
Base Capacity (vph)	903	864	281	1191	361	446
Starvation Cap Reductn	0	0	0	64	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.82	0.22	0.60	0.81	0.72	0.49

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

HCM 6th Signalized Intersection Summary
11/22/2023

1: E-470 Southbound Ramps & Jewell Avenue
2027 Bkgrd + Proj - AM Peak Hour

Movement	EBL	EBT	EBC	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	0	706	180	150	816	0	0	0	0	225	1	190
Future Volume (veh/h)	0	706	180	150	816	0	0	0	0	225	1	190
Initial Q (Q _b), veh	0	0	0	0	0	0				0	0	0
Ped-Bike Adj(A_pbT)	1.00			1.00			1.00			1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00				1.00	1.00	1.00
Work Zone On Approach		No			No						No	
Adj Sat Flow, veh/h/ln	0	1856	1856	1885	1885	0				1648	1841	1648
Adj Flow Rate, veh/h	0	743	189	169	917	0				259	1	218
Peak Hour Factor	0.95	0.95	0.95	0.89	0.89	0.89				0.87	0.87	0.87
Percent Heavy Veh, %	0	3	3	1	1	0				17	4	17
Cap, veh/h	0	929	787	300	1194	0				408	2	326
Arrive On Green	0.00	0.50	0.50	0.13	1.00	0.00				0.23	0.23	0.23
Sat Flow, veh/h	0	1856	1572	1795	1885	0				1747	7	1397
Grp Volume(v), veh/h	0	743	189	169	917	0				260	0	218
Grp Sat Flow(s), veh/h/ln	0	1856	1572	1795	1885	0				1753	0	1397
Q Serve(g_s), s	0.0	30.0	6.1	4.0	0.0	0.0				12.0	0.0	12.8
Cycle Q Clear(g_c), s	0.0	30.0	6.1	4.0	0.0	0.0				12.0	0.0	12.8
Prop In Lane	0.00		1.00	1.00		0.00				1.00		1.00
Lane Grp Cap(c), veh/h	0	929	787	300	1194	0				409	0	326
V/C Ratio(X)	0.00	0.80	0.24	0.56	0.77	0.00				0.64	0.00	0.67
Avail Cap(c_a), veh/h	0	929	787	321	1194	0				409	0	326
HCM Platoon Ratio	1.00	1.00	1.00	2.00	2.00	1.00				1.00	1.00	1.00
Upstream Filter(l)	0.00	1.00	1.00	0.10	0.10	0.00				1.00	0.00	1.00
Uniform Delay (d), s/veh	0.0	18.7	12.8	15.0	0.0	0.0				31.1	0.0	31.3
Incr Delay (d2), s/veh	0.0	7.2	0.7	0.2	0.5	0.0				7.3	0.0	10.4
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0				0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	0.0	12.9	2.1	1.2	0.2	0.0				5.8	0.0	5.1
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	0.0	25.9	13.5	15.2	0.5	0.0				38.4	0.0	41.8
LnGrp LOS	A	C	B	B	A	A				D	A	D
Approach Vol, veh/h		932			1086					478		
Approach Delay, s/veh		23.4			2.8					39.9		
Approach LOS		C			A					D		
Timer - Assigned Phs		3	4		6		8					
Phs Duration (G+Y+R _c), s		11.9	51.1		27.0		63.0					
Change Period (Y+R _c), s		6.0	6.0		6.0		6.0					
Max Green Setting (Gmax), s		7.0	44.0		21.0		57.0					
Max Q Clear Time (g _{c+l1}), s		6.0	32.0		14.8		2.0					
Green Ext Time (p _c), s		0.0	4.3		1.3		8.1					
Intersection Summary												
HCM 6th Ctrl Delay		17.6										
HCM 6th LOS			B									



Lane Group	EBL	EBT	WBT	WBR	NBT	NBR
Lane Configurations	↑ ↗	↑ ↗	↑ ↗	↑ ↗	↑ ↗	↑ ↗
Traffic Volume (vph)	225	706	771	91	0	185
Future Volume (vph)	225	706	771	91	0	185
Turn Type	pm+pt	NA	NA	Perm	NA	Perm
Protected Phases	7	4	8		2	
Permitted Phases	4			8		2
Detector Phase	7	4	8	8	2	2
Switch Phase						
Minimum Initial (s)	3.0	8.0	8.0	8.0	8.0	8.0
Minimum Split (s)	9.5	25.0	25.0	25.0	27.0	27.0
Total Split (s)	15.0	64.0	49.0	49.0	26.0	26.0
Total Split (%)	16.7%	71.1%	54.4%	54.4%	28.9%	28.9%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	6.0
Lead/Lag	Lead		Lag	Lag		
Lead-Lag Optimize?	Yes		Yes	Yes		
Recall Mode	None	C-Max	C-Max	C-Max	Max	Max

Intersection Summary

Cycle Length: 90

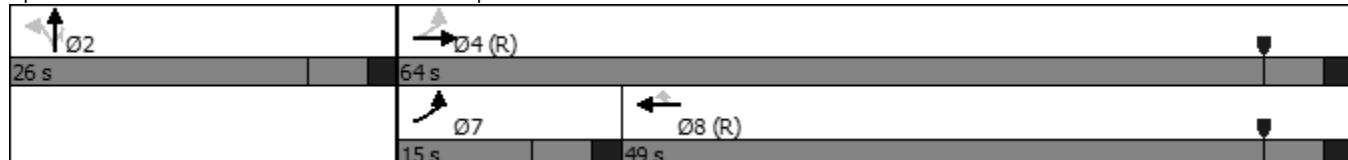
Actuated Cycle Length: 90

Offset: 60 (67%), Referenced to phase 4:EBTL and 8:WBT, Start of Yellow

Natural Cycle: 90

Control Type: Actuated-Coordinated

Splits and Phases: 2: E-470 Northbound Ramps & Jewell Avenue





Lane Group	EBL	EBT	WBT	WBR	NBT	NBR
Lane Group Flow (vph)	237	743	876	103	224	213
v/c Ratio	0.94	0.64	0.99	0.13	0.58	0.41
Control Delay	59.3	11.5	54.1	2.9	38.0	7.0
Queue Delay	0.0	0.3	0.0	0.0	0.0	0.0
Total Delay	59.3	11.8	54.1	2.9	38.0	7.0
Queue Length 50th (ft)	114	208	475	0	114	0
Queue Length 95th (ft)	m#156	m312	#719	22	181	50
Internal Link Dist (ft)		530	2523		717	
Turn Bay Length (ft)	250			250		175
Base Capacity (vph)	252	1166	881	806	389	514
Starvation Cap Reductn	0	79	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.94	0.68	0.99	0.13	0.58	0.41

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

HCM 6th Signalized Intersection Summary
11/22/2023

2: E-470 Northbound Ramps & Jewell Avenue
2027 Bkgrd + Proj - AM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑			↑	↑		↑	↑	0	0	0
Traffic Volume (veh/h)	225	706	0	0	771	91	195	0	185	0	0	0
Future Volume (veh/h)	225	706	0	0	771	91	195	0	185	0	0	0
Initial Q (Q _b), veh	0	0	0	0	0	0	0	0	0			
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00			
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Work Zone On Approach		No			No			No				
Adj Sat Flow, veh/h/ln	1826	1826	0	0	1856	1856	1856	1856	1856			
Adj Flow Rate, veh/h	237	743	0	0	876	103	224	0	213			
Peak Hour Factor	0.95	0.95	0.95	0.88	0.88	0.88	0.87	0.87	0.87			
Percent Heavy Veh, %	5	5	0	0	3	3	3	3	3			
Cap, veh/h	260	1177	0	0	887	751	393	0	349			
Arrive On Green	0.20	1.00	0.00	0.00	0.48	0.48	0.22	0.00	0.22			
Sat Flow, veh/h	1739	1826	0	0	1856	1572	1767	0	1572			
Grp Volume(v), veh/h	237	743	0	0	876	103	224	0	213			
Grp Sat Flow(s), veh/h/ln	1739	1826	0	0	1856	1572	1767	0	1572			
Q Serve(g_s), s	7.4	0.0	0.0	0.0	42.0	3.3	10.2	0.0	11.0			
Cycle Q Clear(g_c), s	7.4	0.0	0.0	0.0	42.0	3.3	10.2	0.0	11.0			
Prop In Lane	1.00		0.00	0.00		1.00	1.00		1.00			
Lane Grp Cap(c), veh/h	260	1177	0	0	887	751	393	0	349			
V/C Ratio(X)	0.91	0.63	0.00	0.00	0.99	0.14	0.57	0.00	0.61			
Avail Cap(c_a), veh/h	260	1177	0	0	887	751	393	0	349			
HCM Platoon Ratio	2.00	2.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Upstream Filter(l)	0.46	0.46	0.00	0.00	0.59	0.59	1.00	0.00	1.00			
Uniform Delay (d), s/veh	20.9	0.0	0.0	0.0	23.2	13.1	31.2	0.0	31.5			
Incr Delay (d2), s/veh	19.1	1.2	0.0	0.0	20.6	0.2	5.9	0.0	7.7			
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0			
%ile BackOfQ(50%), veh/ln	3.3	0.4	0.0	0.0	20.8	1.1	4.9	0.0	4.8			
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	40.1	1.2	0.0	0.0	43.8	13.4	37.1	0.0	39.2			
LnGrp LOS	D	A	A	A	D	B	D	A	D			
Approach Vol, veh/h	980				979				437			
Approach Delay, s/veh	10.6				40.6				38.1			
Approach LOS	B				D				D			
Timer - Assigned Phs	2		4			7		8				
Phs Duration (G+Y+Rc), s	26.0		64.0			15.0		49.0				
Change Period (Y+Rc), s	6.0		6.0			6.0		6.0				
Max Green Setting (Gmax), s	20.0		58.0			9.0		43.0				
Max Q Clear Time (g_c+l1), s	13.0		2.0			9.4		44.0				
Green Ext Time (p_c), s	1.2		5.6			0.0		0.0				
Intersection Summary												
HCM 6th Ctrl Delay		27.9										
HCM 6th LOS		C										
Notes												
User approved pedestrian interval to be less than phase max green.												

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑↑	↑	↑↑	↑↑	↑	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑
Traffic Volume (vph)	150	245	496	340	385	45	307	670	116	40	620	170
Future Volume (vph)	150	245	496	340	385	45	307	670	116	40	620	170
Turn Type	Prot	NA	pm+ov	Prot	NA	Perm	Prot	NA	Perm	pm+pt	NA	Perm
Protected Phases	7	4	5	3	8		5	2		1	6	
Permitted Phases						8			2	6		6
Detector Phase	7	4	5	3	8	8	5	2	2	1	6	6
Switch Phase												
Minimum Initial (s)	3.0	4.0	4.0	3.0	5.0	5.0	4.0	8.0	8.0	4.0	8.0	8.0
Minimum Split (s)	8.0	32.0	9.0	8.0	32.0	32.0	9.0	40.0	40.0	9.0	29.0	29.0
Total Split (s)	12.0	32.0	24.0	25.0	45.0	45.0	24.0	54.0	54.0	9.0	39.0	39.0
Total Split (%)	10.0%	26.7%	20.0%	20.8%	37.5%	37.5%	20.0%	45.0%	45.0%	7.5%	32.5%	32.5%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	4.0	4.0	3.0	4.0	4.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	6.0	6.0	5.0	6.0	6.0
Lead/Lag	Lead	Lag	Lead	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes											
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max						

Intersection Summary

Cycle Length: 120

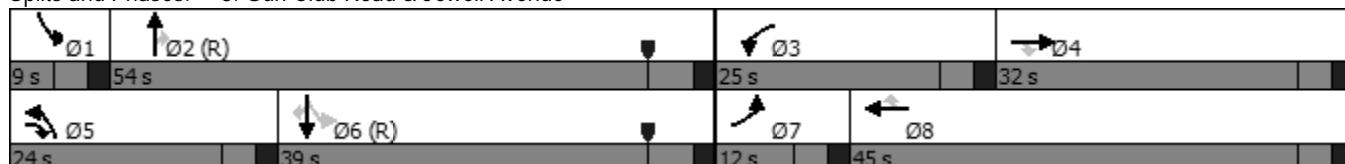
Actuated Cycle Length: 120

Offset: 108.5 (90%), Referenced to phase 2:NBT and 6:SBTL, Start of Yellow

Natural Cycle: 90

Control Type: Actuated-Coordinated

Splits and Phases: 3: Gun Club Road & Jewell Avenue





Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	158	258	522	391	443	52	472	1031	178	46	713	195
v/c Ratio	0.85	0.72	0.75	0.85	0.86	0.09	0.82	0.63	0.22	0.21	0.64	0.31
Control Delay	92.0	56.6	30.2	67.9	57.0	0.3	60.3	28.6	4.0	19.4	39.3	5.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	92.0	56.6	30.2	67.9	57.0	0.3	60.3	28.6	4.0	19.4	39.3	5.6
Queue Length 50th (ft)	63	187	254	154	324	0	177	334	0	16	259	0
Queue Length 95th (ft)	#122	275	404	194	402	0	169	273	7	38	323	47
Internal Link Dist (ft)		2523			1479			1470			790	
Turn Bay Length (ft)	220		330	300			485		360	200		325
Base Capacity (vph)	192	403	699	572	621	637	589	1629	825	220	1116	635
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.82	0.64	0.75	0.68	0.71	0.08	0.80	0.63	0.22	0.21	0.64	0.31

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM 6th Signalized Intersection Summary
11/22/2023

3: Gun Club Road & Jewell Avenue
2027 Bkgrd + Proj - AM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑↑	↑	↑	↑↑	↑	↑	↑↑	↑↑	↑	↑	↑↑	↑
Traffic Volume (veh/h)	150	245	496	340	385	45	307	670	116	40	620	170
Future Volume (veh/h)	150	245	496	340	385	45	307	670	116	40	620	170
Initial Q (Q _b), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00			1.00	1.00		1.00	1.00		1.00	1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No			No			No		
Adj Sat Flow, veh/h/ln	1811	1811	1811	1870	1870	1870	1870	1870	1870	1841	1841	1841
Adj Flow Rate, veh/h	158	258	522	391	443	52	472	1031	178	46	713	195
Peak Hour Factor	0.95	0.95	0.95	0.87	0.87	0.87	0.65	0.65	0.65	0.87	0.87	0.87
Percent Heavy Veh, %	6	6	6	2	2	2	2	2	2	4	4	4
Cap, veh/h	195	407	579	443	551	467	526	1581	705	206	1117	498
Arrive On Green	0.06	0.22	0.22	0.13	0.29	0.29	0.15	0.44	0.44	0.03	0.32	0.32
Sat Flow, veh/h	3346	1811	1535	3456	1870	1585	3456	3554	1585	1753	3497	1560
Grp Volume(v), veh/h	158	258	522	391	443	52	472	1031	178	46	713	195
Grp Sat Flow(s), veh/h/ln	1673	1811	1535	1728	1870	1585	1728	1777	1585	1753	1749	1560
Q Serve(g_s), s	5.6	15.4	27.0	13.3	26.3	2.9	16.1	27.2	8.4	2.1	20.9	11.7
Cycle Q Clear(g_c), s	5.6	15.4	27.0	13.3	26.3	2.9	16.1	27.2	8.4	2.1	20.9	11.7
Prop In Lane	1.00			1.00	1.00		1.00	1.00		1.00	1.00	1.00
Lane Grp Cap(c), veh/h	195	407	579	443	551	467	526	1581	705	206	1117	498
V/C Ratio(X)	0.81	0.63	0.90	0.88	0.80	0.11	0.90	0.65	0.25	0.22	0.64	0.39
Avail Cap(c_a), veh/h	195	407	579	576	623	528	547	1581	705	218	1117	498
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	0.78	0.78	0.78	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	55.8	42.0	35.3	51.4	39.1	30.8	49.9	26.0	20.8	27.0	34.9	31.8
Incr Delay (d2), s/veh	16.6	2.5	14.2	10.4	6.8	0.1	17.1	2.1	0.9	0.5	2.8	2.3
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	2.8	7.0	15.9	6.3	12.6	1.1	7.9	11.0	3.2	0.9	8.8	4.6
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	72.5	44.5	49.5	61.9	45.9	30.9	67.0	28.1	21.7	27.5	37.7	34.1
LnGrp LOS	E	D	D	E	D	C	E	C	C	C	D	C
Approach Vol, veh/h	938				886			1681			954	
Approach Delay, s/veh	52.0				52.0			38.4			36.5	
Approach LOS	D				D			D			D	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	8.2	59.4	20.4	32.0	23.3	44.3	12.0	40.4				
Change Period (Y+Rc), s	5.0	6.0	5.0	5.0	5.0	6.0	5.0	5.0				
Max Green Setting (Gmax), s	4.0	48.0	20.0	27.0	19.0	33.0	7.0	40.0				
Max Q Clear Time (g_c+l1), s	4.1	29.2	15.3	29.0	18.1	22.9	7.6	28.3				
Green Ext Time (p_c), s	0.0	13.4	0.0	0.0	0.2	6.5	0.0	2.1				

Intersection Summary

HCM 6th Ctrl Delay 43.5
HCM 6th LOS D

Notes

User approved pedestrian interval to be less than phase max green.

Intersection

Int Delay, s/veh 0.5

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	0	10	69	1114	1085	100
Future Vol, veh/h	0	10	69	1114	1085	100
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	200	-	-	200
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	95	95	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	11	75	1173	1142	109

Major/Minor	Minor2	Major1	Major2		
Conflicting Flow All	-	571	1251	0	-
Stage 1	-	-	-	-	-
Stage 2	-	-	-	-	-
Critical Hdwy	-	6.94	4.14	-	-
Critical Hdwy Stg 1	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-
Follow-up Hdwy	-	3.32	2.22	-	-
Pot Cap-1 Maneuver	0	464	552	-	-
Stage 1	0	-	-	-	-
Stage 2	0	-	-	-	-
Platoon blocked, %				-	-
Mov Cap-1 Maneuver	-	464	552	-	-
Mov Cap-2 Maneuver	-	-	-	-	-
Stage 1	-	-	-	-	-
Stage 2	-	-	-	-	-

Approach	EB	NB	SB	
HCM Control Delay, s	12.9	0.8	0	
HCM LOS	B			

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	552	-	464	-	-
HCM Lane V/C Ratio	0.136	-	0.023	-	-
HCM Control Delay (s)	12.5	-	12.9	-	-
HCM Lane LOS	B	-	B	-	-
HCM 95th %tile Q(veh)	0.5	-	0.1	-	-

Lane Group	EBL	EBT	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑ ↗	↗ ↘	↖ ↗	↑ ↗	↗ ↗	↖ ↗	↑↑ ↗	↗ ↗	↖ ↗	↑↑ ↗	↗ ↗
Traffic Volume (vph)	50	5	55	5	45	108	1088	30	75	895	125
Future Volume (vph)	50	5	55	5	45	108	1088	30	75	895	125
Turn Type	pm+pt	NA	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases	7	4	3	8		5	2		1	6	
Permitted Phases					8	2		2	6		6
Detector Phase	7	4	3	8	8	5	2	2	1	6	6
Switch Phase											
Minimum Initial (s)	5.0	8.0	5.0	8.0	8.0	5.0	8.0	8.0	5.0	8.0	8.0
Minimum Split (s)	11.0	33.0	11.0	33.0	33.0	11.0	30.0	30.0	11.0	30.0	30.0
Total Split (s)	20.0	42.0	20.0	42.0	42.0	20.0	38.0	38.0	20.0	38.0	38.0
Total Split (%)	16.7%	35.0%	16.7%	35.0%	35.0%	16.7%	31.7%	31.7%	16.7%	31.7%	31.7%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0
Lead/Lag	Lead	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes										
Recall Mode	None	None	None	None	None	None	C-Max	C-Max	None	C-Max	C-Max

Intersection Summary

Cycle Length: 120

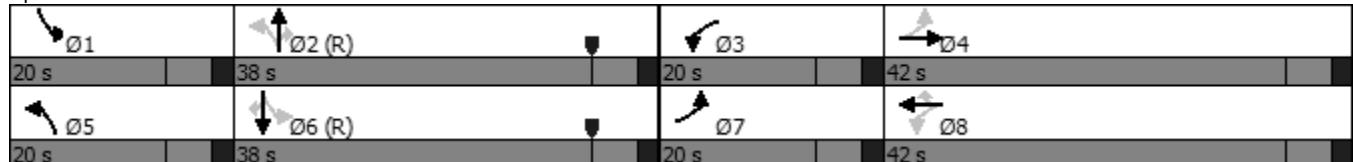
Actuated Cycle Length: 120

Offset: 0 (0%), Referenced to phase 2:NBTL and 6:SBTL, Start of Yellow

Natural Cycle: 85

Control Type: Actuated-Coordinated

Splits and Phases: 102: Gun Club Road & Yale Avenue





Lane Group	EBL	EBT	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	54	18	60	5	49	117	1145	33	82	942	136
v/c Ratio	0.30	0.15	0.25	0.04	0.21	0.28	0.49	0.03	0.24	0.42	0.13
Control Delay	48.4	32.2	42.9	52.8	2.0	7.5	14.5	0.1	7.5	13.5	2.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	48.4	32.2	42.9	52.8	2.0	7.5	14.5	0.1	7.5	13.5	2.4
Queue Length 50th (ft)	41	4	39	4	0	25	262	0	17	202	0
Queue Length 95th (ft)	70	28	77	17	0	48	370	0	36	280	29
Internal Link Dist (ft)		816		718			2631			1641	
Turn Bay Length (ft)	150		350		10	300		300	300		300
Base Capacity (vph)	256	507	279	558	570	494	2358	1100	430	2249	1055
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.21	0.04	0.22	0.01	0.09	0.24	0.49	0.03	0.19	0.42	0.13

Intersection Summary

HCM 6th Signalized Intersection Summary
11/22/2023

102: Gun Club Road & Yale Avenue
2027 Bkgnd + Proj - AM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑		↑	↑	↑	↑	↑↑	↑	↑	↑↑	↑
Traffic Volume (veh/h)	50	5	12	55	5	45	108	1088	30	75	895	125
Future Volume (veh/h)	50	5	12	55	5	45	108	1088	30	75	895	125
Initial Q (Q _b), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00			1.00	1.00		1.00	1.00		1.00	1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	54	5	13	60	5	0	117	1145	33	82	942	136
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.95	0.92	0.92	0.95	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	214	28	74	206	123		411	2337	1042	365	2330	1039
Arrive On Green	0.04	0.06	0.06	0.04	0.07	0.00	0.04	0.66	0.66	0.04	0.66	0.66
Sat Flow, veh/h	1781	460	1195	1781	1870	1585	1781	3554	1585	1781	3554	1585
Grp Volume(v), veh/h	54	0	18	60	5	0	117	1145	33	82	942	136
Grp Sat Flow(s), veh/h/ln	1781	0	1655	1781	1870	1585	1781	1777	1585	1781	1777	1585
Q Serve(g_s), s	3.4	0.0	1.2	3.8	0.3	0.0	2.6	19.5	0.9	1.8	14.9	3.9
Cycle Q Clear(g_c), s	3.4	0.0	1.2	3.8	0.3	0.0	2.6	19.5	0.9	1.8	14.9	3.9
Prop In Lane	1.00			1.00		1.00	1.00	1.00		1.00	1.00	1.00
Lane Grp Cap(c), veh/h	214	0	102	206	123		411	2337	1042	365	2330	1039
V/C Ratio(X)	0.25	0.00	0.18	0.29	0.04		0.28	0.49	0.03	0.22	0.40	0.13
Avail Cap(c_a), veh/h	355	0	497	339	561		546	2337	1042	503	2330	1039
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	0.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	50.2	0.0	53.4	49.9	52.5	0.0	7.1	10.4	7.2	7.7	9.7	7.8
Incr Delay (d2), s/veh	0.6	0.0	0.8	0.8	0.1	0.0	0.4	0.7	0.1	0.3	0.5	0.3
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	1.5	0.0	0.5	1.7	0.1	0.0	0.8	6.5	0.3	0.6	5.0	1.4
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	50.8	0.0	54.2	50.7	52.7	0.0	7.5	11.1	7.2	8.0	10.2	8.0
LnGrp LOS	D	A	D	D	D		A	B	A	A	B	A
Approach Vol, veh/h			72			65			1295		1160	
Approach Delay, s/veh			51.7			50.8			10.7		9.8	
Approach LOS			D			D			B		A	

Intersection Summary

HCM 6th Ctrl Delay 12.4
HCM 6th LOS B

Notes

Unsignalized Delay for [WBR] is excluded from calculations of the approach delay and intersection delay.

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑	↑	↑	↑	↑↑	↑	↑	↑↑	↑
Traffic Volume (vph)	73	1	5	1	73	1026	15	20	1161	194
Future Volume (vph)	73	1	5	1	73	1026	15	20	1161	194
Turn Type	pm+pt	NA	pm+pt	NA	pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases	7	4	3	8	5	2		1	6	
Permitted Phases						2	2	6		6
Detector Phase	7	4	3	8	5	2	2	1	6	6
Switch Phase										
Minimum Initial (s)	5.0	5.0	5.0	8.0	5.0	8.0	8.0	5.0	8.0	8.0
Minimum Split (s)	11.0	38.0	11.0	38.0	11.0	28.0	28.0	11.0	28.0	28.0
Total Split (s)	11.0	38.0	11.0	38.0	11.0	30.0	30.0	11.0	30.0	30.0
Total Split (%)	12.2%	42.2%	12.2%	42.2%	12.2%	33.3%	33.3%	12.2%	33.3%	33.3%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0
Lead/Lag	Lead	Lag	Lead	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes									
Recall Mode	None	None	None	None	None	C-Max	C-Max	None	C-Max	C-Max

Intersection Summary

Cycle Length: 90

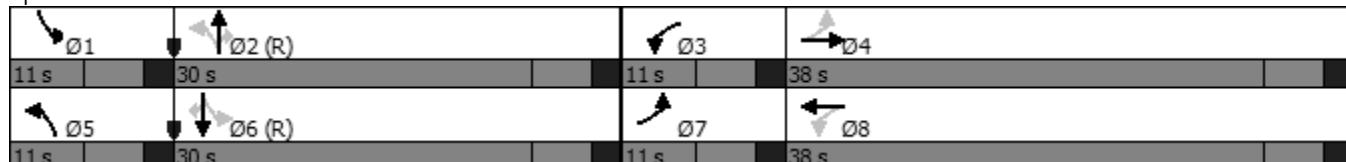
Actuated Cycle Length: 90

Offset: 0 (0%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 100

Control Type: Actuated-Coordinated

Splits and Phases: 103: Gun Club Road & Addison Avenue





Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	79	22	5	17	79	1115	16	22	1262	211
v/c Ratio	0.53	0.13	0.03	0.11	0.25	0.43	0.01	0.06	0.53	0.19
Control Delay	47.3	17.5	30.4	20.2	6.5	9.2	0.0	5.3	12.6	2.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	47.3	17.5	30.4	20.2	6.5	9.2	0.0	5.3	12.6	2.3
Queue Length 50th (ft)	45	1	3	1	7	68	0	2	171	0
Queue Length 95th (ft)	76	23	11	21	29	287	0	12	361	34
Internal Link Dist (ft)		564		952		985			915	
Turn Bay Length (ft)	300		350		350		350	350		350
Base Capacity (vph)	150	581	158	579	311	2582	1204	375	2367	1128
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.53	0.04	0.03	0.03	0.25	0.43	0.01	0.06	0.53	0.19

Intersection Summary

HCM 6th Signalized Intersection Summary
11/22/2023

103: Gun Club Road & Addison Avenue
2027 Bkgnd + Proj - AM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑		↑	↑		↑	↑↑	↑	↑	↑↑	↑
Traffic Volume (veh/h)	73	1	19	5	1	15	73	1026	15	20	1161	194
Future Volume (veh/h)	73	1	19	5	1	15	73	1026	15	20	1161	194
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00			1.00	1.00		1.00	1.00		1.00	1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	79	1	21	5	1	16	79	1115	16	22	1262	211
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	246	5	96	240	6	89	286	2118	945	327	2031	906
Arrive On Green	0.05	0.06	0.06	0.05	0.06	0.06	0.05	0.60	0.60	0.02	0.57	0.57
Sat Flow, veh/h	1781	73	1524	1781	94	1505	1781	3554	1585	1781	3554	1585
Grp Volume(v), veh/h	79	0	22	5	0	17	79	1115	16	22	1262	211
Grp Sat Flow(s), veh/h/ln	1781	0	1596	1781	0	1599	1781	1777	1585	1781	1777	1585
Q Serve(g_s), s	3.7	0.0	1.2	0.2	0.0	0.9	1.6	16.6	0.4	0.5	21.2	5.9
Cycle Q Clear(g_c), s	3.7	0.0	1.2	0.2	0.0	0.9	1.6	16.6	0.4	0.5	21.2	5.9
Prop In Lane	1.00			0.95	1.00		0.94	1.00		1.00	1.00	1.00
Lane Grp Cap(c), veh/h	246	0	100	240	0	95	286	2118	945	327	2031	906
V/C Ratio(X)	0.32	0.00	0.22	0.02	0.00	0.18	0.28	0.53	0.02	0.07	0.62	0.23
Avail Cap(c_a), veh/h	248	0	568	248	0	569	300	2118	945	384	2031	906
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	37.0	0.0	40.1	35.7	0.0	40.2	10.0	10.7	7.4	8.7	12.8	9.5
Incr Delay (d2), s/veh	0.7	0.0	1.1	0.0	0.0	0.9	0.5	0.9	0.0	0.1	1.4	0.6
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	1.6	0.0	0.5	0.1	0.0	0.4	0.5	5.2	0.1	0.1	7.0	2.1
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	37.7	0.0	41.2	35.8	0.0	41.1	10.5	11.6	7.5	8.8	14.2	10.1
LnGrp LOS	D	A	D	D	A	D	B	B	A	A	B	B
Approach Vol, veh/h	101				22			1210			1495	
Approach Delay, s/veh	38.5				39.9			11.5			13.6	
Approach LOS	D				D			B			B	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	8.1	59.6	10.6	11.6	10.3	57.4	10.9	11.3				
Change Period (Y+Rc), s	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0				
Max Green Setting (Gmax), s	5.0	24.0	5.0	32.0	5.0	24.0	5.0	32.0				
Max Q Clear Time (g_c+l1), s	2.5	18.6	2.2	3.2	3.6	23.2	5.7	2.9				
Green Ext Time (p_c), s	0.0	3.0	0.0	0.1	0.0	0.6	0.0	0.0				
Intersection Summary												
HCM 6th Ctrl Delay				13.8								
HCM 6th LOS				B								

Intersection

Int Delay, s/veh 0.2

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	0	6	21	1093	1369	87
Future Vol, veh/h	0	6	21	1093	1369	87
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	300	-	-	300
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	7	23	1188	1488	95

Major/Minor	Minor2	Major1	Major2		
Conflicting Flow All	-	744	1583	0	-
Stage 1	-	-	-	-	-
Stage 2	-	-	-	-	-
Critical Hdwy	-	6.94	4.14	-	-
Critical Hdwy Stg 1	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-
Follow-up Hdwy	-	3.32	2.22	-	-
Pot Cap-1 Maneuver	0	357	411	-	-
Stage 1	0	-	-	-	-
Stage 2	0	-	-	-	-
Platoon blocked, %				-	-
Mov Cap-1 Maneuver	-	357	411	-	-
Mov Cap-2 Maneuver	-	-	-	-	-
Stage 1	-	-	-	-	-
Stage 2	-	-	-	-	-

Approach	EB	NB	SB	
HCM Control Delay, s	15.3	0.3	0	
HCM LOS	C			

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	411	-	357	-	-
HCM Lane V/C Ratio	0.056	-	0.018	-	-
HCM Control Delay (s)	14.3	-	15.3	-	-
HCM Lane LOS	B	-	C	-	-
HCM 95th %tile Q(veh)	0.2	-	0.1	-	-



Lane Group	EBT	EBR	WBL	WBT	SBT	SBR
Lane Configurations	↑	↗	↖	↑	↖	↗
Traffic Volume (vph)	805	320	227	738	5	260
Future Volume (vph)	805	320	227	738	5	260
Turn Type	NA	Perm	pm+pt	NA	NA	Perm
Protected Phases	4			3	8	6
Permitted Phases				4	8	6
Detector Phase	4	4	3	8	6	6
Switch Phase						
Minimum Initial (s)	8.0	8.0	4.0	8.0	8.0	8.0
Minimum Split (s)	25.0	25.0	12.5	25.0	27.0	27.0
Total Split (s)	50.5	50.5	12.5	63.0	27.0	27.0
Total Split (%)	56.1%	56.1%	13.9%	70.0%	30.0%	30.0%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	6.0
Lead/Lag	Lag	Lag	Lead			
Lead-Lag Optimize?	Yes	Yes	Yes			
Recall Mode	C-Max	C-Max	None	C-Max	Max	Max

Intersection Summary

Cycle Length: 90

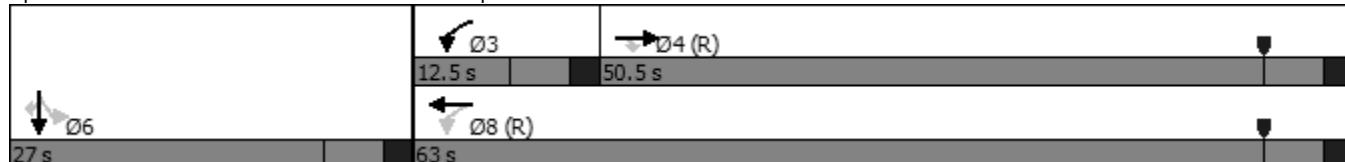
Actuated Cycle Length: 90

Offset: 83 (92%), Referenced to phase 4:EBT and 8:WBTL, Start of Yellow

Natural Cycle: 90

Control Type: Actuated-Coordinated

Splits and Phases: 1: E-470 Southbound Ramps & Jewell Avenue





Lane Group	EBT	EBR	WBL	WBT	SBT	SBR
Lane Group Flow (vph)	847	337	236	769	103	302
v/c Ratio	0.92	0.35	1.12	0.65	0.26	0.57
Control Delay	38.2	2.6	105.1	12.1	30.3	13.7
Queue Delay	0.0	0.0	0.0	0.4	0.0	0.0
Total Delay	38.2	2.6	105.1	12.5	30.3	13.7
Queue Length 50th (ft)	427	0	~132	212	48	37
Queue Length 95th (ft)	#687	41	m#124	m213	88	103
Internal Link Dist (ft)	679			530	532	
Turn Bay Length (ft)		300	250			
Base Capacity (vph)	921	953	210	1179	399	526
Starvation Cap Reductn	0	0	0	109	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.92	0.35	1.12	0.72	0.26	0.57

Intersection Summary

- ~ Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.
- m Volume for 95th percentile queue is metered by upstream signal.

HCM 6th Signalized Intersection Summary
11/22/2023

1: E-470 Southbound Ramps & Jewell Avenue
2027 Bkgrd + Proj - PM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	0	805	320	227	738	0	0	0	0	83	5	260
Future Volume (veh/h)	0	805	320	227	738	0	0	0	0	83	5	260
Initial Q (Q _b), veh	0	0	0	0	0	0				0	0	0
Ped-Bike Adj(A_pbT)	1.00			1.00			1.00			1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00				1.00	1.00	1.00
Work Zone On Approach		No			No						No	
Adj Sat Flow, veh/h/ln	0	1870	1870	1870	1870	0				1811	1811	1811
Adj Flow Rate, veh/h	0	847	337	236	769	0				97	6	302
Peak Hour Factor	0.95	0.95	0.95	0.96	0.96	0.96				0.86	0.86	0.86
Percent Heavy Veh, %	0	2	2	2	2	0				6	6	6
Cap, veh/h	0	925	784	245	1185	0				380	24	358
Arrive On Green	0.00	0.49	0.49	0.14	1.00	0.00				0.23	0.23	0.23
Sat Flow, veh/h	0	1870	1585	1781	1870	0				1629	101	1535
Grp Volume(v), veh/h	0	847	337	236	769	0				103	0	302
Grp Sat Flow(s), veh/h/ln	0	1870	1585	1781	1870	0				1730	0	1535
Q Serve(g_s), s	0.0	37.7	12.3	6.2	0.0	0.0				4.4	0.0	16.9
Cycle Q Clear(g_c), s	0.0	37.7	12.3	6.2	0.0	0.0				4.4	0.0	16.9
Prop In Lane	0.00		1.00	1.00		0.00				0.94		1.00
Lane Grp Cap(c), veh/h	0	925	784	245	1185	0				404	0	358
V/C Ratio(X)	0.00	0.92	0.43	0.96	0.65	0.00				0.26	0.00	0.84
Avail Cap(c_a), veh/h	0	925	784	245	1185	0				404	0	358
HCM Platoon Ratio	1.00	1.00	1.00	2.00	2.00	1.00				1.00	1.00	1.00
Upstream Filter(l)	0.00	1.00	1.00	0.09	0.09	0.00				1.00	0.00	1.00
Uniform Delay (d), s/veh	0.0	21.0	14.6	18.7	0.0	0.0				28.1	0.0	32.9
Incr Delay (d2), s/veh	0.0	15.1	1.7	10.4	0.3	0.0				1.5	0.0	20.9
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0				0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	0.0	17.9	4.3	2.7	0.1	0.0				2.0	0.0	8.2
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	0.0	36.2	16.3	29.1	0.3	0.0				29.6	0.0	53.8
LnGrp LOS	A	D	B	C	A	A				C	A	D
Approach Vol, veh/h		1184			1005						405	
Approach Delay, s/veh		30.5			7.0						47.7	
Approach LOS		C			A						D	
Timer - Assigned Phs		3	4		6		8					
Phs Duration (G+Y+R _c), s		12.5	50.5		27.0		63.0					
Change Period (Y+R _c), s		6.0	6.0		6.0		6.0					
Max Green Setting (Gmax), s		6.5	44.5		21.0		57.0					
Max Q Clear Time (g _{c+l1}), s		8.2	39.7		18.9		2.0					
Green Ext Time (p _c), s		0.0	2.8		0.4		5.9					
Intersection Summary												
HCM 6th Ctrl Delay			24.1									
HCM 6th LOS			C									



Lane Group	EBL	EBT	WBT	WBR	NBT	NBR
Lane Configurations	↑ ↗	↑ ↗	↑ ↗	↑ ↗	↑ ↗	↑ ↗
Traffic Volume (vph)	195	693	845	246	0	100
Future Volume (vph)	195	693	845	246	0	100
Turn Type	pm+pt	NA	NA	Perm	NA	Perm
Protected Phases	7	4	8		2	
Permitted Phases	4			8		2
Detector Phase	7	4	8	8	2	2
Switch Phase						
Minimum Initial (s)	3.0	8.0	8.0	8.0	8.0	8.0
Minimum Split (s)	9.5	25.0	25.0	25.0	27.0	27.0
Total Split (s)	13.0	65.0	52.0	52.0	25.0	25.0
Total Split (%)	14.4%	72.2%	57.8%	57.8%	27.8%	27.8%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	6.0
Lead/Lag	Lead		Lag	Lag		
Lead-Lag Optimize?	Yes		Yes	Yes		
Recall Mode	None	C-Max	C-Max	C-Max	Max	Max

Intersection Summary

Cycle Length: 90

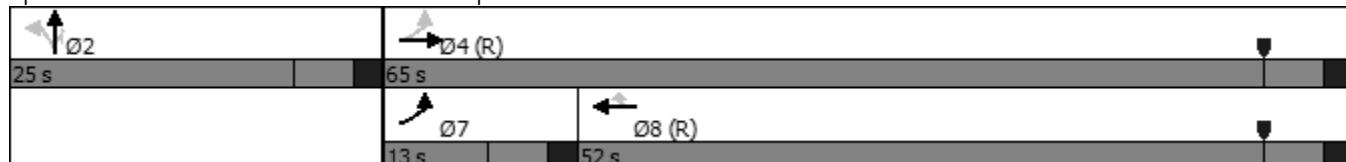
Actuated Cycle Length: 90

Offset: 0 (0%), Referenced to phase 4:EBTL and 8:WBT, Start of Yellow

Natural Cycle: 100

Control Type: Actuated-Coordinated

Splits and Phases: 2: E-470 Northbound Ramps & Jewell Avenue





Lane Group	EBL	EBT	WBT	WBR	NBT	NBR
Lane Group Flow (vph)	210	745	939	273	150	125
v/c Ratio	0.97	0.62	1.04	0.30	0.41	0.30
Control Delay	73.4	4.5	63.2	2.8	34.8	7.8
Queue Delay	0.0	0.2	0.0	0.0	0.0	0.0
Total Delay	73.4	4.7	63.2	2.8	34.8	7.8
Queue Length 50th (ft)	90	60	~580	3	74	0
Queue Length 95th (ft)	m#113	m77	#811	40	115	32
Internal Link Dist (ft)		530	2523		717	
Turn Bay Length (ft)	250			250		175
Base Capacity (vph)	216	1197	907	899	362	423
Starvation Cap Reductn	0	70	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.97	0.66	1.04	0.30	0.41	0.30

Intersection Summary

- ~ Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.
- m Volume for 95th percentile queue is metered by upstream signal.

HCM 6th Signalized Intersection Summary
11/22/2023

2: E-470 Northbound Ramps & Jewell Avenue
2027 Bkgnd + Proj - PM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	195	693	0	0	845	246	120	0	100	0	0	0
Future Volume (veh/h)	195	693	0	0	845	246	120	0	100	0	0	0
Initial Q (Q _b), veh	0	0	0	0	0	0	0	0	0			
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00			
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Work Zone On Approach		No			No			No				
Adj Sat Flow, veh/h/ln	1841	1841	0	0	1796	1796	1826	1826	1826			
Adj Flow Rate, veh/h	210	745	0	0	939	273	150	0	125			
Peak Hour Factor	0.93	0.93	0.93	0.90	0.90	0.90	0.80	0.80	0.80			
Percent Heavy Veh, %	4	4	0	0	7	7	5	5	5			
Cap, veh/h	216	1207	0	0	918	778	367	0	327			
Arrive On Green	0.16	1.00	0.00	0.00	0.51	0.51	0.21	0.00	0.21			
Sat Flow, veh/h	1753	1841	0	0	1796	1522	1739	0	1547			
Grp Volume(v), veh/h	210	745	0	0	939	273	150	0	125			
Grp Sat Flow(s), veh/h/ln	1753	1841	0	0	1796	1522	1739	0	1547			
Q Serve(g_s), s	6.6	0.0	0.0	0.0	46.0	9.6	6.7	0.0	6.2			
Cycle Q Clear(g_c), s	6.6	0.0	0.0	0.0	46.0	9.6	6.7	0.0	6.2			
Prop In Lane	1.00		0.00	0.00		1.00	1.00		1.00			
Lane Grp Cap(c), veh/h	216	1207	0	0	918	778	367	0	327			
V/C Ratio(X)	0.97	0.62	0.00	0.00	1.02	0.35	0.41	0.00	0.38			
Avail Cap(c_a), veh/h	216	1207	0	0	918	778	367	0	327			
HCM Platoon Ratio	2.00	2.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Upstream Filter(l)	0.27	0.27	0.00	0.00	0.57	0.57	1.00	0.00	1.00			
Uniform Delay (d), s/veh	23.2	0.0	0.0	0.0	22.0	13.1	30.6	0.0	30.5			
Incr Delay (d2), s/veh	24.7	0.6	0.0	0.0	28.4	0.7	3.3	0.0	3.4			
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0			
%ile BackOfQ(50%), veh/ln	3.4	0.2	0.0	0.0	23.5	3.0	3.1	0.0	2.6			
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	47.9	0.6	0.0	0.0	50.4	13.8	34.0	0.0	33.8			
LnGrp LOS	D	A	A	A	F	B	C	A	C			
Approach Vol, veh/h	955				1212				275			
Approach Delay, s/veh	11.0				42.1				33.9			
Approach LOS	B				D				C			
Timer - Assigned Phs	2		4			7		8				
Phs Duration (G+Y+Rc), s	25.0		65.0			13.0		52.0				
Change Period (Y+Rc), s	6.0		6.0			6.0		6.0				
Max Green Setting (Gmax), s	19.0		59.0			7.0		46.0				
Max Q Clear Time (g_c+l1), s	8.7		2.0			8.6		48.0				
Green Ext Time (p_c), s	0.9		5.6			0.0		0.0				

Intersection Summary

HCM 6th Ctrl Delay 29.1
HCM 6th LOS C

Notes

User approved pedestrian interval to be less than phase max green.

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑↑	↑	↑↑	↑↑	↑	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑
Traffic Volume (vph)	180	335	278	250	350	40	556	761	373	15	906	185
Future Volume (vph)	180	335	278	250	350	40	556	761	373	15	906	185
Turn Type	Prot	NA	pm+ov	Prot	NA	Perm	Prot	NA	Perm	pm+pt	NA	Perm
Protected Phases	7	4	5	3	8		5	2		1	6	
Permitted Phases						8				2	6	6
Detector Phase	7	4	5	3	8	8	5	2	2	1	6	6
Switch Phase												
Minimum Initial (s)	3.0	4.0	4.0	3.0	5.0	5.0	4.0	8.0	8.0	4.0	8.0	8.0
Minimum Split (s)	8.0	32.0	9.0	8.0	32.0	32.0	9.0	40.0	40.0	9.0	29.0	29.0
Total Split (s)	15.0	37.0	25.0	16.0	38.0	38.0	25.0	58.0	58.0	9.0	42.0	42.0
Total Split (%)	12.5%	30.8%	20.8%	13.3%	31.7%	31.7%	20.8%	48.3%	48.3%	7.5%	35.0%	35.0%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	4.0	4.0	3.0	4.0	4.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	6.0	6.0	5.0	6.0	6.0
Lead/Lag	Lead	Lag	Lead	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes											
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max						

Intersection Summary

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBTL, Start of Yellow

Natural Cycle: 90

Control Type: Actuated-Coordinated

Splits and Phases: 3: Gun Club Road & Jewell Avenue





Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	191	356	296	258	361	41	585	801	393	16	954	195
v/c Ratio	0.78	0.85	0.38	0.88	0.81	0.08	0.87	0.46	0.40	0.06	0.88	0.31
Control Delay	76.0	63.2	16.5	83.4	57.6	0.3	62.1	20.9	3.4	16.2	50.3	5.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	76.0	63.2	16.5	83.4	57.6	0.3	62.1	20.9	3.4	16.2	50.3	5.7
Queue Length 50th (ft)	75	263	105	103	262	0	230	190	0	5	371	0
Queue Length 95th (ft)	#117	364	174	#173	365	0	#374	300	57	17	#485	54
Internal Link Dist (ft)		2523			1479			1470			790	
Turn Bay Length (ft)	220		330	300			485		360	200		325
Base Capacity (vph)	280	487	777	308	502	545	670	1753	973	280	1086	621
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.68	0.73	0.38	0.84	0.72	0.08	0.87	0.46	0.40	0.06	0.88	0.31

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM 6th Signalized Intersection Summary
11/22/2023

3: Gun Club Road & Jewell Avenue
2027 Bkgrd + Proj - PM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑↑	↑	↑↑	↑↑	↑	↑↑	↑↑	↑↑	↑	↑	↑↑	↑↑
Traffic Volume (veh/h)	180	335	278	250	350	40	556	761	373	15	906	185
Future Volume (veh/h)	180	335	278	250	350	40	556	761	373	15	906	185
Initial Q (Q _b), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00	1.00	1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No			No			No		
Adj Sat Flow, veh/h/ln	1841	1841	1841	1841	1841	1841	1796	1796	1796	1885	1885	1885
Adj Flow Rate, veh/h	191	356	296	258	361	41	585	801	393	16	954	195
Peak Hour Factor	0.94	0.94	0.94	0.97	0.97	0.97	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, %	4	4	4	4	4	4	7	7	7	1	1	1
Cap, veh/h	244	404	602	310	439	372	553	1709	762	249	1246	556
Arrive On Green	0.07	0.22	0.22	0.09	0.24	0.24	0.17	0.50	0.50	0.01	0.35	0.35
Sat Flow, veh/h	3401	1841	1560	3401	1841	1560	3319	3413	1522	1795	3582	1598
Grp Volume(v), veh/h	191	356	296	258	361	41	585	801	393	16	954	195
Grp Sat Flow(s), veh/h/ln	1700	1841	1560	1700	1841	1560	1659	1706	1522	1795	1791	1598
Q Serve(g_s), s	6.6	22.5	17.3	9.0	22.3	2.5	20.0	18.4	20.9	0.7	28.4	10.9
Cycle Q Clear(g_c), s	6.6	22.5	17.3	9.0	22.3	2.5	20.0	18.4	20.9	0.7	28.4	10.9
Prop In Lane	1.00		1.00	1.00		1.00	1.00	1.00	1.00	1.00		1.00
Lane Grp Cap(c), veh/h	244	404	602	310	439	372	553	1709	762	249	1246	556
V/C Ratio(X)	0.78	0.88	0.49	0.83	0.82	0.11	1.06	0.47	0.52	0.06	0.77	0.35
Avail Cap(c_a), veh/h	283	491	676	312	506	429	553	1709	762	284	1246	556
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	0.79	0.79	0.79	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	54.8	45.3	27.9	53.6	43.3	35.7	50.0	19.5	20.2	24.7	34.8	29.1
Incr Delay (d2), s/veh	7.7	12.2	0.5	16.1	9.3	0.1	54.4	0.9	2.5	0.1	4.5	1.7
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	3.0	11.3	6.3	4.4	10.9	0.9	12.1	6.8	7.4	0.3	12.3	4.3
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	62.5	57.5	28.4	69.7	52.5	35.8	104.4	20.5	22.7	24.8	39.3	30.8
LnGrp LOS	E	E	C	E	D	D	F	C	C	C	D	C
Approach Vol, veh/h		843			660			1779			1165	
Approach Delay, s/veh		48.4			58.2			48.6			37.7	
Approach LOS		D			E			D			D	

Intersection Summary

HCM 6th Ctrl Delay 47.1

HCM 6th LOS D

Notes

User approved pedestrian interval to be less than phase max green.

Intersection

Int Delay, s/veh 0.3

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	0	37	10	1450	1398	15
Future Vol, veh/h	0	37	10	1450	1398	15
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	200	-	-	200
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	95	95	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	40	11	1526	1472	16

Major/Minor	Minor2	Major1	Major2		
Conflicting Flow All	-	736	1488	0	-
Stage 1	-	-	-	-	-
Stage 2	-	-	-	-	-
Critical Hdwy	-	6.94	4.14	-	-
Critical Hdwy Stg 1	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-
Follow-up Hdwy	-	3.32	2.22	-	-
Pot Cap-1 Maneuver	0	361	448	-	-
Stage 1	0	-	-	-	-
Stage 2	0	-	-	-	-
Platoon blocked, %				-	-
Mov Cap-1 Maneuver	-	361	448	-	-
Mov Cap-2 Maneuver	-	-	-	-	-
Stage 1	-	-	-	-	-
Stage 2	-	-	-	-	-

Approach	EB	NB	SB	
HCM Control Delay, s	16.2	0.1	0	
HCM LOS	C			

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	448	-	361	-	-
HCM Lane V/C Ratio	0.024	-	0.111	-	-
HCM Control Delay (s)	13.2	-	16.2	-	-
HCM Lane LOS	B	-	C	-	-
HCM 95th %tile Q(veh)	0.1	-	0.4	-	-

Lane Group	EBL	EBT	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑ ↗	↑ ↘	↑ ↗	↑ ↘	↗ ↗	↑ ↗	↑↑ ↗	↗ ↗	↑ ↗	↑↑ ↗	↗ ↗
Traffic Volume (vph)	305	5	95	5	75	17	1080	50	105	1310	20
Future Volume (vph)	305	5	95	5	75	17	1080	50	105	1310	20
Turn Type	pm+pt	NA	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases	7	4	3	8		5	2		1	6	
Permitted Phases					8	2		2	6		6
Detector Phase	7	4	3	8	8	5	2	2	1	6	6
Switch Phase											
Minimum Initial (s)	5.0	8.0	5.0	8.0	8.0	5.0	8.0	8.0	5.0	8.0	8.0
Minimum Split (s)	11.0	33.0	11.0	29.0	29.0	11.0	30.0	30.0	11.0	30.0	30.0
Total Split (s)	16.0	33.0	12.0	29.0	29.0	11.0	34.0	34.0	11.0	34.0	34.0
Total Split (%)	17.8%	36.7%	13.3%	32.2%	32.2%	12.2%	37.8%	37.8%	12.2%	37.8%	37.8%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0
Lead/Lag	Lead	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes										
Recall Mode	None	None	None	None	None	None	C-Max	C-Max	None	C-Max	C-Max

Intersection Summary

Cycle Length: 90

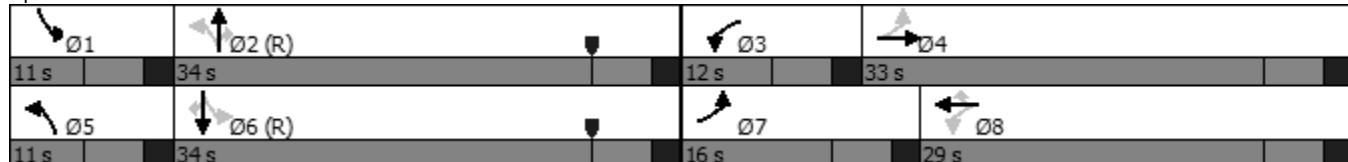
Actuated Cycle Length: 90

Offset: 0 (0%), Referenced to phase 2:NBTL and 6:SBTL, Start of Yellow

Natural Cycle: 95

Control Type: Actuated-Coordinated

Splits and Phases: 102: Gun Club Road & Yale Avenue





Lane Group	EBL	EBT	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	332	83	103	5	82	18	1137	54	114	1379	22
v/c Ratio	1.19	0.30	0.49	0.03	0.27	0.08	0.68	0.06	0.40	0.68	0.02
Control Delay	145.9	12.9	36.0	38.0	2.1	8.8	22.5	0.1	11.4	22.1	0.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	145.9	12.9	36.0	38.0	2.1	8.8	22.5	0.1	11.4	22.1	0.1
Queue Length 50th (ft)	~193	3	46	3	0	4	270	0	45	435	0
Queue Length 95th (ft)	#318	43	88	13	0	12	370	0	m25	510	m0
Internal Link Dist (ft)		816		718			2631			1641	
Turn Bay Length (ft)	150		350		10	300		300	300		300
Base Capacity (vph)	279	534	211	476	540	215	1670	843	283	2039	989
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	1.19	0.16	0.49	0.01	0.15	0.08	0.68	0.06	0.40	0.68	0.02

Intersection Summary

- ~ Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.
- m Volume for 95th percentile queue is metered by upstream signal.

HCM 6th Signalized Intersection Summary
11/22/2023

102: Gun Club Road & Yale Avenue
2027 Bkgnd + Proj - PM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑		↑	↑	↑	↑	↑↑	↑	↑	↑↑	↑
Traffic Volume (veh/h)	305	5	72	95	5	75	17	1080	50	105	1310	20
Future Volume (veh/h)	305	5	72	95	5	75	17	1080	50	105	1310	20
Initial Q (Q _b), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00			1.00	1.00		1.00	1.00	1.00	1.00	1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	332	5	78	103	5	0	18	1137	54	114	1379	22
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.95	0.92	0.92	0.95	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	399	13	199	311	165		181	1712	764	285	1826	815
Arrive On Green	0.11	0.13	0.13	0.07	0.09	0.00	0.02	0.48	0.48	0.04	0.34	0.34
Sat Flow, veh/h	1781	96	1503	1781	1870	1585	1781	3554	1585	1781	3554	1585
Grp Volume(v), veh/h	332	0	83	103	5	0	18	1137	54	114	1379	22
Grp Sat Flow(s), veh/h/ln	1781	0	1600	1781	1870	1585	1781	1777	1585	1781	1777	1585
Q Serve(g_s), s	10.0	0.0	4.3	4.7	0.2	0.0	0.5	21.9	1.6	2.9	30.9	0.8
Cycle Q Clear(g_c), s	10.0	0.0	4.3	4.7	0.2	0.0	0.5	21.9	1.6	2.9	30.9	0.8
Prop In Lane	1.00		0.94	1.00		1.00	1.00		1.00	1.00	1.00	1.00
Lane Grp Cap(c), veh/h	399	0	212	311	165		181	1712	764	285	1826	815
V/C Ratio(X)	0.83	0.00	0.39	0.33	0.03		0.10	0.66	0.07	0.40	0.76	0.03
Avail Cap(c_a), veh/h	399	0	480	311	478		244	1712	764	291	1826	815
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.67	0.67	0.67
Upstream Filter(l)	1.00	0.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	35.9	0.0	35.7	34.1	37.5	0.0	15.9	17.8	12.5	14.2	24.5	14.6
Incr Delay (d2), s/veh	13.9	0.0	1.2	0.6	0.1	0.0	0.2	2.1	0.2	0.9	3.0	0.1
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	3.8	0.0	1.7	2.0	0.1	0.0	0.2	8.0	0.6	1.0	13.5	0.3
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	49.8	0.0	36.9	34.7	37.6	0.0	16.1	19.8	12.7	15.1	27.4	14.7
LnGrp LOS	D	A	D	C	D		B	B	B	B	C	B
Approach Vol, veh/h		415			108			1209			1515	
Approach Delay, s/veh		47.2			34.9			19.5			26.3	
Approach LOS		D			C			B			C	

Intersection Summary

HCM 6th Ctrl Delay 26.7
HCM 6th LOS C

Notes

User approved pedestrian interval to be less than phase max green.

Unsignalized Delay for [WBR] is excluded from calculations of the approach delay and intersection delay.

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑	↑	↑	↑	↑↑	↑	↑	↑↑	↑
Traffic Volume (vph)	278	3	23	3	34	1403	13	33	1316	63
Future Volume (vph)	278	3	23	3	34	1403	13	33	1316	63
Turn Type	pm+pt	NA	pm+pt	NA	pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases	7	4	3	8	5	2		1	6	
Permitted Phases						2		2	6	
Detector Phase	7	4	3	8	5	2	2	1	6	6
Switch Phase										
Minimum Initial (s)	5.0	8.0	5.0	8.0	5.0	8.0	8.0	5.0	8.0	8.0
Minimum Split (s)	11.0	33.0	11.0	33.0	11.0	28.0	28.0	11.0	28.0	28.0
Total Split (s)	16.0	33.0	11.0	28.0	11.0	35.0	35.0	11.0	35.0	35.0
Total Split (%)	17.8%	36.7%	12.2%	31.1%	12.2%	38.9%	38.9%	12.2%	38.9%	38.9%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0
Lead/Lag	Lead	Lag	Lead	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes									
Recall Mode	None	None	None	None	None	C-Max	C-Max	None	C-Max	C-Max

Intersection Summary

Cycle Length: 90

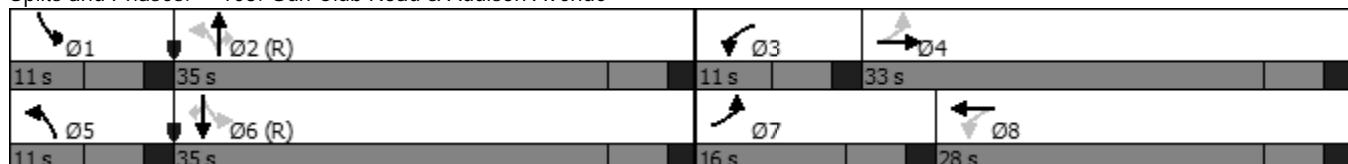
Actuated Cycle Length: 90

Offset: 0 (0%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 105

Control Type: Actuated-Coordinated

Splits and Phases: 103: Gun Club Road & Addison Avenue





Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	302	83	25	28	37	1525	14	36	1430	68
v/c Ratio	1.24	0.31	0.15	0.17	0.16	0.71	0.01	0.17	0.66	0.07
Control Delay	171.9	12.4	31.1	19.5	6.1	11.8	0.0	8.3	16.9	0.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	171.9	12.4	31.1	19.5	6.1	11.8	0.0	8.3	16.9	0.1
Queue Length 50th (ft)	~243	2	14	2	3	143	0	4	244	0
Queue Length 95th (ft)	#260	42	30	27	m10	m#247	m0	19	#528	0
Internal Link Dist (ft)		564		952		985			915	
Turn Bay Length (ft)	300		350		350		350	350		350
Base Capacity (vph)	243	533	164	413	226	2156	1035	211	2156	1035
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	1.24	0.16	0.15	0.07	0.16	0.71	0.01	0.17	0.66	0.07

Intersection Summary

- ~ Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.
- m Volume for 95th percentile queue is metered by upstream signal.

HCM 6th Signalized Intersection Summary
11/22/2023

103: Gun Club Road & Addison Avenue
2027 Bkgnd + Proj - PM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑		↑	↑		↑	↑↑		↑	↑↑	↑
Traffic Volume (veh/h)	278	3	74	23	3	23	34	1403	13	33	1316	63
Future Volume (veh/h)	278	3	74	23	3	23	34	1403	13	33	1316	63
Initial Q (Q _b), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	302	3	80	25	3	25	37	1525	14	36	1430	68
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	374	8	217	292	15	124	203	1789	798	308	1787	797
Arrive On Green	0.11	0.14	0.14	0.06	0.09	0.09	0.07	1.00	1.00	0.03	0.50	0.50
Sat Flow, veh/h	1781	58	1536	1781	173	1439	1781	3554	1585	1781	3554	1585
Grp Volume(v), veh/h	302	0	83	25	0	28	37	1525	14	36	1430	68
Grp Sat Flow(s), veh/h/ln	1781	0	1594	1781	0	1611	1781	1777	1585	1781	1777	1585
Q Serve(g_s), s	10.0	0.0	4.2	1.1	0.0	1.5	0.9	0.0	0.0	0.9	30.1	2.0
Cycle Q Clear(g_c), s	10.0	0.0	4.2	1.1	0.0	1.5	0.9	0.0	0.0	0.9	30.1	2.0
Prop In Lane	1.00		0.96	1.00		0.89	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	374	0	226	292	0	138	203	1789	798	308	1787	797
V/C Ratio(X)	0.81	0.00	0.37	0.09	0.00	0.20	0.18	0.85	0.02	0.12	0.80	0.09
Avail Cap(c_a), veh/h	374	0	478	292	0	394	242	1789	798	348	1787	797
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	2.00	2.00	2.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	35.0	0.0	35.0	33.6	0.0	38.3	14.9	0.0	0.0	9.9	18.6	11.6
Incr Delay (d2), s/veh	12.3	0.0	1.0	0.1	0.0	0.7	0.4	5.4	0.0	0.2	3.9	0.2
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	2.8	0.0	1.7	0.5	0.0	0.6	0.3	1.3	0.0	0.3	11.1	0.7
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	47.3	0.0	36.0	33.8	0.0	39.0	15.3	5.4	0.0	10.1	22.5	11.8
LnGrp LOS	D	A	D	C	A	D	B	A	A	B	C	B
Approach Vol, veh/h		385			53			1576			1534	
Approach Delay, s/veh		44.8			36.5			5.6			21.7	
Approach LOS		D			D			A			C	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	9.0	51.3	11.0	18.7	9.0	51.2	16.0	13.7				
Change Period (Y+Rc), s	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0				
Max Green Setting (Gmax), s	5.0	29.0	5.0	27.0	5.0	29.0	10.0	22.0				
Max Q Clear Time (g_c+l1), s	2.9	2.0	3.1	6.2	2.9	32.1	12.0	3.5				
Green Ext Time (p_c), s	0.0	12.0	0.0	0.4	0.0	0.0	0.0	0.1				

Intersection Summary

HCM 6th Ctrl Delay 17.3
HCM 6th LOS B

Notes

User approved pedestrian interval to be less than phase max green.

Intersection

Int Delay, s/veh 0.2

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	0	30	14	1690	1382	52
Future Vol, veh/h	0	30	14	1690	1382	52
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	300	-	-	300
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	33	15	1837	1502	57

Major/Minor	Minor2	Major1	Major2		
Conflicting Flow All	-	751	1559	0	-
Stage 1	-	-	-	-	-
Stage 2	-	-	-	-	-
Critical Hdwy	-	6.94	4.14	-	-
Critical Hdwy Stg 1	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-
Follow-up Hdwy	-	3.32	2.22	-	-
Pot Cap-1 Maneuver	0	*502	*751	-	-
Stage 1	0	-	-	-	-
Stage 2	0	-	-	-	-
Platoon blocked, %	1	1	-	-	-
Mov Cap-1 Maneuver	-	*502	*751	-	-
Mov Cap-2 Maneuver	-	-	-	-	-
Stage 1	-	-	-	-	-
Stage 2	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	12.7	0.1	0
HCM LOS	B		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	* 751	-	502	-	-
HCM Lane V/C Ratio	0.02	-	0.065	-	-
HCM Control Delay (s)	9.9	-	12.7	-	-
HCM Lane LOS	A	-	B	-	-
HCM 95th %tile Q(veh)	0.1	-	0.2	-	-

Notes

~: Volume exceeds capacity \$: Delay exceeds 300s +: Computation Not Defined *: All major volume in platoon

***Intersection Capacity Worksheets:
2050 Background +
Project***



Lane Group	EBT	EBR	WBL	WBT	SBT	SBR
Lane Configurations	↑↑↑	↗	↖↑↑	↑↑↑	↖	↗
Traffic Volume (vph)	1696	260	335	2651	5	250
Future Volume (vph)	1696	260	335	2651	5	250
Turn Type	NA	Perm	pm+pt	NA	NA	Perm
Protected Phases	4			3	8	6
Permitted Phases				4	8	6
Detector Phase	4	4	3	8	6	6
Switch Phase						
Minimum Initial (s)	8.0	8.0	4.0	8.0	8.0	8.0
Minimum Split (s)	25.0	25.0	10.0	25.0	27.0	27.0
Total Split (s)	48.0	48.0	14.0	62.0	28.0	28.0
Total Split (%)	53.3%	53.3%	15.6%	68.9%	31.1%	31.1%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	6.0
Lead/Lag	Lag	Lag	Lead			
Lead-Lag Optimize?	Yes	Yes	Yes			
Recall Mode	C-Max	C-Max	None	C-Max	Max	Max

Intersection Summary

Cycle Length: 90

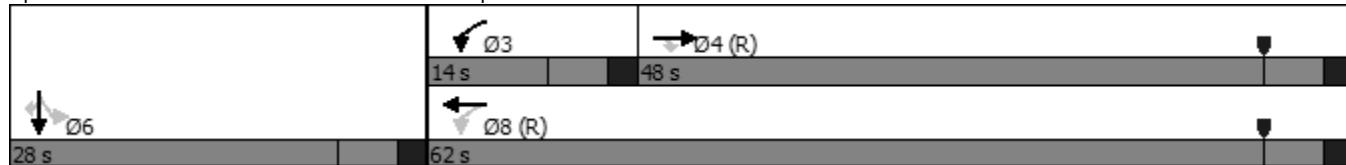
Actuated Cycle Length: 90

Offset: 68 (76%), Referenced to phase 4:EBT and 8:WBTL, Start of Yellow

Natural Cycle: 75

Control Type: Actuated-Coordinated

Splits and Phases: 1: E-470 Southbound Ramps & Jewell Avenue





Lane Group	EBT	EBR	WBL	WBT	SBT	SBR
Lane Group Flow (vph)	1731	265	342	2705	367	255
v/c Ratio	0.74	0.30	0.74	0.85	0.86	0.55
Control Delay	21.8	2.9	22.2	22.5	54.3	21.8
Queue Delay	0.0	0.0	0.0	0.2	0.0	0.0
Total Delay	21.8	2.9	22.2	22.8	54.3	21.8
Queue Length 50th (ft)	282	0	81	443	200	70
Queue Length 95th (ft)	339	40	m78	m426	#354	148
Internal Link Dist (ft)	679			530	532	
Turn Bay Length (ft)		300	250			
Base Capacity (vph)	2353	874	465	3164	425	461
Starvation Cap Reductn	0	0	0	84	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.74	0.30	0.74	0.88	0.86	0.55

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

HCM 6th Signalized Intersection Summary
11/22/2023

1: E-470 Southbound Ramps & Jewell Avenue
2050 Bkgrd + Project - AM Peak Hour



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	0	1696	260	335	2651	0	0	0	0	340	5	250
Future Volume (veh/h)	0	1696	260	335	2651	0	0	0	0	340	5	250
Initial Q (Q _b), veh	0	0	0	0	0	0				0	0	0
Ped-Bike Adj(A_pbT)	1.00			1.00	1.00					1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00				1.00	1.00	1.00
Work Zone On Approach		No			No						No	
Adj Sat Flow, veh/h/ln	0	1856	1856	1870	1870	0				1841	1841	1841
Adj Flow Rate, veh/h	0	1731	265	342	2705	0				362	5	0
Peak Hour Factor	0.92	0.98	0.98	0.98	0.98	0.92				0.94	0.98	0.98
Percent Heavy Veh, %	0	3	3	2	2	0				4	4	4
Cap, veh/h	0	2446	759	501	3177	0				423	6	
Arrive On Green	0.00	0.48	0.48	0.15	1.00	0.00				0.24	0.24	0.00
Sat Flow, veh/h	0	5233	1572	3456	5274	0				1730	24	1560
Grp Volume(v), veh/h	0	1731	265	342	2705	0				367	0	0
Grp Sat Flow(s), veh/h/ln	0	1689	1572	1728	1702	0				1754	0	1560
Q Serve(g_s), s	0.0	24.2	9.4	4.3	0.0	0.0				18.0	0.0	0.0
Cycle Q Clear(g_c), s	0.0	24.2	9.4	4.3	0.0	0.0				18.0	0.0	0.0
Prop In Lane	0.00		1.00	1.00		0.00				0.99		1.00
Lane Grp Cap(c), veh/h	0	2446	759	501	3177	0				429	0	
V/C Ratio(X)	0.00	0.71	0.35	0.68	0.85	0.00				0.86	0.00	
Avail Cap(c_a), veh/h	0	2446	759	557	3177	0				429	0	
HCM Platoon Ratio	1.00	1.00	1.00	2.00	2.00	1.00				1.00	1.00	1.00
Upstream Filter(l)	0.00	1.00	1.00	0.09	0.09	0.00				1.00	0.00	0.00
Uniform Delay (d), s/veh	0.0	18.3	14.5	16.3	0.0	0.0				32.5	0.0	0.0
Incr Delay (d2), s/veh	0.0	1.8	1.3	0.3	0.3	0.0				19.2	0.0	0.0
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0				0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	0.0	8.6	3.2	1.4	0.1	0.0				9.7	0.0	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	0.0	20.0	15.7	16.6	0.3	0.0				51.7	0.0	0.0
LnGrp LOS	A	C	B	B	A	A				D	A	
Approach Vol, veh/h		1996			3047					367		
Approach Delay, s/veh		19.5			2.1					51.7		
Approach LOS		B			A					D		

Timer - Assigned Phs	3	4	6	8
Phs Duration (G+Y+R _c), s	12.5	49.5	28.0	62.0
Change Period (Y+R _c), s	6.0	6.0	6.0	6.0
Max Green Setting (Gmax), s	8.0	42.0	22.0	56.0
Max Q Clear Time (g _{c+l1}), s	6.3	26.2	20.0	2.0
Green Ext Time (p _c), s	0.2	10.9	0.5	39.7

Intersection Summary

HCM 6th Ctrl Delay	11.9
HCM 6th LOS	B

Notes

Unsignalized Delay for [SBR] is excluded from calculations of the approach delay and intersection delay.



Lane Group	EBL	EBT	WBT	WBR	NBT	NBR
Lane Configurations	↑↑	↑↑↑	↑↑↑	↑	↑	↑
Traffic Volume (vph)	280	1756	2636	471	0	310
Future Volume (vph)	280	1756	2636	471	0	310
Turn Type	pm+pt	NA	NA	Perm	NA	Perm
Protected Phases	7	4	8		2	
Permitted Phases	4			8		2
Detector Phase	7	4	8	8	2	2
Switch Phase						
Minimum Initial (s)	3.0	8.0	8.0	8.0	8.0	8.0
Minimum Split (s)	9.5	25.0	25.0	25.0	27.0	27.0
Total Split (s)	11.0	63.0	52.0	52.0	27.0	27.0
Total Split (%)	12.2%	70.0%	57.8%	57.8%	30.0%	30.0%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	6.0
Lead/Lag	Lead		Lag	Lag		
Lead-Lag Optimize?	Yes		Yes	Yes		
Recall Mode	None	C-Max	C-Max	C-Max	Max	Max

Intersection Summary

Cycle Length: 90

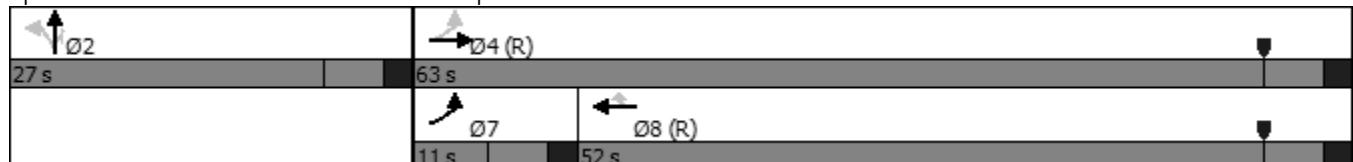
Actuated Cycle Length: 90

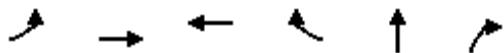
Offset: 0 (0%), Referenced to phase 4:EBTL and 8:WBT, Start of Yellow

Natural Cycle: 90

Control Type: Actuated-Coordinated

Splits and Phases: 2: E-470 Northbound Ramps & Jewell Avenue





Lane Group	EBL	EBT	WBT	WBR	NBT	NBR
Lane Group Flow (vph)	286	1792	2690	481	357	316
v/c Ratio	0.85	0.58	1.06	0.48	0.86	0.70
Control Delay	42.4	3.8	57.7	3.6	55.5	29.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	42.4	3.8	57.7	3.6	55.5	29.8
Queue Length 50th (ft)	35	45	~619	11	196	107
Queue Length 95th (ft)	m#77	54	#714	60	#350	203
Internal Link Dist (ft)		530	2523		717	
Turn Bay Length (ft)	250				175	
Base Capacity (vph)	338	3098	2549	1008	413	452
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.85	0.58	1.06	0.48	0.86	0.70

Intersection Summary

- ~ Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.
- m Volume for 95th percentile queue is metered by upstream signal.

HCM 6th Signalized Intersection Summary
11/22/2023

2: E-470 Northbound Ramps & Jewell Avenue
2050 Bkgrd + Project - AM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑↑	↑↑↑			↑↑↑	↑		↑	↑			
Traffic Volume (veh/h)	280	1756	0	0	2636	471	350	0	310	0	0	0
Future Volume (veh/h)	280	1756	0	0	2636	471	350	0	310	0	0	0
Initial Q (Q _b), veh	0	0	0	0	0	0	0	0	0			
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00	1.00	1.00			
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Work Zone On Approach		No			No			No				
Adj Sat Flow, veh/h/ln	1811	1811	0	0	1841	1841	1870	1870	1870			
Adj Flow Rate, veh/h	286	1792	0	0	2690	481	357	0	0			
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98			
Percent Heavy Veh, %	6	6	0	0	4	4	2	2	2			
Cap, veh/h	346	3131	0	0	2568	797	416	0				
Arrive On Green	0.11	1.00	0.00	0.00	0.51	0.51	0.23	0.00	0.00			
Sat Flow, veh/h	3346	5107	0	0	5191	1560	1781	0	1585			
Grp Volume(v), veh/h	286	1792	0	0	2690	481	357	0	0			
Grp Sat Flow(s), veh/h/ln	1673	1648	0	0	1675	1560	1781	0	1585			
Q Serve(g_s), s	3.6	0.0	0.0	0.0	46.0	19.6	17.3	0.0	0.0			
Cycle Q Clear(g_c), s	3.6	0.0	0.0	0.0	46.0	19.6	17.3	0.0	0.0			
Prop In Lane	1.00		0.00	0.00		1.00	1.00	1.00	1.00			
Lane Grp Cap(c), veh/h	346	3131	0	0	2568	797	416	0				
V/C Ratio(X)	0.83	0.57	0.00	0.00	1.05	0.60	0.86	0.00				
Avail Cap(c_a), veh/h	346	3131	0	0	2568	797	416	0				
HCM Platoon Ratio	2.00	2.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Upstream Filter(l)	0.60	0.60	0.00	0.00	0.09	0.09	1.00	0.00	0.00			
Uniform Delay (d), s/veh	19.9	0.0	0.0	0.0	22.0	15.6	33.1	0.0	0.0			
Incr Delay (d2), s/veh	9.7	0.5	0.0	0.0	22.6	0.3	20.0	0.0	0.0			
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0			
%ile BackOfQ(50%), veh/ln	1.8	0.1	0.0	0.0	20.5	6.1	9.6	0.0	0.0			
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	29.6	0.5	0.0	0.0	44.6	15.9	53.1	0.0	0.0			
LnGrp LOS	C	A	A	A	F	B	D	A				
Approach Vol, veh/h		2078			3171			357				
Approach Delay, s/veh		4.5			40.3			53.1				
Approach LOS		A			D			D				
Timer - Assigned Phs	2		4			7		8				
Phs Duration (G+Y+Rc), s	27.0		63.0			11.0		52.0				
Change Period (Y+Rc), s	6.0		6.0			6.0		6.0				
Max Green Setting (Gmax), s	21.0		57.0			5.0		46.0				
Max Q Clear Time (g_c+l1), s	19.3		2.0			5.6		48.0				
Green Ext Time (p_c), s	0.4		20.3			0.0		0.0				
Intersection Summary												
HCM 6th Ctrl Delay			27.8									
HCM 6th LOS			C									
Notes												
User approved pedestrian interval to be less than phase max green.												
Unsignalized Delay for [NBR] is excluded from calculations of the approach delay and intersection delay.												

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑↑	↑↑↑	↑	↑↑	↑↑↑	↑	↑↑	↑↑↑	↑	↑↑	↑↑↑	↑
Traffic Volume (vph)	365	1110	591	520	2355	505	442	1595	341	285	1080	310
Future Volume (vph)	365	1110	591	520	2355	505	442	1595	341	285	1080	310
Turn Type	Prot	NA	Free	Prot	NA	pm+ov	Prot	NA	Free	Prot	NA	pm+ov
Protected Phases	7	4		3	8	1	5	2		1	6	7
Permitted Phases			Free			8			Free			6
Detector Phase	7	4		3	8	1	5	2		1	6	7
Switch Phase												
Minimum Initial (s)	3.0	8.0		3.0	8.0	4.0	5.0	8.0		4.0	8.0	3.0
Minimum Split (s)	9.5	40.0		9.5	40.0	9.5	9.5	40.0		9.5	29.0	9.5
Total Split (s)	14.2	48.8		34.2	68.8	16.0	24.2	51.0		16.0	42.8	14.2
Total Split (%)	9.5%	32.5%		22.8%	45.9%	10.7%	16.1%	34.0%		10.7%	28.5%	9.5%
Yellow Time (s)	3.0	4.0		3.0	4.0	3.0	3.5	4.0		3.0	4.0	3.0
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	1.0	2.0		2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Total Lost Time (s)	5.0	6.0		5.0	6.0	5.0	4.5	6.0		5.0	6.0	5.0
Lead/Lag	Lead	Lag		Lead	Lag	Lead	Lead	Lag		Lead	Lag	Lead
Lead-Lag Optimize?	Yes	Yes		Yes	Yes	Yes	Yes	Yes		Yes	Yes	Yes
Recall Mode	None	None		None	None	None	None	C-Max		None	C-Max	None

Intersection Summary

Cycle Length: 150

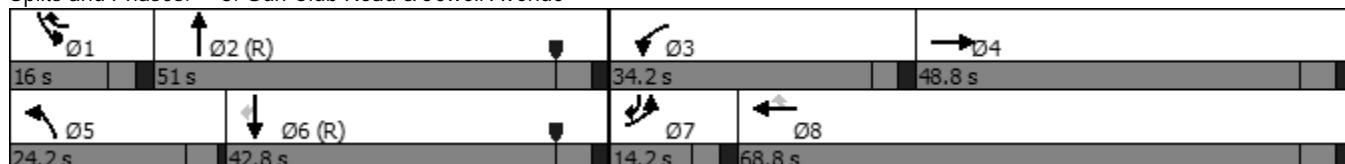
Actuated Cycle Length: 150

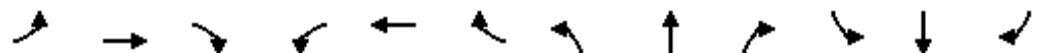
Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBT, Start of Yellow

Natural Cycle: 150

Control Type: Actuated-Coordinated

Splits and Phases: 3: Gun Club Road & Jewell Avenue





Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	372	1133	603	531	2403	515	451	1628	348	291	1102	316
v/c Ratio	1.84	0.75	0.40	0.92	1.15	0.59	1.01	1.08	0.22	1.21	0.93	0.52
Control Delay	432.9	51.1	0.8	82.3	113.7	22.6	108.8	96.4	0.3	183.2	68.8	26.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	432.9	51.1	0.8	82.3	113.7	22.6	108.8	96.4	0.3	183.2	68.8	26.0
Queue Length 50th (ft)	~282	369	0	264	~1009	277	~234	~648	0	~178	389	146
Queue Length 95th (ft)	#389	438	0	327	#1095	396	#352	#745	0	#277	#474	244
Internal Link Dist (ft)		2523			1479			1470			790	
Turn Bay Length (ft)	420		330	300		565	485		300	200		325
Base Capacity (vph)	202	1505	1524	655	2088	868	446	1510	1568	240	1189	606
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	1.84	0.75	0.40	0.81	1.15	0.59	1.01	1.08	0.22	1.21	0.93	0.52

Intersection Summary

- ~ Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

HCM 6th Signalized Intersection Summary
11/22/2023

3: Gun Club Road & Jewell Avenue
2050 Bkgrd + Project - AM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑↑	↑↑↑	↑	↑↑	↑↑↑	↑	↑↑	↑↑↑	↑	↑↑	↑↑↑	↑
Traffic Volume (veh/h)	365	1110	591	520	2355	505	442	1595	341	285	1080	310
Future Volume (veh/h)	365	1110	591	520	2355	505	442	1595	341	285	1080	310
Initial Q (Q _b), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No			No			No		
Adj Sat Flow, veh/h/ln	1811	1811	1811	1841	1841	1841	1856	1856	1856	1796	1796	1796
Adj Flow Rate, veh/h	372	1133	0	531	2403	515	451	1628	0	291	1102	316
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Percent Heavy Veh, %	6	6	6	4	4	4	3	3	3	7	7	7
Cap, veh/h	205	1544		570	2104	767	450	1520		243	1203	467
Arrive On Green	0.06	0.31	0.00	0.17	0.42	0.42	0.13	0.30	0.00	0.07	0.25	0.25
Sat Flow, veh/h	3346	4944	1535	3401	5025	1560	3428	5066	1572	3319	4904	1522
Grp Volume(v), veh/h	372	1133	0	531	2403	515	451	1628	0	291	1102	316
Grp Sat Flow(s), veh/h/ln	1673	1648	1535	1700	1675	1560	1714	1689	1572	1659	1635	1522
Q Serve(g_s), s	9.2	30.7	0.0	23.1	62.8	37.6	19.7	45.0	0.0	11.0	32.8	27.2
Cycle Q Clear(g_c), s	9.2	30.7	0.0	23.1	62.8	37.6	19.7	45.0	0.0	11.0	32.8	27.2
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	205	1544		570	2104	767	450	1520		243	1203	467
V/C Ratio(X)	1.81	0.73		0.93	1.14	0.67	1.00	1.07		1.20	0.92	0.68
Avail Cap(c_a), veh/h	205	1544		662	2104	767	450	1520		243	1203	467
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	0.77	0.77	0.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	70.4	46.0	0.0	61.6	43.6	28.9	65.1	52.5	0.0	69.5	55.1	45.5
Incr Delay (d2), s/veh	380.1	2.2	0.0	17.4	70.2	4.0	42.8	44.8	0.0	121.0	12.3	7.7
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	14.7	12.6	0.0	11.2	38.6	14.4	11.0	24.5	0.0	8.6	14.4	11.1
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	450.5	48.2	0.0	79.0	113.8	32.9	108.0	97.3	0.0	190.5	67.4	53.2
LnGrp LOS	F	D		E	F	C	F	F		F	E	D
Approach Vol, veh/h		1505			3449			2079		1709		
Approach Delay, s/veh		147.6			96.4			99.6		85.7		
Approach LOS		F			F			F		F		

Intersection Summary

HCM 6th Ctrl Delay 103.9
HCM 6th LOS F

Notes

User approved pedestrian interval to be less than phase max green.

Unsignalized Delay for [NBR, EBR] is excluded from calculations of the approach delay and intersection delay.

Intersection

Int Delay, s/veh 0.2

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	0	10	69	2399	1780	100
Future Vol, veh/h	0	10	69	2399	1780	100
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	250	-	-	250
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	98	98	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	11	75	2448	1816	109

Major/Minor	Minor2	Major1	Major2		
Conflicting Flow All	-	908	1925	0	-
Stage 1	-	-	-	-	-
Stage 2	-	-	-	-	-
Critical Hdwy	-	7.14	5.34	-	-
Critical Hdwy Stg 1	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-
Follow-up Hdwy	-	3.92	3.12	-	-
Pot Cap-1 Maneuver	0	*511	*643	-	-
Stage 1	0	-	-	-	-
Stage 2	0	-	-	-	-
Platoon blocked, %	1	1	-	-	-
Mov Cap-1 Maneuver	-	*511	*643	-	-
Mov Cap-2 Maneuver	-	-	-	-	-
Stage 1	-	-	-	-	-
Stage 2	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	12.2	0.3	0
HCM LOS	B		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	* 643	-	511	-	-
HCM Lane V/C Ratio	0.117	-	0.021	-	-
HCM Control Delay (s)	11.3	-	12.2	-	-
HCM Lane LOS	B	-	B	-	-
HCM 95th %tile Q(veh)	0.4	-	0.1	-	-

Notes

~: Volume exceeds capacity \$: Delay exceeds 300s +: Computation Not Defined *: All major volume in platoon

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑	↑, ↓	↑	↑	↑, ↑, ↑	↑	↑, ↓	↑, ↑, ↑	↑
Traffic Volume (vph)	50	5	340	5	108	2208	150	300	1365	125
Future Volume (vph)	50	5	340	5	108	2208	150	300	1365	125
Turn Type	pm+pt	NA	pm+pt	NA	pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases	7	4	3	8	5	2		1	6	
Permitted Phases						2		2	6	
Detector Phase	7	4	3	8	5	2	2	1	6	6
Switch Phase										
Minimum Initial (s)	5.0	8.0	5.0	8.0	5.0	8.0	8.0	5.0	8.0	8.0
Minimum Split (s)	11.0	33.0	11.0	33.0	11.0	30.0	30.0	11.0	30.0	30.0
Total Split (s)	11.0	33.0	11.0	33.0	15.0	59.0	59.0	17.0	61.0	61.0
Total Split (%)	9.2%	27.5%	9.2%	27.5%	12.5%	49.2%	49.2%	14.2%	50.8%	50.8%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0
Lead/Lag	Lead	Lag	Lead	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None	None	Max	None	C-Max	C-Max	None	C-Max	C-Max

Intersection Summary

Cycle Length: 120

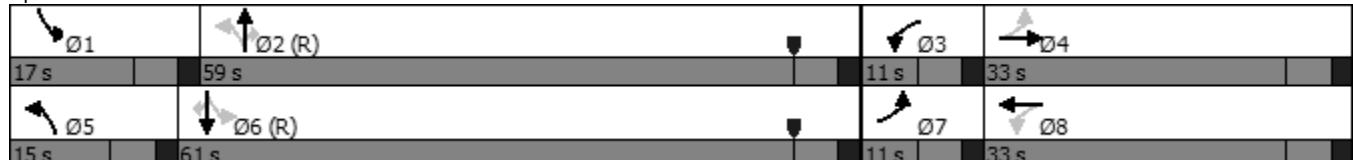
Actuated Cycle Length: 120

Offset: 53 (44%), Referenced to phase 2:NBTL and 6:SBTL, Start of Yellow

Natural Cycle: 105

Control Type: Actuated-Coordinated

Splits and Phases: 102: Gun Club Road & Yale Avenue





Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	54	18	370	233	117	2253	163	326	1393	136
v/c Ratio	0.31	0.08	0.43	0.43	0.54	0.99	0.21	0.76	0.59	0.17
Control Delay	43.1	21.9	36.2	10.6	22.3	50.9	5.7	34.6	25.1	3.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	43.1	21.9	36.2	10.6	22.3	50.9	5.7	34.6	25.1	3.6
Queue Length 50th (ft)	42	4	112	19	38	~632	12	72	288	0
Queue Length 95th (ft)	61	23	155	90	70	#761	52	123	336	35
Internal Link Dist (ft)		816		845		4562			1641	
Turn Bay Length (ft)	150				250		250	250		250
Base Capacity (vph)	176	384	865	539	227	2267	781	443	2359	807
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.31	0.05	0.43	0.43	0.52	0.99	0.21	0.74	0.59	0.17

Intersection Summary

- ~ Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

HCM 6th Signalized Intersection Summary
11/22/2023

102: Gun Club Road & Yale Avenue
2050 Bkgd + Project - AM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑		↑↑	↑		↑	↑↑↑	↑	↑↑	↑↑↑	↑
Traffic Volume (veh/h)	50	5	12	340	5	210	108	2208	150	300	1365	125
Future Volume (veh/h)	50	5	12	340	5	210	108	2208	150	300	1365	125
Initial Q (Q _b), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00			1.00	1.00		1.00	1.00	1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	54	5	0	370	5	0	117	2253	163	326	1393	136
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.98	0.92	0.92	0.98	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	430	408		855	421		251	2385	740	386	2503	777
Arrive On Green	0.03	0.22	0.00	0.04	0.22	0.00	0.05	0.47	0.47	0.07	0.49	0.49
Sat Flow, veh/h	1781	1870	0	3456	1870	0	1781	5106	1585	3456	5106	1585
Grp Volume(v), veh/h	54	5	0	370	5	0	117	2253	163	326	1393	136
Grp Sat Flow(s), veh/h/ln	1781	1870	0	1728	1870	0	1781	1702	1585	1728	1702	1585
Q Serve(g_s), s	2.8	0.3	0.0	5.0	0.2	0.0	4.1	50.5	7.3	6.5	23.0	5.7
Cycle Q Clear(g_c), s	2.8	0.3	0.0	5.0	0.2	0.0	4.1	50.5	7.3	6.5	23.0	5.7
Prop In Lane	1.00			1.00			1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	430	408		855	421		251	2385	740	386	2503	777
V/C Ratio(X)	0.13	0.01		0.43	0.01		0.47	0.94	0.22	0.85	0.56	0.18
Avail Cap(c_a), veh/h	442	421		855	421		295	2385	740	450	2503	777
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	34.5	36.8	0.0	37.3	36.1	0.0	17.6	30.5	19.0	30.6	21.4	17.1
Incr Delay (d2), s/veh	0.1	0.0	0.0	0.3	0.1	0.0	1.3	9.4	0.7	12.4	0.9	0.5
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	1.2	0.1	0.0	2.2	0.1	0.0	1.6	20.6	2.8	3.1	8.5	2.2
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	34.7	36.8	0.0	37.6	36.2	0.0	18.9	39.9	19.7	43.0	22.3	17.6
LnGrp LOS	C	D		D	D		B	D	B	D	C	B
Approach Vol, veh/h		59			375			2533			1855	
Approach Delay, s/veh		34.8			37.6			37.6			25.6	
Approach LOS		C			D			D			C	

Intersection Summary

HCM 6th Ctrl Delay 33.0
HCM 6th LOS C

Notes

Unsignalized Delay for [EBR, WBR] is excluded from calculations of the approach delay and intersection delay.

Lane Group	EBL	EBT	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑	↑	↑	↑	↑	↑↑↑	↑	↑	↑↑↑	↑
Traffic Volume (vph)	73	1	25	1	50	73	2276	50	80	1836	194
Future Volume (vph)	73	1	25	1	50	73	2276	50	80	1836	194
Turn Type	pm+pt	NA	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases	7	4	3	8		5	2		1	6	
Permitted Phases					8	2		2	6		6
Detector Phase	7	4	3	8	8	5	2	2	1	6	6
Switch Phase											
Minimum Initial (s)	5.0	8.0	5.0	8.0	8.0	5.0	8.0	8.0	5.0	8.0	8.0
Minimum Split (s)	11.0	38.0	11.0	38.0	38.0	11.0	28.0	28.0	11.0	28.0	28.0
Total Split (s)	11.0	38.0	38.0	38.0	38.0	11.0	28.0	28.0	11.0	28.0	28.0
Total Split (%)	9.6%	33.0%	33.0%	33.0%	33.0%	9.6%	24.3%	24.3%	9.6%	24.3%	24.3%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0
Lead/Lag	Lead	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes										
Recall Mode	None	None	None	None	None	None	C-Max	C-Max	None	C-Max	C-Max

Intersection Summary

Cycle Length: 115

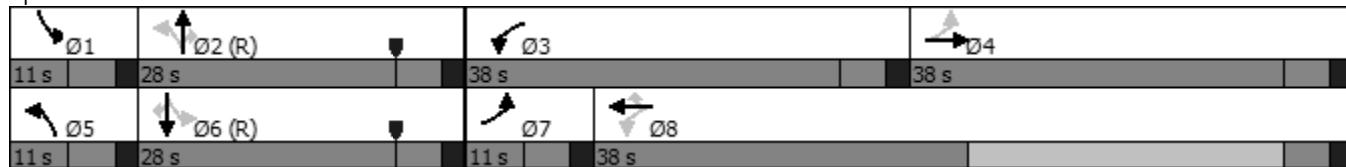
Actuated Cycle Length: 115

Offset: 19 (17%), Referenced to phase 2:NBT and 6:SBTL, Start of Yellow

Natural Cycle: 110

Control Type: Actuated-Coordinated

Splits and Phases: 103: Gun Club Road & Addison Avenue





Lane Group	EBL	EBT	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	79	22	27	1	54	79	2322	54	87	1873	211
v/c Ratio	0.53	0.14	0.14	0.01	0.21	0.39	0.71	0.05	0.46	0.54	0.19
Control Delay	58.2	22.9	40.4	47.0	1.8	13.3	17.4	0.1	21.1	13.4	2.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	58.2	22.9	40.4	47.0	1.8	13.3	17.4	0.1	21.1	13.4	2.3
Queue Length 50th (ft)	~62	1	17	1	0	14	416	0	16	285	3
Queue Length 95th (ft)	94	27	42	6	0	39	569	0	65	378	36
Internal Link Dist (ft)		564		1036			985			915	
Turn Bay Length (ft)	300				300		300	300		300	
Base Capacity (vph)	150	459	492	955	881	201	3263	1086	189	3456	1139
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.53	0.05	0.05	0.00	0.06	0.39	0.71	0.05	0.46	0.54	0.19

Intersection Summary

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

HCM 6th Signalized Intersection Summary
11/22/2023

103: Gun Club Road & Addison Avenue
2050 Bkgrd + Project - AM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑		↑	↑	↑	↑	↑↑↑	↑	↑	↑↑↑	↑
Traffic Volume (veh/h)	73	1	19	25	1	50	73	2276	50	80	1836	194
Future Volume (veh/h)	73	1	19	25	1	50	73	2276	50	80	1836	194
Initial Q (Q _b), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00			1.00	1.00		1.00	1.00	1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	79	1	21	27	1	0	79	2322	54	87	1873	211
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.98	0.92	0.92	0.98	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	232	5	102	212	122		220	3276	1017	186	3280	1018
Arrive On Green	0.04	0.07	0.07	0.04	0.07	0.00	0.04	0.64	0.64	0.04	0.64	0.64
Sat Flow, veh/h	1781	73	1524	1781	1870	1585	1781	5106	1585	1781	5106	1585
Grp Volume(v), veh/h	79	0	22	27	1	0	79	2322	54	87	1873	211
Grp Sat Flow(s), veh/h/ln	1781	0	1596	1781	1870	1585	1781	1702	1585	1781	1702	1585
Q Serve(g_s), s	4.8	0.0	1.5	1.6	0.1	0.0	1.7	34.4	1.5	1.9	23.8	6.3
Cycle Q Clear(g_c), s	4.8	0.0	1.5	1.6	0.1	0.0	1.7	34.4	1.5	1.9	23.8	6.3
Prop In Lane	1.00		0.95	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	232	0	107	212	122		220	3276	1017	186	3280	1018
V/C Ratio(X)	0.34	0.00	0.21	0.13	0.01		0.36	0.71	0.05	0.47	0.57	0.21
Avail Cap(c_a), veh/h	232	0	444	633	520		226	3276	1017	191	3280	1018
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	0.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	47.8	0.0	50.8	46.5	50.2	0.0	10.2	13.5	7.6	18.2	11.6	8.5
Incr Delay (d2), s/veh	0.9	0.0	0.9	0.3	0.0	0.0	1.0	1.3	0.1	1.8	0.7	0.5
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	2.2	0.0	0.6	0.7	0.0	0.0	0.6	11.0	0.5	1.3	7.6	2.2
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	48.6	0.0	51.7	46.8	50.3	0.0	11.2	14.9	7.7	20.1	12.3	8.9
LnGrp LOS	D	A	D	D	D		B	B	A	C	B	A
Approach Vol, veh/h	101				28			2455			2171	
Approach Delay, s/veh	49.3				46.9			14.6			12.3	
Approach LOS	D				D			B			B	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	10.7	79.8	10.8	13.7	10.6	79.9	11.0	13.5				
Change Period (Y+Rc), s	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0				
Max Green Setting (Gmax), s	5.0	22.0	32.0	32.0	5.0	22.0	5.0	32.0				
Max Q Clear Time (g_c+l1), s	3.9	36.4	3.6	3.5	3.7	25.8	6.8	2.1				
Green Ext Time (p_c), s	0.0	0.0	0.0	0.1	0.0	0.0	0.0	0.0				
Intersection Summary												
HCM 6th Ctrl Delay				14.5								
HCM 6th LOS				B								
Notes												
Unsignalized Delay for [WBR] is excluded from calculations of the approach delay and intersection delay.												

Intersection

Int Delay, s/veh 0.3

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	0	6	21	2378	2104	87
Future Vol, veh/h	0	6	21	2378	2104	87
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	300	-	-	300
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	98	98	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	7	23	2427	2147	95

Major/Minor	Minor2	Major1	Major2		
Conflicting Flow All	-	1074	2242	0	-
Stage 1	-	-	-	-	-
Stage 2	-	-	-	-	-
Critical Hdwy	-	7.14	5.34	-	-
Critical Hdwy Stg 1	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-
Follow-up Hdwy	-	3.92	3.12	-	-
Pot Cap-1 Maneuver	0	185	94	-	-
Stage 1	0	-	-	-	-
Stage 2	0	-	-	-	-
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	-	185	94	-	-
Mov Cap-2 Maneuver	-	-	-	-	-
Stage 1	-	-	-	-	-
Stage 2	-	-	-	-	-

Approach	EB	NB	SB		
HCM Control Delay, s	25.2	0.5	0		
HCM LOS	D				
<hr/>					
Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	94	-	185	-	-
HCM Lane V/C Ratio	0.243	-	0.035	-	-
HCM Control Delay (s)	55.2	-	25.2	-	-
HCM Lane LOS	F	-	D	-	-
HCM 95th %tile Q(veh)	0.9	-	0.1	-	-



Lane Group	EBT	EBR	WBL	WBT	SBT	SBR
Lane Configurations	↑↑↑	↑↓	↑↑↑	↑↑↑	↑↓	↑
Traffic Volume (vph)	2710	330	382	1903	5	355
Future Volume (vph)	2710	330	382	1903	5	355
Turn Type	NA	Perm	pm+pt	NA	NA	Perm
Protected Phases	4			3	8	6
Permitted Phases				4	8	6
Detector Phase	4	4	3	8	6	6
Switch Phase						
Minimum Initial (s)	8.0	8.0	4.0	8.0	8.0	8.0
Minimum Split (s)	25.0	25.0	12.5	25.0	27.0	27.0
Total Split (s)	60.0	60.0	14.0	74.0	36.0	36.0
Total Split (%)	54.5%	54.5%	12.7%	67.3%	32.7%	32.7%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	6.0
Lead/Lag	Lag	Lag	Lead			
Lead-Lag Optimize?	Yes	Yes	Yes			
Recall Mode	C-Max	C-Max	None	C-Max	None	None

Intersection Summary

Cycle Length: 110

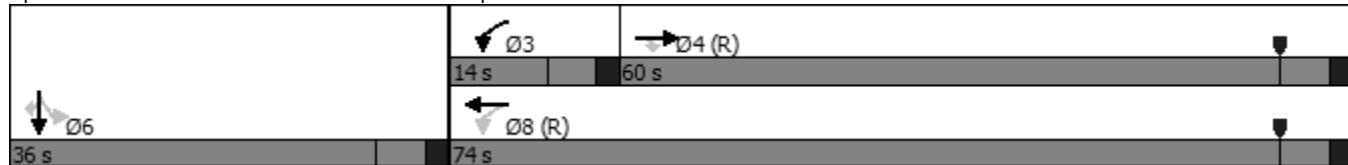
Actuated Cycle Length: 110

Offset: 0 (0%), Referenced to phase 4:EBT and 8:WBTL, Start of Yellow

Natural Cycle: 100

Control Type: Actuated-Coordinated

Splits and Phases: 1: E-470 Southbound Ramps & Jewell Avenue





Lane Group	EBT	EBR	WBL	WBT	SBT	SBR
Lane Group Flow (vph)	2765	337	390	1942	273	362
v/c Ratio	1.08	0.36	0.82	0.58	0.69	0.86
Control Delay	70.0	4.6	49.1	7.2	47.7	50.2
Queue Delay	0.0	0.0	0.0	0.1	0.0	0.0
Total Delay	70.0	4.6	49.1	7.3	47.7	50.2
Queue Length 50th (ft)	~818	21	118	119	172	184
Queue Length 95th (ft)	#909	72	m#139	m164	256	#303
Internal Link Dist (ft)	679			530	532	
Turn Bay Length (ft)		300	250			
Base Capacity (vph)	2570	937	474	3356	465	480
Starvation Cap Reductn	0	0	0	234	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	1.08	0.36	0.82	0.62	0.59	0.75

Intersection Summary

- ~ Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.
- m Volume for 95th percentile queue is metered by upstream signal.

HCM 6th Signalized Intersection Summary
11/22/2023

1: E-470 Southbound Ramps & Jewell Avenue
2050 Bkgrd + Project - PM Peak Hour



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	0	2710	330	382	1903	0	0	0	0	263	5	355
Future Volume (veh/h)	0	2710	330	382	1903	0	0	0	0	263	5	355
Initial Q (Q _b), veh	0	0	0	0	0	0				0	0	0
Ped-Bike Adj(A_pbT)	1.00			1.00	1.00		1.00			1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00				1.00	1.00	1.00
Work Zone On Approach		No			No						No	
Adj Sat Flow, veh/h/ln	0	1870	1870	1870	1870	0				1811	1811	1811
Adj Flow Rate, veh/h	0	2765	337	390	1942	0				268	5	0
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98				0.98	0.98	0.98
Percent Heavy Veh, %	0	2	2	2	2	0				6	6	6
Cap, veh/h	0	2968	921	394	3618	0				309	6	
Arrive On Green	0.00	0.58	0.58	0.15	1.00	0.00				0.18	0.18	0.00
Sat Flow, veh/h	0	5274	1585	3456	5274	0				1695	32	1535
Grp Volume(v), veh/h	0	2765	337	390	1942	0				273	0	0
Grp Sat Flow(s), veh/h/ln	0	1702	1585	1728	1702	0				1726	0	1535
Q Serve(g_s), s	0.0	54.4	12.4	7.8	0.0	0.0				16.9	0.0	0.0
Cycle Q Clear(g_c), s	0.0	54.4	12.4	7.8	0.0	0.0				16.9	0.0	0.0
Prop In Lane	0.00		1.00	1.00		0.00				0.98		1.00
Lane Grp Cap(c), veh/h	0	2968	921	394	3618	0				315	0	
V/C Ratio(X)	0.00	0.93	0.37	0.99	0.54	0.00				0.87	0.00	
Avail Cap(c_a), veh/h	0	2968	921	394	3618	0				471	0	
HCM Platoon Ratio	1.00	1.00	1.00	2.00	2.00	1.00				1.00	1.00	1.00
Upstream Filter(l)	0.00	1.00	1.00	0.25	0.25	0.00				1.00	0.00	0.00
Uniform Delay (d), s/veh	0.0	21.0	12.2	32.1	0.0	0.0				43.7	0.0	0.0
Incr Delay (d2), s/veh	0.0	6.8	1.1	20.1	0.1	0.0				10.8	0.0	0.0
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0				0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	0.0	20.4	4.2	5.8	0.0	0.0				8.1	0.0	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	0.0	27.8	13.4	52.2	0.1	0.0				54.4	0.0	0.0
LnGrp LOS	A	C	B	D	A	A				D	A	
Approach Vol, veh/h		3102			2332					273		
Approach Delay, s/veh		26.2			8.8					54.4		
Approach LOS		C			A					D		
Timer - Assigned Phs		3	4		6		8					
Phs Duration (G+Y+R _c), s		14.0	69.9		26.1		83.9					
Change Period (Y+R _c), s		6.0	6.0		6.0		6.0					
Max Green Setting (Gmax), s		8.0	54.0		30.0		68.0					
Max Q Clear Time (g _{c+l1}), s		9.8	56.4		18.9		2.0					
Green Ext Time (p _c), s		0.0	0.0		1.2		24.8					
Intersection Summary												
HCM 6th Ctrl Delay		20.5										
HCM 6th LOS		C										
Notes												
Unsignalized Delay for [SBR] is excluded from calculations of the approach delay and intersection delay.												



Lane Group	EBL	EBT	WBT	WBR	NBT	NBR
Lane Configurations	↑↑	↑↑↑	↑↑↑	↑	↑	↑
Traffic Volume (vph)	275	2698	2010	386	0	390
Future Volume (vph)	275	2698	2010	386	0	390
Turn Type	pm+pt	NA	NA	Perm	NA	Perm
Protected Phases	7	4	8		2	
Permitted Phases	4			8		2
Detector Phase	7	4	8	8	2	2
Switch Phase						
Minimum Initial (s)	3.0	8.0	8.0	8.0	8.0	8.0
Minimum Split (s)	9.5	25.0	25.0	25.0	27.0	27.0
Total Split (s)	13.0	69.0	56.0	56.0	41.0	41.0
Total Split (%)	11.8%	62.7%	50.9%	50.9%	37.3%	37.3%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	6.0
Lead/Lag	Lead		Lag	Lag		
Lead-Lag Optimize?	Yes		Yes	Yes		
Recall Mode	None	C-Max	C-Max	C-Max	Max	Max

Intersection Summary

Cycle Length: 110

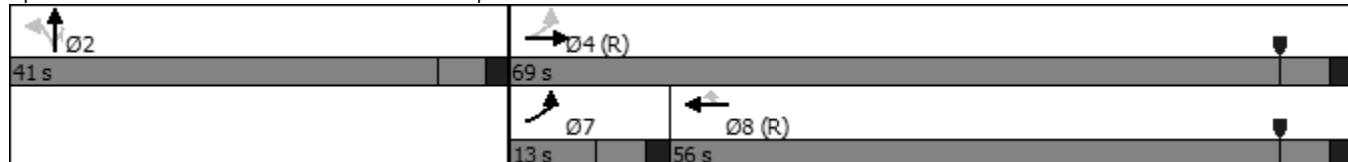
Actuated Cycle Length: 110

Offset: 0 (0%), Referenced to phase 4:EBTL and 8:WBT, Start of Yellow

Natural Cycle: 80

Control Type: Actuated-Coordinated

Splits and Phases: 2: E-470 Northbound Ramps & Jewell Avenue





Lane Group	EBL	EBT	WBT	WBR	NBT	NBR
Lane Group Flow (vph)	281	2753	2051	394	281	398
v/c Ratio	0.82	0.96	0.93	0.45	0.51	0.72
Control Delay	39.4	14.2	37.4	5.0	34.6	34.2
Queue Delay	0.0	2.3	0.0	0.0	0.0	0.0
Total Delay	39.4	16.5	37.4	5.0	34.6	34.2
Queue Length 50th (ft)	70	202	487	19	161	195
Queue Length 95th (ft)	m67	m176	#579	80	246	314
Internal Link Dist (ft)		530	2523		717	
Turn Bay Length (ft)	250				175	
Base Capacity (vph)	342	2856	2203	874	546	550
Starvation Cap Reductn	0	59	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.82	0.98	0.93	0.45	0.51	0.72

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

HCM 6th Signalized Intersection Summary
11/22/2023

2: E-470 Northbound Ramps & Jewell Avenue
2050 Bkgrd + Project - PM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	275	2698	0	0	2010	386	275	0	390	0	0	0
Future Volume (veh/h)	275	2698	0	0	2010	386	275	0	390	0	0	0
Initial Q (Q _b), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00			
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Work Zone On Approach		No			No			No				
Adj Sat Flow, veh/h/ln	1841	1841	0	0	1796	1796	1826	1826	1826			
Adj Flow Rate, veh/h	281	2753	0	0	2051	394	281	0	0			
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98			
Percent Heavy Veh, %	4	4	0	0	7	7	5	5	5			
Cap, veh/h	357	2878	0	0	2240	695	553	0				
Arrive On Green	0.12	1.00	0.00	0.00	0.46	0.46	0.32	0.00	0.00			
Sat Flow, veh/h	3401	5191	0	0	5065	1522	1739	0	1547			
Grp Volume(v), veh/h	281	2753	0	0	2051	394	281	0	0			
Grp Sat Flow(s), veh/h/ln	1700	1675	0	0	1635	1522	1739	0	1547			
Q Serve(g_s), s	4.7	0.0	0.0	0.0	43.0	20.9	14.5	0.0	0.0			
Cycle Q Clear(g_c), s	4.7	0.0	0.0	0.0	43.0	20.9	14.5	0.0	0.0			
Prop In Lane	1.00		0.00	0.00		1.00	1.00		1.00			
Lane Grp Cap(c), veh/h	357	2878	0	0	2240	695	553	0				
V/C Ratio(X)	0.79	0.96	0.00	0.00	0.92	0.57	0.51	0.00				
Avail Cap(c_a), veh/h	365	2878	0	0	2240	695	553	0				
HCM Platoon Ratio	2.00	2.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Upstream Filter(l)	0.09	0.09	0.00	0.00	0.09	0.09	1.00	0.00	0.00			
Uniform Delay (d), s/veh	23.4	0.0	0.0	0.0	27.9	21.9	30.5	0.0	0.0			
Incr Delay (d2), s/veh	1.1	1.2	0.0	0.0	0.8	0.3	3.3	0.0	0.0			
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0			
%ile BackOfQ(50%), veh/ln	1.6	0.3	0.0	0.0	15.5	7.0	6.5	0.0	0.0			
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	24.5	1.2	0.0	0.0	28.7	22.2	33.8	0.0	0.0			
LnGrp LOS	C	A	A	A	C	C	C	A				
Approach Vol, veh/h		3034			2445			281				
Approach Delay, s/veh		3.3			27.6			33.8				
Approach LOS		A			C			C				
Timer - Assigned Phs	2		4			7		8				
Phs Duration (G+Y+Rc), s	41.0		69.0			12.7		56.3				
Change Period (Y+Rc), s	6.0		6.0			6.0		6.0				
Max Green Setting (Gmax), s	35.0		63.0			7.0		50.0				
Max Q Clear Time (g_c+l1), s	16.5		2.0			6.7		45.0				
Green Ext Time (p_c), s	1.6		44.5			0.0		4.5				
Intersection Summary												
HCM 6th Ctrl Delay		15.1										
HCM 6th LOS		B										
Notes												
User approved pedestrian interval to be less than phase max green.												
Unsignalized Delay for [NBR] is excluded from calculations of the approach delay and intersection delay.												

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑↑	↑↑↑	↑	↑↑	↑↑↑	↑	↑↑	↑↑↑	↑	↑↑	↑↑↑	↑
Traffic Volume (vph)	495	2205	388	485	1705	470	441	1371	533	505	1566	250
Future Volume (vph)	495	2205	388	485	1705	470	441	1371	533	505	1566	250
Turn Type	Prot	NA	Free	Prot	NA	pm+ov	Prot	NA	Free	Prot	NA	pm+ov
Protected Phases	7	4		3	8	1	5	2		1	6	7
Permitted Phases			Free			8			Free			6
Detector Phase	7	4		3	8	1	5	2		1	6	7
Switch Phase												
Minimum Initial (s)	3.0	4.0		3.0	5.0	4.0	4.0	8.0		4.0	8.0	3.0
Minimum Split (s)	9.5	32.0		9.5	32.0	9.5	9.5	40.0		9.5	29.0	9.5
Total Split (s)	26.8	64.0		16.0	53.2	22.0	21.0	48.0		22.0	49.0	26.8
Total Split (%)	17.9%	42.7%		10.7%	35.5%	14.7%	14.0%	32.0%		14.7%	32.7%	17.9%
Yellow Time (s)	3.0	3.0		3.0	3.0	3.0	3.0	4.0		3.0	4.0	3.0
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	2.0	2.0		2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0		5.0	5.0	5.0	5.0	6.0		5.0	6.0	5.0
Lead/Lag	Lead	Lag		Lead	Lag	Lead	Lead	Lag		Lead	Lag	Lead
Lead-Lag Optimize?	Yes	Yes		Yes	Yes	Yes	Yes	Yes		Yes	Yes	Yes
Recall Mode	None	None		None	None	None	None	C-Max		None	C-Max	None

Intersection Summary

Cycle Length: 150

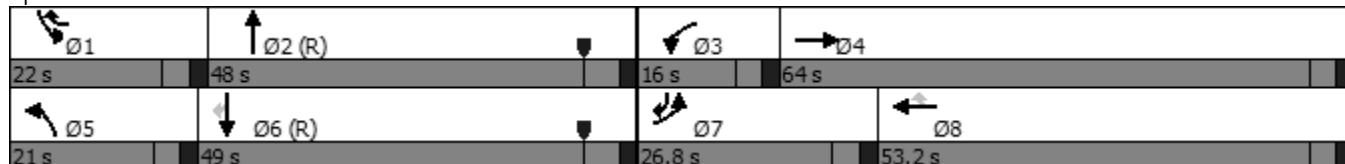
Actuated Cycle Length: 150

Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBT, Start of Yellow

Natural Cycle: 145

Control Type: Actuated-Coordinated

Splits and Phases: 3: Gun Club Road & Jewell Avenue





Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	505	2250	396	495	1740	480	450	1399	544	515	1598	255
v/c Ratio	1.03	1.15	0.25	2.01	1.09	0.62	1.29	1.03	0.36	1.31	1.09	0.33
Control Delay	110.5	114.1	0.4	501.8	97.1	27.5	200.7	84.9	0.7	207.7	99.9	20.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	110.5	114.1	0.4	501.8	97.1	27.5	200.7	84.9	0.7	207.7	99.9	20.7
Queue Length 50th (ft)	~272	~942	0	~388	~697	280	~288	~535	0	~332	~640	121
Queue Length 95th (ft)	#389	#1031	0	#505	#792	404	#402	#632	0	#451	#736	189
Internal Link Dist (ft)		2523			1479			1470			790	
Turn Bay Length (ft)	420		330	300		565	485		300	200		325
Base Capacity (vph)	489	1961	1553	246	1602	777	349	1357	1509	392	1472	781
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	1.03	1.15	0.25	2.01	1.09	0.62	1.29	1.03	0.36	1.31	1.09	0.33

Intersection Summary

- ~ Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

HCM 6th Signalized Intersection Summary
11/22/2023

3: Gun Club Road & Jewell Avenue
2050 Bkgrd + Project - PM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑↑	↑↑↑	↑	↑↑	↑↑↑	↑	↑↑	↑↑↑	↑	↑↑	↑↑↑	↑
Traffic Volume (veh/h)	495	2205	388	485	1705	470	441	1371	533	505	1566	250
Future Volume (veh/h)	495	2205	388	485	1705	470	441	1371	533	505	1566	250
Initial Q (Q _b), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No		No			No			No		No	
Adj Sat Flow, veh/h/ln	1841	1841	1841	1841	1841	1841	1796	1796	1796	1885	1885	1885
Adj Flow Rate, veh/h	505	2250	0	495	1740	480	450	1399	0	515	1598	255
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Percent Heavy Veh, %	4	4	4	4	4	4	7	7	7	1	1	1
Cap, veh/h	494	1977		249	1615	678	354	1373		395	1475	690
Arrive On Green	0.15	0.39	0.00	0.07	0.32	0.32	0.11	0.28	0.00	0.11	0.29	0.29
Sat Flow, veh/h	3401	5025	1560	3401	5025	1560	3319	4904	1522	3483	5147	1598
Grp Volume(v), veh/h	505	2250	0	495	1740	480	450	1399	0	515	1598	255
Grp Sat Flow(s), veh/h/ln	1700	1675	1560	1700	1675	1560	1659	1635	1522	1742	1716	1598
Q Serve(g_s), s	21.8	59.0	0.0	11.0	48.2	37.7	16.0	42.0	0.0	17.0	43.0	16.2
Cycle Q Clear(g_c), s	21.8	59.0	0.0	11.0	48.2	37.7	16.0	42.0	0.0	17.0	43.0	16.2
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	494	1977		249	1615	678	354	1373		395	1475	690
V/C Ratio(X)	1.02	1.14		1.98	1.08	0.71	1.27	1.02		1.30	1.08	0.37
Avail Cap(c_a), veh/h	494	1977		249	1615	678	354	1373		395	1475	690
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	0.24	0.24	0.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	64.1	45.5	0.0	69.5	50.9	34.6	67.0	54.0	0.0	66.5	53.5	28.8
Incr Delay (d2), s/veh	25.5	64.0	0.0	457.2	46.5	3.4	142.4	29.1	0.0	154.4	49.4	1.5
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	11.0	35.3	0.0	20.5	26.6	14.5	13.6	20.2	0.0	15.8	24.5	6.4
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	89.6	109.5	0.0	526.7	97.4	38.0	209.4	83.1	0.0	220.9	102.9	30.3
LnGrp LOS	F	F		F	F	D	F	F		F	F	C
Approach Vol, veh/h		2755			2715			1849			2368	
Approach Delay, s/veh		105.9			165.2			113.9			120.8	
Approach LOS		F			F			F			F	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	22.0	48.0	16.0	64.0	21.0	49.0	26.8	53.2				
Change Period (Y+Rc), s	5.0	6.0	5.0	5.0	5.0	6.0	5.0	5.0				
Max Green Setting (Gmax), s	17.0	42.0	11.0	59.0	16.0	43.0	21.8	48.2				
Max Q Clear Time (g_c+l1), s	19.0	44.0	13.0	61.0	18.0	45.0	23.8	50.2				
Green Ext Time (p_c), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0				

Intersection Summary

HCM 6th Ctrl Delay 127.7
HCM 6th LOS F

Notes

User approved pedestrian interval to be less than phase max green.

Unsignalized Delay for [NBR, EBR] is excluded from calculations of the approach delay and intersection delay.

Intersection

Int Delay, s/veh 0.2

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	0	37	10	2060	2373	15
Future Vol, veh/h	0	37	10	2060	2373	15
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	250	-	-	250
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	98	98	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	40	11	2102	2421	16

Major/Minor	Minor2	Major1	Major2		
Conflicting Flow All	-	1211	2437	0	-
Stage 1	-	-	-	-	-
Stage 2	-	-	-	-	-
Critical Hdwy	-	7.14	5.34	-	-
Critical Hdwy Stg 1	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-
Follow-up Hdwy	-	3.92	3.12	-	-
Pot Cap-1 Maneuver	0	*378	*476	-	-
Stage 1	0	-	-	-	-
Stage 2	0	-	-	-	-
Platoon blocked, %	1	1	-	-	-
Mov Cap-1 Maneuver	-	*378	*476	-	-
Mov Cap-2 Maneuver	-	-	-	-	-
Stage 1	-	-	-	-	-
Stage 2	-	-	-	-	-

Approach EB NB SB

HCM Control Delay, s	15.7	0.1	0
HCM LOS	C		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	* 476	-	378	-	-
HCM Lane V/C Ratio	0.023	-	0.106	-	-
HCM Control Delay (s)	12.7	-	15.7	-	-
HCM Lane LOS	B	-	C	-	-
HCM 95th %tile Q(veh)	0.1	-	0.4	-	-

Notes

~: Volume exceeds capacity \$: Delay exceeds 300s +: Computation Not Defined *: All major volume in platoon

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑	↑	↑	↑	↑↑↑	↑	↑	↑↑↑	↑
Traffic Volume (vph)	305	5	460	5	17	1445	325	450	1940	20
Future Volume (vph)	305	5	460	5	17	1445	325	450	1940	20
Turn Type	pm+pt	NA	pm+pt	NA	pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases	7	4	3	8	5	2		1	6	
Permitted Phases						2		2	6	
Detector Phase	7	4	3	8	5	2	2	1	6	6
Switch Phase										
Minimum Initial (s)	5.0	8.0	5.0	8.0	5.0	8.0	8.0	5.0	8.0	8.0
Minimum Split (s)	11.0	33.0	11.0	33.0	11.0	30.0	30.0	11.0	30.0	30.0
Total Split (s)	21.0	37.0	17.0	33.0	11.0	46.0	46.0	20.0	55.0	55.0
Total Split (%)	17.5%	30.8%	14.2%	27.5%	9.2%	38.3%	38.3%	16.7%	45.8%	45.8%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0
Lead/Lag	Lead	Lag	Lead	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes									
Recall Mode	None	None	None	Max	None	C-Max	C-Max	None	C-Max	C-Max

Intersection Summary

Cycle Length: 120

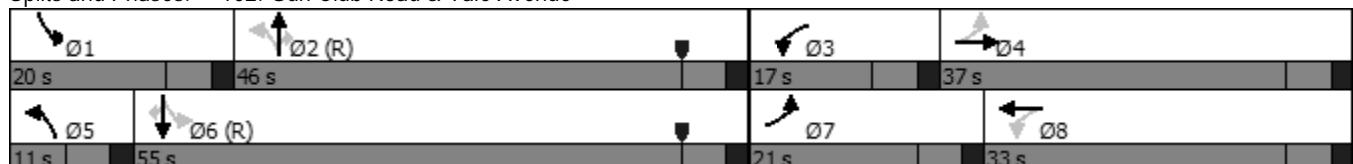
Actuated Cycle Length: 120

Offset: 0 (0%), Referenced to phase 2:NBTL and 6:SBTL, Start of Yellow

Natural Cycle: 105

Control Type: Actuated-Coordinated

Splits and Phases: 102: Gun Club Road & Yale Avenue





Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	332	83	500	353	18	1474	353	489	1980	22
v/c Ratio	1.03	0.20	0.54	0.72	0.13	0.87	0.48	0.93	0.84	0.03
Control Delay	91.1	9.8	29.8	31.1	18.4	44.3	6.8	70.1	30.6	0.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	91.1	9.8	29.8	31.1	18.4	44.3	6.8	70.1	30.6	0.1
Queue Length 50th (ft)	188	3	138	132	7	390	15	175	306	0
Queue Length 95th (ft)	#333	43	182	246	19	455	87	m#248	#635	m0
Internal Link Dist (ft)		816		845		4562			1641	
Turn Bay Length (ft)	150				250		250	250		250
Base Capacity (vph)	322	471	928	489	137	1695	743	523	2356	806
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	1.03	0.18	0.54	0.72	0.13	0.87	0.48	0.93	0.84	0.03

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

HCM 6th Signalized Intersection Summary
11/22/2023

102: Gun Club Road & Yale Avenue
2050 Bkgd + Project - PM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑		↑↑	↑		↑	↑↑↑		↑↑	↑↑↑	↑
Traffic Volume (veh/h)	305	5	72	460	5	320	17	1445	325	450	1940	20
Future Volume (veh/h)	305	5	72	460	5	320	17	1445	325	450	1940	20
Initial Q (Q _b), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00			1.00	1.00		1.00	1.00	1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	332	5	0	500	5	0	18	1474	353	489	1980	22
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.98	0.92	0.92	0.98	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	597	483		1053	421		113	1729	537	540	2202	683
Arrive On Green	0.13	0.26	0.00	0.09	0.22	0.00	0.02	0.34	0.34	0.15	0.57	0.57
Sat Flow, veh/h	1781	1870	0	3456	1870	0	1781	5106	1585	3456	5106	1585
Grp Volume(v), veh/h	332	5	0	500	5	0	18	1474	353	489	1980	22
Grp Sat Flow(s), veh/h/ln	1781	1870	0	1728	1870	0	1781	1702	1585	1728	1702	1585
Q Serve(g_s), s	15.0	0.2	0.0	11.0	0.2	0.0	0.8	32.2	22.7	11.2	41.0	0.7
Cycle Q Clear(g_c), s	15.0	0.2	0.0	11.0	0.2	0.0	0.8	32.2	22.7	11.2	41.0	0.7
Prop In Lane	1.00			1.00			0.00	1.00	1.00	1.00		1.00
Lane Grp Cap(c), veh/h	597	483		1053	421		113	1729	537	540	2202	683
V/C Ratio(X)	0.56	0.01		0.47	0.01		0.16	0.85	0.66	0.91	0.90	0.03
Avail Cap(c_a), veh/h	597	483		1053	421		153	1729	537	558	2202	683
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.33	1.33	1.33
Upstream Filter(l)	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	31.2	33.1	0.0	32.8	36.1	0.0	29.1	36.9	33.8	27.9	23.3	14.7
Incr Delay (d2), s/veh	1.1	0.0	0.0	0.3	0.1	0.0	0.7	5.6	6.2	18.1	6.4	0.1
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	7.9	0.1	0.0	5.9	0.1	0.0	0.3	13.3	9.6	5.3	13.4	0.3
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	32.4	33.1	0.0	33.2	36.2	0.0	29.7	42.5	40.0	46.0	29.7	14.8
LnGrp LOS	C	C		C	D		C	D	D	D	C	B
Approach Vol, veh/h		337			505			1845			2491	
Approach Delay, s/veh		32.4			33.2			41.9			32.8	
Approach LOS		C			C			D			C	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	19.4	46.6	17.0	37.0	8.3	57.7	21.0	33.0				
Change Period (Y+Rc), s	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0				
Max Green Setting (Gmax), s	14.0	40.0	11.0	31.0	5.0	49.0	15.0	27.0				
Max Q Clear Time (g_c+l1), s	13.2	34.2	13.0	2.2	2.8	43.0	17.0	2.2				
Green Ext Time (p_c), s	0.2	4.3	0.0	0.0	0.0	5.0	0.0	0.0				
Intersection Summary												
HCM 6th Ctrl Delay			36.0									
HCM 6th LOS			D									
Notes												
Unsignalized Delay for [EBR, WBR] is excluded from calculations of the approach delay and intersection delay.												

Lane Group	EBL	EBT	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑ ↗	↑ ↘	↑ ↗	↑ ↘	↗ ↗	↑ ↗	↑↑↑	↗ ↗	↑ ↗	↑↑↑	↗ ↗
Traffic Volume (vph)	278	3	88	3	103	34	1978	48	128	2226	63
Future Volume (vph)	278	3	88	3	103	34	1978	48	128	2226	63
Turn Type	pm+pt	NA	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases	7	4	3	8		5	2		1	6	
Permitted Phases					8	2		2	6		6
Detector Phase	7	4	3	8	8	5	2	2	1	6	6
Switch Phase											
Minimum Initial (s)	5.0	8.0	5.0	8.0	8.0	5.0	8.0	8.0	5.0	8.0	8.0
Minimum Split (s)	11.0	38.0	11.0	38.0	38.0	11.0	28.0	28.0	11.0	28.0	28.0
Total Split (s)	25.0	39.0	14.0	28.0	28.0	11.0	55.0	55.0	12.0	56.0	56.0
Total Split (%)	20.8%	32.5%	11.7%	23.3%	23.3%	9.2%	45.8%	45.8%	10.0%	46.7%	46.7%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0
Lead/Lag	Lead	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes										
Recall Mode	Max	None	None	None	None	None	C-Max	C-Max	None	C-Max	C-Max

Intersection Summary

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 0 (0%), Referenced to phase 2:NBTL and 6:SBTL, Start of Yellow

Natural Cycle: 110

Control Type: Actuated-Coordinated

Splits and Phases: 103: Gun Club Road & Addison Avenue





Lane Group	EBL	EBT	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	302	83	96	3	112	37	2018	52	139	2271	68
v/c Ratio	0.80	0.23	0.48	0.02	0.40	0.23	0.85	0.06	0.56	0.80	0.07
Control Delay	55.0	12.1	43.5	53.0	4.1	21.1	50.3	0.6	28.2	24.8	0.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	55.0	12.1	43.5	53.0	4.1	21.1	50.3	0.6	28.2	24.8	0.1
Queue Length 50th (ft)	205	2	57	2	0	19	541	0	49	536	0
Queue Length 95th (ft)	#337	48	103	13	0	m22	m589	m0	116	623	0
Internal Link Dist (ft)		564		1036			985			915	
Turn Bay Length (ft)	300				300		300	300		300	
Base Capacity (vph)	379	496	205	341	446	158	2388	844	249	2848	971
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.80	0.17	0.47	0.01	0.25	0.23	0.85	0.06	0.56	0.80	0.07

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

HCM 6th Signalized Intersection Summary
11/22/2023

103: Gun Club Road & Addison Avenue
2050 Bkgrd + Project - PM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑		↑	↑	↑	↑	↑↑↑	↑	↑	↑↑↑	↑
Traffic Volume (veh/h)	278	3	74	88	3	103	34	1978	48	128	2226	63
Future Volume (veh/h)	278	3	74	88	3	103	34	1978	48	128	2226	63
Initial Q (Q _b), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00			1.00	1.00		1.00	1.00	1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	302	3	80	96	3	0	37	2018	52	139	2271	68
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.98	0.92	0.92	0.98	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	434	9	244	265	124		141	2681	832	204	2786	865
Arrive On Green	0.16	0.16	0.16	0.07	0.07	0.00	0.04	0.70	0.70	0.05	0.55	0.55
Sat Flow, veh/h	1781	58	1536	1781	1870	1585	1781	5106	1585	1781	5106	1585
Grp Volume(v), veh/h	302	0	83	96	3	0	37	2018	52	139	2271	68
Grp Sat Flow(s), veh/h/ln	1781	0	1594	1781	1870	1585	1781	1702	1585	1781	1702	1585
Q Serve(g_s), s	18.6	0.0	5.5	5.9	0.2	0.0	1.1	30.1	1.2	4.3	43.7	2.4
Cycle Q Clear(g_c), s	18.6	0.0	5.5	5.9	0.2	0.0	1.1	30.1	1.2	4.3	43.7	2.4
Prop In Lane	1.00		0.96	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	434	0	253	265	124		141	2681	832	204	2786	865
V/C Ratio(X)	0.70	0.00	0.33	0.36	0.02		0.26	0.75	0.06	0.68	0.82	0.08
Avail Cap(c_a), veh/h	434	0	438	266	343		162	2681	832	204	2786	865
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.33	1.33	1.33	1.00	1.00	1.00
Upstream Filter(l)	1.00	0.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	41.6	0.0	44.8	47.7	52.4	0.0	21.2	13.1	8.8	21.8	22.3	12.9
Incr Delay (d2), s/veh	8.9	0.0	0.7	0.8	0.1	0.0	1.0	2.0	0.1	8.9	2.8	0.2
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	9.2	0.0	2.3	2.7	0.1	0.0	0.4	7.2	0.5	2.4	15.9	0.9
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	50.5	0.0	45.5	48.5	52.4	0.0	22.2	15.1	8.9	30.6	25.1	13.1
LnGrp LOS	D	A	D	D	D		C	B	A	C	C	B
Approach Vol, veh/h		385			99			2107			2478	
Approach Delay, s/veh		49.4			48.7			15.1			25.1	
Approach LOS		D			D			B			C	

Intersection Summary

HCM 6th Ctrl Delay	23.2
HCM 6th LOS	C

Notes

User approved pedestrian interval to be less than phase max green.

Unsignalized Delay for [WBR] is excluded from calculations of the approach delay and intersection delay.

Intersection

Int Delay, s/veh 0.4

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	0	30	14	2345	2387	52
Future Vol, veh/h	0	30	14	2345	2387	52
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	300	-	-	300
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	98	98	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	33	15	2393	2436	57

Major/Minor	Minor2	Major1	Major2		
Conflicting Flow All	-	1218	2493	0	-
Stage 1	-	-	-	-	-
Stage 2	-	-	-	-	-
Critical Hdwy	-	7.14	5.34	-	-
Critical Hdwy Stg 1	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-
Follow-up Hdwy	-	3.92	3.12	-	-
Pot Cap-1 Maneuver	0	148	70	-	-
Stage 1	0	-	-	-	-
Stage 2	0	-	-	-	-
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	-	148	70	-	-
Mov Cap-2 Maneuver	-	-	-	-	-
Stage 1	-	-	-	-	-
Stage 2	-	-	-	-	-

Approach	EB	NB	SB	
HCM Control Delay, s	36.1	0.4	0	
HCM LOS	E			

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	70	-	148	-	-
HCM Lane V/C Ratio	0.217	-	0.22	-	-
HCM Control Delay (s)	70.2	-	36.1	-	-
HCM Lane LOS	F	-	E	-	-
HCM 95th %tile Q(veh)	0.8	-	0.8	-	-

***Signal Warrant
Worksheets***



Intersection: Jewell Ave. at E-470 Southbound Ramps

Warrant 2: 4 Hour Analysis - 2023 Existing Volumes

	Major	Minor*		Warrant 2 (Figure 4C-2)
	Jewell Avenue	E-470 SB Ramp		
Time of Day	Number of Lanes			
	2	1		
0:00				no
1:00				no
2:00				no
3:00				no
4:00	256	35		no
5:00	603	82		no
6:00	1,115	152		Yes
7:00	1,507	206		Yes
8:00	1,251	171		Yes
9:00	919	126		Yes
10:00			Warrant is Met (yes/no)	no
11:00				no
12:00				no
13:00				no
14:00	1,173	193		Yes
15:00	1,466	241		Yes
16:00	1,629	268		Yes
17:00	1,580	260		Yes
18:00	1,205	198		Yes
19:00	782	129		Yes
20:00				no
21:00				no
22:00				no
23:00				no
Total	13,486	2,061	10	Met

*70% factor applied since posted speed limit is above 40mph. The minor volume for each hour represents the higher of either minor approach.

Intersection: Jewell Ave. at E-470 Northbound Ramps

Warrant 2: 4 Hour Analysis - 2023 Existing Volumes

	Major	Minor*	Warrant 2 (Figure 4C-2)
	Jewell Avenue	E-470 NB Ramp	
Time of Day	Number of Lanes		
	2	1	
0:00			no
1:00			no
2:00			no
3:00			no
4:00	214	37	no
5:00	504	87	no
6:00	932	161	Yes
7:00	1,260	218	Yes
8:00	1,046	181	Yes
9:00	769	133	Yes
10:00			no
11:00			no
12:00			no
13:00			no
14:00	969	120	Yes
15:00	1,211	150	Yes
16:00	1,346	167	Yes
17:00	1,306	162	Yes
18:00	996	124	Yes
19:00	646	80	no
20:00			no
21:00			no
22:00			no
23:00			no
Total	11,199	1,620	9
			Met

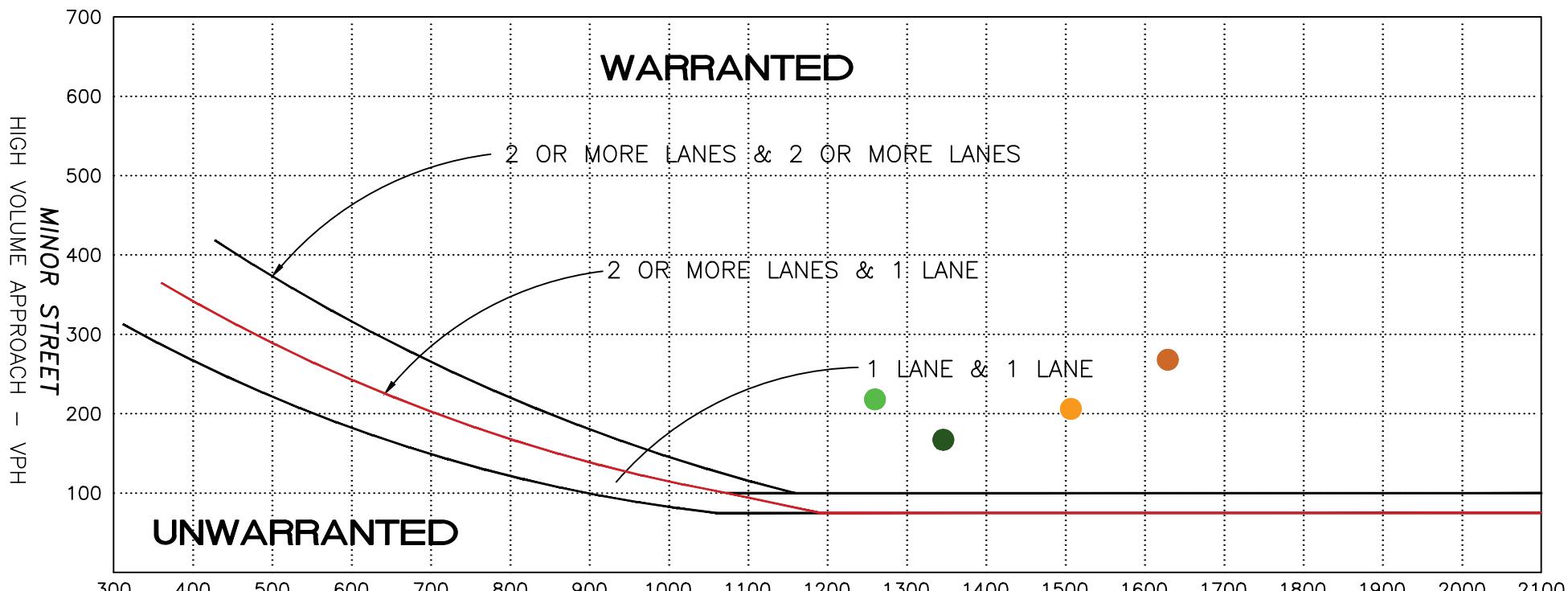
*70% factor applied since posted speed limit is above 40mph. The minor volume for each hour represents the higher of either minor approach.

PEAK HOUR VOLUME WARRANT (70%)
APPLIED FOR PLANNING PURPOSES

KEY FOR INTERSECTIONS

MET	AM	PM
Yes	Orange Circle	Brown Circle
Yes	Green Circle	Black Circle

E-470 Southbound Ramp: Existing
E-470 Northbound Ramp: Existing



NOTE: 100 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACH WITH TWO OR MORE LANES AND 75 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACHING WITH ONE LANE.

FOX TUTTLE
TRANSPORTATION GROUP

GUN CLUB BUSINESS PARK TRAFFIC IMPACT STUDY - AURORA, CO

PEAK HOUR SIGNAL WARRANT - JEWELL AVENUE AT E-470 RAMPS

FT Project #	23087	Original Scale	NTS	Date	3/14/2022	Drawn by	CRS	Figure #	APPENDIX 1
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Intersection: Gun Club Road at Yale Avenue

Warrant 2: 4 Hour Analysis - 2027 Background Volumes

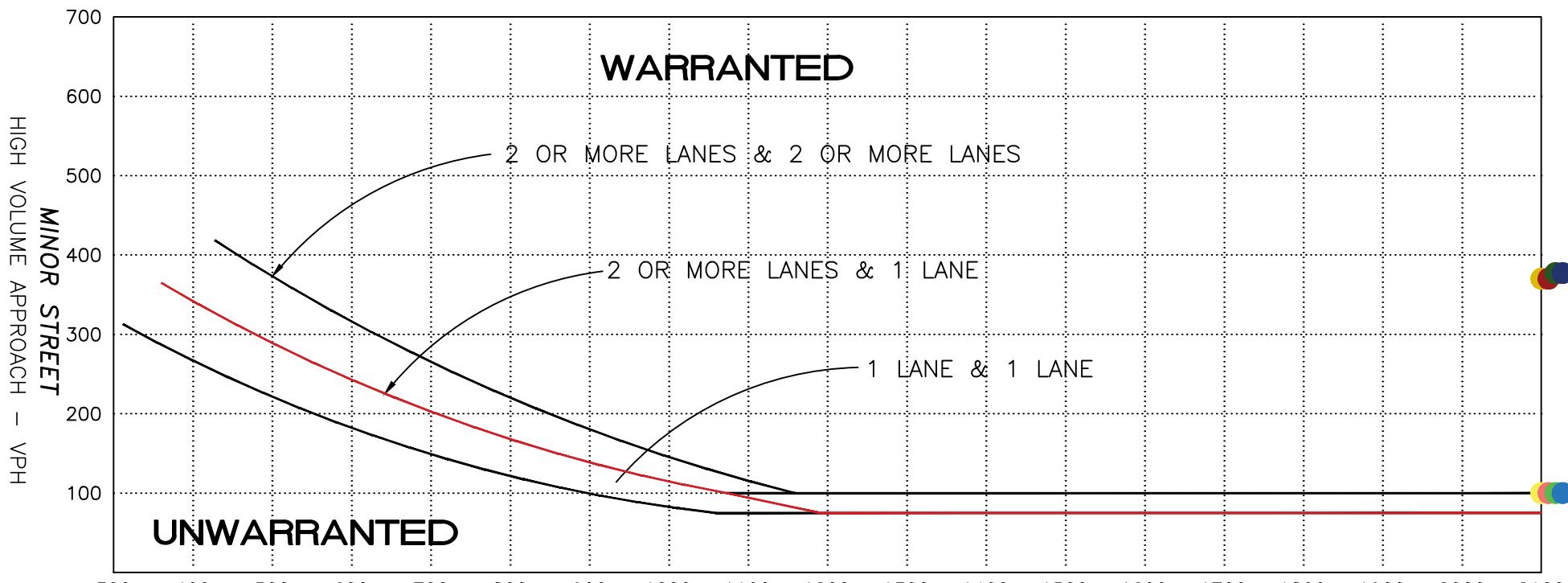
Day 1	Major		Minor*	Warrant 2 (Figure 4C-2)
	Gun Club Road	Yale Avenue		
Time of Day	Number of Lanes			
	2	2		
0:00				no
1:00				no
2:00				no
3:00				no
4:00	369	17		no
5:00	868	40		no
6:00	1,606	74		Yes
7:00	2,170	100		Yes
8:00	1,801	83		Yes
9:00	1,324	61		no
10:00			Warrant is Met (yes/no)	no
11:00				no
12:00				no
13:00				no
14:00	1,753	266		Yes
15:00	2,192	333		Yes
16:00	2,435	370		Yes
17:00	2,362	359		Yes
18:00	1,802	274		Yes
19:00	1,169	178		Yes
20:00				no
21:00				no
22:00				no
23:00				no
Total	19,851	2,155	9	Met

*70% factor applied since posted speed limit is above 40mph. The minor volume for each hour represents the higher of either minor approach.

PEAK HOUR VOLUME WARRANT (70%)
APPLIED FOR PLANNING PURPOSES

KEY FOR INTERSECTIONS

MET	AM	PM	
Yes	Yellow	Yellow	Year 2027 Background
Yes	Red	Red	Year 2050 Background
Yes	Green	Green	Year 2027 Bkgrd + Project
Yes	Blue	Blue	Year 2050 Bkgrd + Project



FOX TUTTLE
TRANSPORTATION GROUP

GUN CLUB BUSINESS PARK TRAFFIC IMPACT STUDY - AURORA, CO

PEAK HOUR SIGNAL WARRANT - GUN CLUB ROAD AT YALE AVENUE

FT Project #	23087	Original Scale	NTS	Date	11/17/2023	Drawn by	CRS	Figure #	APPENDIX 2
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Gun Club Business Park Signal Warrant Evaluation

Intersection: Gun Club Road at Addison Avenue

Warrant 1: 8 Hour Analysis (70% Factor) - 2027 Background Volumes

Day 1	Major Gun Club Rd NB/SB	Minor* Addison Ave EB	Warrant Type								
	Street Designation		Condition A		Condition B		Condition A + B				
Time of Day	Number of Lanes		Vehicles per Hour Needed to Meet Warrant	420	105	630	53	Major A	Minor A	Major B	Minor B
	2	1		420	105	630	53	336	84	504	42
0:00	112	0		no	no	no	no	no	no	no	no
1:00	95	0		no	no	no	no	no	no	no	no
2:00	122	0		no	no	no	no	no	no	no	no
3:00	199	5		no	no	no	no	no	no	no	no
4:00	374	5		no	no	no	no	yes	no	no	no
5:00	860	15		yes	no	yes	no	yes	no	yes	no
6:00	1,611	15		yes	no	yes	no	yes	no	yes	no
7:00	2,339	19		yes	no	yes	no	yes	no	yes	no
8:00	2,196	20		yes	no	yes	no	yes	no	yes	no
9:00	1,318	20		yes	no	yes	no	yes	no	yes	no
10:00	1,247	20		yes	no	yes	no	yes	no	yes	no
11:00	1,377	25		yes	no	yes	no	yes	no	yes	no
12:00	1,398	26		yes	no	yes	no	yes	no	yes	no
13:00	1,546	25		yes	no	yes	no	yes	no	yes	no
14:00	1,812	29		yes	no	yes	no	yes	no	yes	no
15:00	2,387	41		yes	no	yes	no	yes	no	yes	no
16:00	2,545	47		yes	no	yes	no	yes	no	yes	yes
17:00	2,500	49		yes	no	yes	no	yes	no	yes	yes
18:00	1,738	44		yes	no	yes	no	yes	no	yes	yes
19:00	1,143	35		yes	no	yes	no	yes	no	yes	no
20:00	1,116	20		yes	no	yes	no	yes	no	yes	no
21:00	758	15		yes	no	yes	no	yes	no	yes	no
22:00	481	10		yes	no	no	no	yes	no	no	no
23:00	305	5		no	no	no	no	no	no	no	no
Total	29,579	490		0	Not Met	0	Not Met	0	Not Met	0	Not Met

* Vehicles per hour on higher-volume minor-street approach (one direction only)

Intersection: Gun Club Road at Addison Avenue

Warrant 1: 8 Hour Analysis (70% Factor) - 2027 Background Volumes

Day 2	Major Gun Club Rd NB / SB	Minor* Addison Ave WB	Warrant Type	Condition A		Condition B		Condition A + B			
				Street Designation	Major	Minor	Major	Minor	Major A	Minor A	Major B
Time of Day	Number of Lanes		Vehicles per Hour Needed to Meet Warrant	420	105	630	53	336	84	504	42
0:00	171	0		no	no	no	no	no	no	no	no
1:00	106	0		no	no	no	no	no	no	no	no
2:00	134	0		no	no	no	no	no	no	no	no
3:00	186	5		no	no	no	no	no	no	no	no
4:00	386	5		no	no	no	no	yes	no	no	no
5:00	859	15		yes	no	yes	no	yes	no	yes	no
6:00	1,675	15		yes	no	yes	no	yes	no	yes	no
7:00	1,903	19		yes	no	yes	no	yes	no	yes	no
8:00	1,281	20		yes	no	yes	no	yes	no	yes	no
9:00	1,384	20		yes	no	yes	no	yes	no	yes	no
10:00	1,214	20		yes	no	yes	no	yes	no	yes	no
11:00	1,270	25		yes	no	yes	no	yes	no	yes	no
12:00	1,405	26		yes	no	yes	no	yes	no	yes	no
13:00	1,476	25		yes	no	yes	no	yes	no	yes	no
14:00	1,766	29		yes	no	yes	no	yes	no	yes	no
15:00	2,205	41		yes	no	yes	no	yes	no	yes	no
16:00	2,529	47		yes	no	yes	no	yes	no	yes	yes
17:00	2,506	49		yes	no	yes	no	yes	no	yes	yes
18:00	1,963	44		yes	no	yes	no	yes	no	yes	yes
19:00	1,202	35		yes	no	yes	no	yes	no	yes	no
20:00	914	20		yes	no	yes	no	yes	no	yes	no
21:00	612	15		yes	no	no	no	yes	no	yes	no
22:00	426	10		yes	no	no	no	yes	no	no	no
23:00	256	5		no	no	no	no	no	no	no	no
Total	27,829	490		0	Not Met	0	Not Met	0	Not Met	0	Not Met

* Vehicles per hour on higher-volume minor-street approach (one direction only)

Intersection: Gun Club Road at Addison Avenue

Warrant 1: 8 Hour Analysis (70% Factor) - 2027 Background Volumes

Day 3	Major Gun Club Rd EB / WB	Minor* Addison Ave WB
Time of Day	Number of Lanes	
	2	1
0:00	135	0
1:00	92	0
2:00	106	0
3:00	173	5
4:00	383	5
5:00	947	15
6:00	1,642	15
7:00	2,395	19
8:00	2,007	20
9:00	1,361	20
10:00	1,208	20
11:00	1,377	25
12:00	1,424	26
13:00	1,473	25
14:00	1,898	29
15:00	2,309	41
16:00	2,584	47
17:00	2,444	49
18:00	1,966	44
19:00	1,320	35
20:00	919	20
21:00	600	15
22:00	464	10
23:00	309	5
Total	29,536	490

Street Designation	Warrant Type		Condition A		Condition B		Condition A + B			
	Major	Minor	Major	Minor	Major A Minor A Major B		Minor B			
					420	105		630	53	
Vehicles per Hour Needed to Meet Warrant	420	105	630	53	336	84	504	42		
Warrant is Met (yes/no)	no	no	no	no	no	no	no	no	no	no
	no	no	no	no	no	no	no	no	no	no
	no	no	no	no	no	no	no	no	no	no
	no	no	no	no	no	yes	no	no	no	no
	yes	no	yes	no	yes	no	yes	no	yes	no
	yes	no	yes	no	yes	no	yes	no	yes	no
	yes	no	yes	no	yes	no	yes	no	yes	no
	yes	no	yes	no	yes	no	yes	no	yes	no
	yes	no	yes	no	yes	no	yes	no	yes	no
	yes	no	yes	no	yes	no	yes	no	yes	no
	yes	no	yes	no	yes	no	yes	no	yes	no
	yes	no	yes	no	yes	no	yes	no	yes	no
	yes	no	yes	no	yes	no	yes	no	yes	no
	yes	no	yes	no	yes	no	yes	no	yes	no
	yes	no	yes	no	yes	no	yes	no	yes	no
	yes	no	yes	no	yes	no	yes	no	yes	no
	yes	no	yes	no	yes	no	yes	no	yes	no
	yes	no	yes	no	yes	no	yes	no	yes	no
	yes	no	yes	no	yes	no	yes	no	yes	no
	yes	no	yes	no	yes	no	yes	no	yes	no
	yes	no	yes	no	yes	no	yes	no	yes	no
	yes	no	yes	no	yes	no	yes	no	yes	no
	yes	no	yes	no	yes	no	yes	no	yes	no
	no	no	no	no	no	no	no	no	no	no
	0	Not Met	0	Not Met	0	Not Met	0	Not Met	0	Not Met

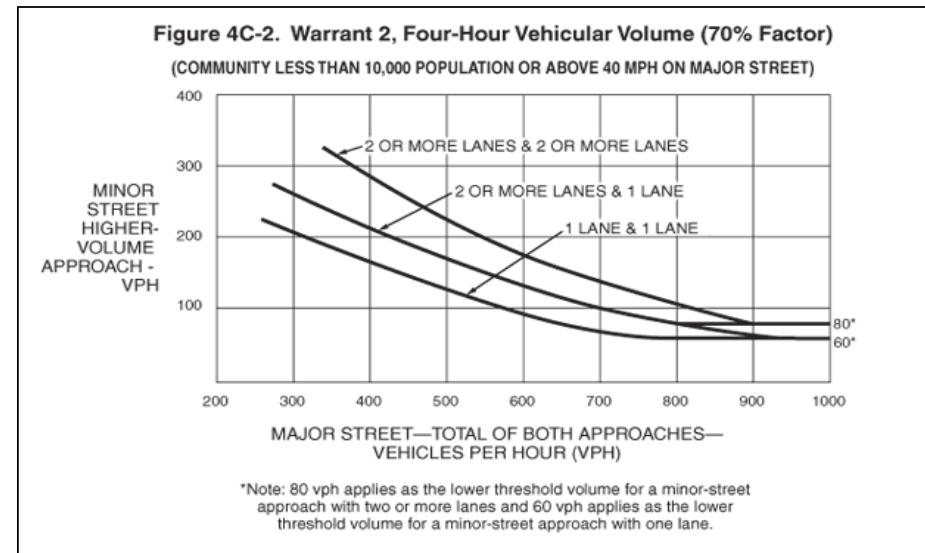
* Vehicles per hour on higher-volume minor-street approach (one direction only)

Intersection: Gun Club Rd at Addison Ave

**Warrant 2: 4 Hour Analysis (70% Factor) - 2027 Background
Volumes**

Day 1	Major Gun Club Rd NB/SB	Minor* Addison Ave EB		Warrant 2 (Figure 4C-2)
Time of Day	Number of Lanes			
	2	1		
0:00	112	0		no
1:00	95	0		no
2:00	122	0		no
3:00	199	5		no
4:00	374	5		no
5:00	860	15		no
6:00	1,611	15		no
7:00	2,339	19		no
8:00	2,196	20		no
9:00	1,318	20		no
10:00	1,247	20		no
11:00	1,377	25		no
12:00	1,398	26		no
13:00	1,546	25		no
14:00	1,812	29		no
15:00	2,387	41		no
16:00	2,545	47		no
17:00	2,500	49		no
18:00	1,738	44		no
19:00	1,143	35		no
20:00	1,116	20		no
21:00	758	15		no
22:00	481	10		no
23:00	305	5		no
Total	29,579	490	0	Not Met

*The minor volume used in this analysis comes from the minor approach with the higher total volume during the full study day.

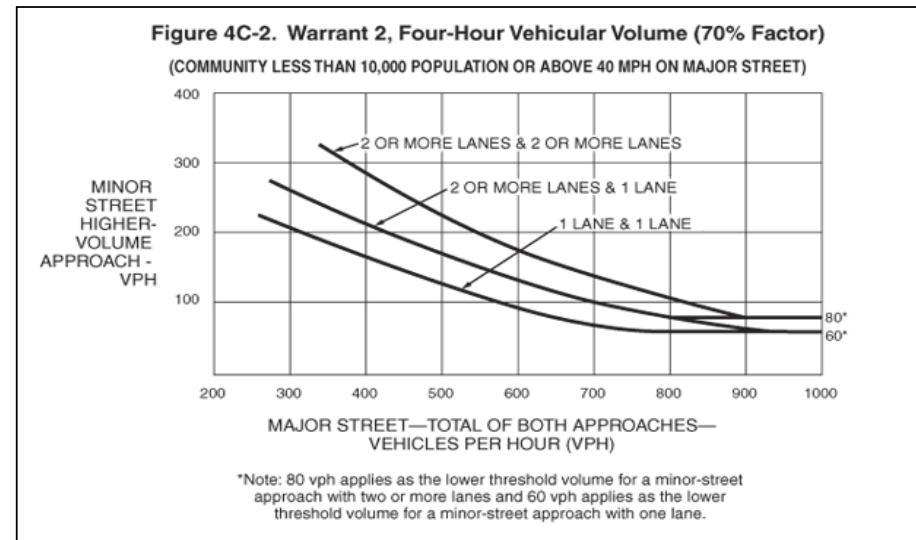


Intersection: Gun Club Rd at Addison Ave

Warrant 2: 4 Hour Analysis (70% Factor) - 2027 Background + Project Volumes

Day 2	Major Gun Club Rd NB / SB	Minor* Addison Ave WB		Warrant 2 (Figure 4C-2)
Time of Day	Number of Lanes			
	2	1		
0:00	171	0		no
1:00	106	0		no
2:00	134	0		no
3:00	186	5		no
4:00	386	5		no
5:00	859	15		no
6:00	1,675	15		no
7:00	1,903	19		no
8:00	1,281	20		no
9:00	1,384	20		no
10:00	1,214	20		no
11:00	1,270	25		no
12:00	1,405	26		no
13:00	1,476	25		no
14:00	1,766	29		no
15:00	2,205	41		no
16:00	2,529	47		no
17:00	2,506	49		no
18:00	1,963	44		no
19:00	1,202	35		no
20:00	914	20		no
21:00	612	15		no
22:00	426	10		no
23:00	256	5		no
Total	27,829	490	0	Not Met

*The minor volume used in this analysis comes from the minor approach with the higher total volume during the full study day.

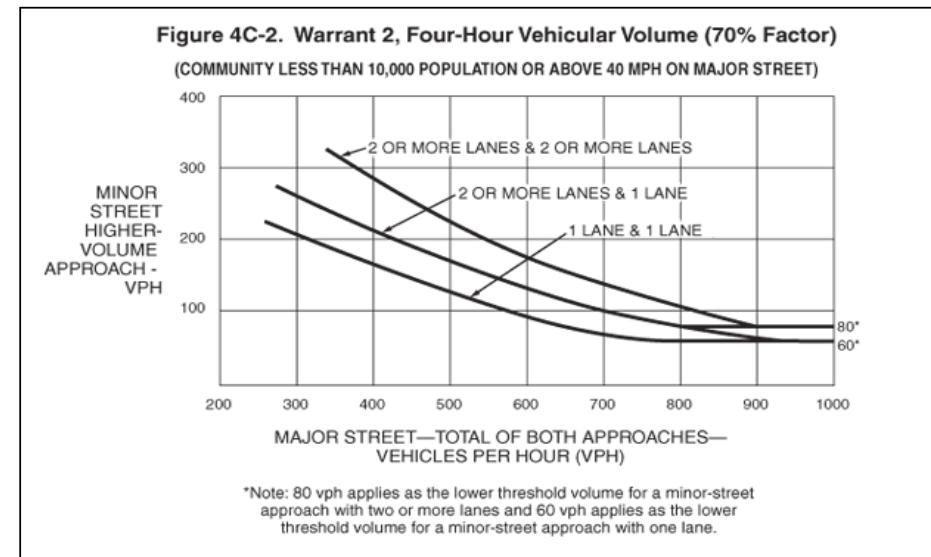


Intersection: Gun Club Rd at Addison Ave

**Warrant 2: 4 Hour Analysis (70% Factor) - 2027 Background
Volumes**

Day 3	Major Gun Club Rd NB / SB	Minor* Addison Ave WB		Warrant 2 (Figure 4C-2)
Time of Day	Number of Lanes			
	2	1		
0:00	135	0		no
1:00	92	0		no
2:00	106	0		no
3:00	173	5		no
4:00	383	5		no
5:00	947	15		no
6:00	1,642	15		no
7:00	2,395	19		no
8:00	2,007	20		no
9:00	1,361	20		no
10:00	1,208	20		no
11:00	1,377	25		no
12:00	1,424	26		no
13:00	1,473	25		no
14:00	1,898	29		no
15:00	2,309	41		no
16:00	2,584	47		no
17:00	2,444	49		no
18:00	1,966	44		no
19:00	1,320	35		no
20:00	919	20		no
21:00	600	15		no
22:00	464	10		no
23:00	309	5		no
Total	29,536	490	0	Not Met

*The minor volume used in this analysis comes from the minor approach with the higher total volume during the full study day.



Intersection: Gun Club Road at Addison Avenue

Warrant 1: 8 Hour Analysis (70% Factor) - 2027 Background + Project Volumes

Day 1	Major Gun Club Rd NB/SB	Minor* Addison Ave / Access EB	Warrant Type								
			Condition A		Condition B		Condition A + B				
Time of Day	Number of Lanes		Street Designation	Major	Minor	Major	Minor	Major A	Minor A	Major B	Minor B
	2	1									
0:00	112	0		no	no	no	no	no	no	no	no
1:00	95	0		no	no	no	no	no	no	no	no
2:00	122	0		no	no	no	no	no	no	no	no
3:00	199	0		no	no	no	no	no	no	no	no
4:00	379	5		no	no	no	no	yes	no	no	no
5:00	887	28		yes	no	yes	no	yes	no	yes	no
6:00	1,679	78		yes	no	yes	yes	yes	no	yes	yes
7:00	2,467	90		yes	no	yes	yes	yes	yes	yes	yes
8:00	2,490	92		yes	no	yes	yes	yes	yes	yes	yes
9:00	1,431	100		yes	no	yes	yes	yes	yes	yes	yes
10:00	1,340	90		yes	no	yes	yes	yes	yes	yes	yes
11:00	1,445	113		yes	yes	yes	yes	yes	yes	yes	yes
12:00	1,464	113		yes	yes	yes	yes	yes	yes	yes	yes
13:00	1,619	130		yes	yes	yes	yes	yes	yes	yes	yes
14:00	1,891	135		yes	yes	yes	yes	yes	yes	yes	yes
15:00	2,498	180		yes	yes	yes	yes	yes	yes	yes	yes
16:00	2,655	283		yes	yes	yes	yes	yes	yes	yes	yes
17:00	2,612	355		yes	yes	yes	yes	yes	yes	yes	yes
18:00	1,816	292		yes	yes	yes	yes	yes	yes	yes	yes
19:00	1,188	115		yes	yes	yes	yes	yes	yes	yes	yes
20:00	1,136	25		yes	no	yes	no	yes	no	yes	no
21:00	768	18		yes	no	yes	no	yes	no	yes	no
22:00	481	5		yes	no	no	no	yes	no	no	no
23:00	305	3		no	no	no	no	no	no	no	no
Total	31,079	2,250		9	Met	14	Met	13	Met		

* Vehicles per hour on higher-volume minor-street approach (one direction only)

Intersection: Gun Club Road at Addison Avenue

Warrant 1: 8 Hour Analysis (70% Factor) - 2027 Background + Project Volumes

Day 2	Major Gun Club Rd NB / SB	Minor* Addison Ave / Access WB	Warrant Type								
			Condition A		Condition B		Condition A + B				
Time of Day	Number of Lanes		Street Designation	Major	Minor	Major	Minor	Major A	Minor A	Major B	Minor B
	2	1									
0:00	171	0		no	no	no	no	no	no	no	no
1:00	106	0		no	no	no	no	no	no	no	no
2:00	134	0		no	no	no	no	no	no	no	no
3:00	186	0		no	no	no	no	no	no	no	no
4:00	391	5		no	no	no	no	yes	no	no	no
5:00	886	28		yes	no	yes	no	yes	no	yes	no
6:00	1,743	78		yes	no	yes	yes	yes	no	yes	yes
7:00	2,031	90		yes	no	yes	yes	yes	yes	yes	yes
8:00	1,575	92		yes	no	yes	yes	yes	yes	yes	yes
9:00	1,497	100		yes	no	yes	yes	yes	yes	yes	yes
10:00	1,307	90		yes	no	yes	yes	yes	yes	yes	yes
11:00	1,338	113		yes	yes	yes	yes	yes	yes	yes	yes
12:00	1,471	113		yes	yes	yes	yes	yes	yes	yes	yes
13:00	1,549	130		yes	yes	yes	yes	yes	yes	yes	yes
14:00	1,845	135		yes	yes	yes	yes	yes	yes	yes	yes
15:00	2,316	180		yes	yes	yes	yes	yes	yes	yes	yes
16:00	2,639	283		yes	yes	yes	yes	yes	yes	yes	yes
17:00	2,618	355		yes	yes	yes	yes	yes	yes	yes	yes
18:00	2,041	292		yes	yes	yes	yes	yes	yes	yes	yes
19:00	1,247	115		yes	yes	yes	yes	yes	yes	yes	yes
20:00	934	25		yes	no	yes	no	yes	no	yes	no
21:00	622	18		yes	no	no	no	yes	no	yes	no
22:00	426	5		yes	no	no	no	yes	no	no	no
23:00	256	3		no	no	no	no	no	no	no	no
Total	29,329	2,250		9	Met	14	Met	13	Met		

* Vehicles per hour on higher-volume minor-street approach (one direction only)

Intersection: Gun Club Road at Addison Avenue

Warrant 1: 8 Hour Analysis (70% Factor) - 2027 Background + Project Volumes

Day 3	Major Gun Club Rd NB / SB	Minor* Addison Ave / Access WB	Warrant Type	Condition A		Condition B		Condition A + B			
				Street Designation	Major	Minor	Major	Minor	Major A	Minor A	Major B
Time of Day	Number of Lanes		Vehicles per Hour Needed to Meet Warrant	420	105	630	53	336	84	504	42
0:00	135	0		no	no	no	no	no	no	no	no
1:00	92	0		no	no	no	no	no	no	no	no
2:00	106	0		no	no	no	no	no	no	no	no
3:00	173	0		no	no	no	no	no	no	no	no
4:00	388	5		no	no	no	no	yes	no	no	no
5:00	974	28		yes	no	yes	no	yes	no	yes	no
6:00	1,710	78		yes	no	yes	yes	yes	no	yes	yes
7:00	2,523	90		yes	no	yes	yes	yes	yes	yes	yes
8:00	2,301	92		yes	no	yes	yes	yes	yes	yes	yes
9:00	1,474	100		yes	no	yes	yes	yes	yes	yes	yes
10:00	1,301	90		yes	no	yes	yes	yes	yes	yes	yes
11:00	1,445	113		yes	yes	yes	yes	yes	yes	yes	yes
12:00	1,490	113		yes	yes	yes	yes	yes	yes	yes	yes
13:00	1,546	130		yes	yes	yes	yes	yes	yes	yes	yes
14:00	1,977	135		yes	yes	yes	yes	yes	yes	yes	yes
15:00	2,420	180		yes	yes	yes	yes	yes	yes	yes	yes
16:00	2,694	283		yes	yes	yes	yes	yes	yes	yes	yes
17:00	2,556	355		yes	yes	yes	yes	yes	yes	yes	yes
18:00	2,044	292		yes	yes	yes	yes	yes	yes	yes	yes
19:00	1,365	115		yes	yes	yes	yes	yes	yes	yes	yes
20:00	939	25		yes	no	yes	no	yes	no	yes	no
21:00	610	18		yes	no	no	no	yes	no	yes	no
22:00	464	5		yes	no	no	no	yes	no	no	no
23:00	309	3		no	no	no	no	no	no	no	no
Total	31,036	2,250		9	Met	14	Met	13	Met		

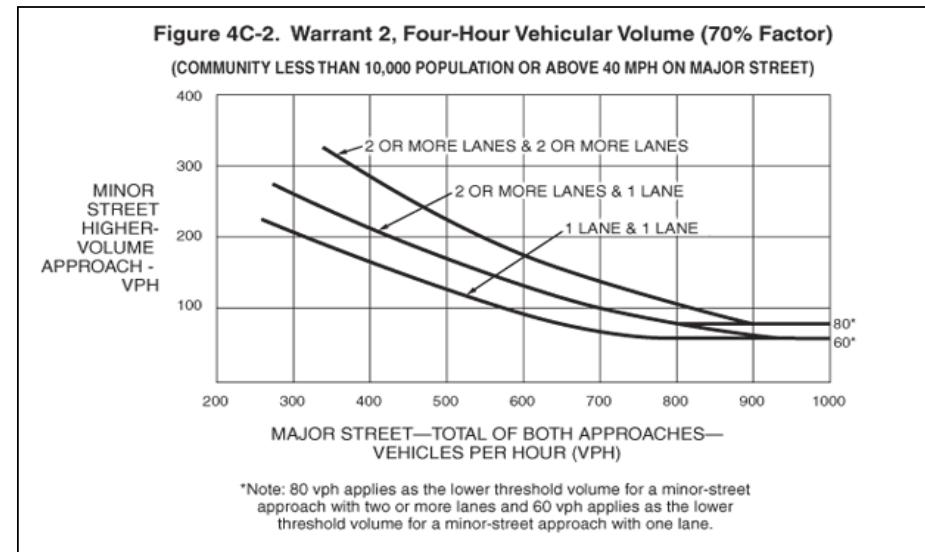
* Vehicles per hour on higher-volume minor-street approach (one direction only)

Intersection: Gun Club Rd at Addison Ave / Access

Warrant 2: 4 Hour Analysis (70% Factor) - 2027 Background + Project Volumes

Day 1	Major Gun Club Rd NB/SB	Minor* Addison Ave / Access EB		Warrant 2 (Figure 4C-2)
Time of Day	Number of Lanes			
	2	1		
0:00	112	0		no
1:00	95	0		no
2:00	122	0		no
3:00	199	0		no
4:00	379	5		no
5:00	887	28		no
6:00	1,679	78		Yes
7:00	2,467	90		Yes
8:00	2,490	92		Yes
9:00	1,431	100		Yes
10:00	1,340	90		Yes
11:00	1,445	113		Yes
12:00	1,464	113		Yes
13:00	1,619	130		Yes
14:00	1,891	135		Yes
15:00	2,498	180		Yes
16:00	2,655	283		Yes
17:00	2,612	355		Yes
18:00	1,816	292		Yes
19:00	1,188	115		Yes
20:00	1,136	25		no
21:00	768	18		no
22:00	481	5		no
23:00	305	3		no
Total	31,079	2,250	14	Met

*The minor volume used in this analysis comes from the minor approach with the higher total volume during the full study day.

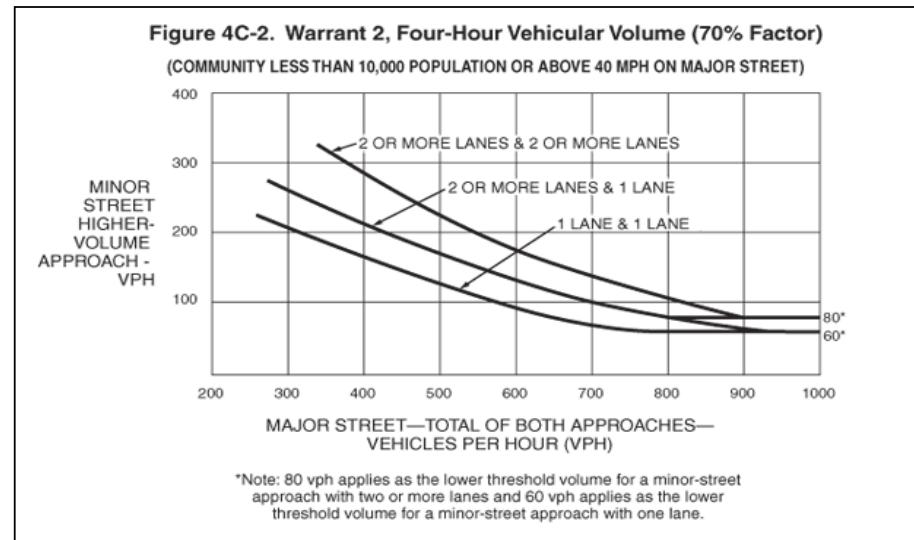


Intersection: Gun Club Rd at Addison Ave / Access

Warrant 2: 4 Hour Analysis (70% Factor) - 2027 Background + Project Volumes

Day 2	Major Gun Club Rd NB / SB	Minor* Addison Ave / Access WB		Warrant 2 (Figure 4C-2)
Time of Day	Number of Lanes			
	2	1		
0:00	171	0		no
1:00	106	0		no
2:00	134	0		no
3:00	186	0		no
4:00	391	5		no
5:00	886	28		no
6:00	1,743	78		Yes
7:00	2,031	90		Yes
8:00	1,575	92		Yes
9:00	1,497	100		Yes
10:00	1,307	90		Yes
11:00	1,338	113		Yes
12:00	1,471	113		Yes
13:00	1,549	130		Yes
14:00	1,845	135		Yes
15:00	2,316	180		Yes
16:00	2,639	283		Yes
17:00	2,618	355		Yes
18:00	2,041	292		Yes
19:00	1,247	115		Yes
20:00	934	25		no
21:00	622	18		no
22:00	426	5		no
23:00	256	3		no
Total	29,329	2,250	14	Met

Warrant
is Met
(yes/no)



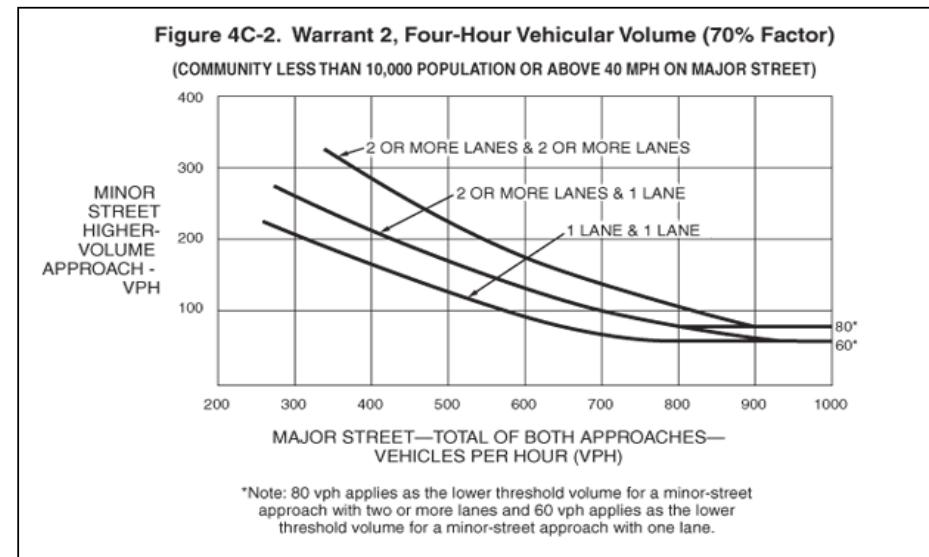
*The minor volume used in this analysis comes from the minor approach with the higher total volume during the full study day.

Intersection: Gun Club Rd at Addison Ave / Access

Warrant 2: 4 Hour Analysis (70% Factor) - 2027 Background + Project Volumes

Day 3	Major Gun Club Rd NB / SB	Minor* Addison Ave / Access WB		Warrant 2 (Figure 4C-2)
Time of Day	Number of Lanes			
	2	1		
0:00	135	0		no
1:00	92	0		no
2:00	106	0		no
3:00	173	0		no
4:00	388	5		no
5:00	974	28		no
6:00	1,710	78		Yes
7:00	2,523	90		Yes
8:00	2,301	92		Yes
9:00	1,474	100		Yes
10:00	1,301	90		Yes
11:00	1,445	113		Yes
12:00	1,490	113		Yes
13:00	1,546	130		Yes
14:00	1,977	135		Yes
15:00	2,420	180		Yes
16:00	2,694	283		Yes
17:00	2,556	355		Yes
18:00	2,044	292		Yes
19:00	1,365	115		Yes
20:00	939	25		no
21:00	610	18		no
22:00	464	5		no
23:00	309	3		no
Total	31,036	2,250	14	Met

*The minor volume used in this analysis comes from the minor approach with the higher total volume during the full study day.



Gun Club Business Park Signal Warrant EvaluationIntersection: **Gun Club Road at Addison Avenue****Warrant 1: 8 Hour Analysis (70% Factor) - 2050 Background Volumes**

Day 1	Major	Minor*					Condition A + B				
	Gun Club Rd	Addison Ave	Street Designation	Major	Minor	Major	Minor	Major A	Minor A	Major B	Minor B
Time of Day	Number of Lanes		Vehicles per Hour Needed to Meet Warrant	420	105	630	53	336	84	504	42
0:00	180	0		no	no	no	no	no	no	no	no
1:00	153	0		no	no	no	no	no	no	no	no
2:00	197	0		no	no	no	no	no	no	no	no
3:00	321	19		no	no	no	no	no	no	no	no
4:00	604	19		yes	no	no	no	yes	no	yes	no
5:00	1,388	43		yes	no	yes	no	yes	no	yes	yes
6:00	2,600	67		yes	no	yes	yes	yes	no	yes	yes
7:00	3,777	70		yes	no	yes	yes	yes	no	yes	yes
8:00	3,544	75		yes	no	yes	yes	yes	no	yes	yes
9:00	2,129	75		yes	no	yes	yes	yes	no	yes	yes
10:00	2,013	70		yes	no	yes	yes	yes	no	yes	yes
11:00	2,223	90		yes	no	yes	yes	yes	yes	yes	yes
12:00	2,256	100		yes	no	yes	yes	yes	yes	yes	yes
13:00	2,495	100		yes	no	yes	yes	yes	yes	yes	yes
14:00	2,926	130		yes	yes	yes	yes	yes	yes	yes	yes
15:00	3,855	165		yes	yes	yes	yes	yes	yes	yes	yes
16:00	4,108	185		yes	yes	yes	yes	yes	yes	yes	yes
17:00	4,035	194		yes	yes	yes	yes	yes	yes	yes	yes
18:00	2,806	181		yes	yes	yes	yes	yes	yes	yes	yes
19:00	1,845	123		yes	yes	yes	yes	yes	yes	yes	yes
20:00	1,801	98		yes	no	yes	yes	yes	yes	yes	yes
21:00	1,223	78		yes	no	yes	yes	yes	no	yes	yes
22:00	775	39		yes	no	yes	no	yes	no	yes	no
23:00	492	19		yes	no	no	no	yes	no	no	no
Total	47,746	1,940		6	Not Met	16	Met	10	Met		

* Vehicles per hour on higher-volume minor-street approach (one direction only)

Gun Club Business Park Signal Warrant Evaluation

Intersection: Gun Club Road at Addison Avenue

Warrant 1: 8 Hour Analysis (70% Factor) - 2050 Background Volumes

Day 2	Major Gun Club Rd NB / SB	Minor* Addison Ave WB
Time of Day	Number of Lanes	
	3	1
0:00	277	0
1:00	170	0
2:00	216	0
3:00	299	19
4:00	623	19
5:00	1,386	43
6:00	2,703	67
7:00	3,073	70
8:00	2,068	75
9:00	2,233	75
10:00	1,960	70
11:00	2,051	90
12:00	2,269	100
13:00	2,382	100
14:00	2,852	130
15:00	3,560	165
16:00	4,082	185
17:00	4,045	194
18:00	3,169	181
19:00	1,942	123
20:00	1,476	98
21:00	988	78
22:00	688	39
23:00	413	19
Total	44,925	1,940

Warrant Type Street Designation	Condition A		Condition B		Condition A + B			
	Major	Minor	Major	Minor	Major A	Minor A	Major B	Minor B
Vehicles per Hour Needed to Meet Warrant	420	105	630	53	336	84	504	42
Warrant is Met (yes/no)	no	no	no	no	no	no	no	no
	no	no	no	no	no	no	no	no
	no	no	no	no	no	no	no	no
	no	no	no	no	no	no	no	no
	yes	no	no	no	yes	no	yes	no
	yes	no	yes	no	yes	no	yes	yes
	yes	no	yes	yes	yes	no	yes	yes
	yes	no	yes	yes	yes	no	yes	yes
	yes	no	yes	yes	yes	no	yes	yes
	yes	no	yes	yes	yes	no	yes	yes
	yes	no	yes	yes	yes	no	yes	yes
	yes	no	yes	yes	yes	no	yes	yes
	yes	no	yes	yes	yes	no	yes	yes
	yes	no	yes	yes	yes	no	yes	yes
	yes	no	yes	yes	yes	no	yes	yes
	yes	no	yes	yes	yes	no	yes	yes
	yes	no	yes	yes	yes	no	yes	yes
	yes	no	yes	yes	yes	no	yes	yes
	yes	no	yes	yes	yes	no	yes	yes
	yes	no	yes	yes	yes	no	yes	yes
	no	no	no	no	yes	no	no	no
	6	Not Met	16	Met	10	Met		

* Vehicles per hour on higher-volume minor-street approach (one direction only)

Intersection: Gun Club Road at Addison Avenue

Warrant 1: 8 Hour Analysis (70% Factor) - 2050 Background Volumes

Day 3	Major Gun Club Rd NB / SB	Minor* Addison Ave WB	Warrant Type Street Designation	Condition A		Condition B		Condition A + B			
				Major	Minor	Major	Minor	Major A	Minor A	Major B	Minor B
Time of Day	Number of Lanes		Vehicles per Hour Needed to Meet Warrant	420	105	630	53	336	84	504	42
0:00	218	0	no	no	no	no	no	no	no	no	no
1:00	148	0	no	no	no	no	no	no	no	no	no
2:00	172	0	no	no	no	no	no	no	no	no	no
3:00	279	19	no	no	no	no	no	no	no	no	no
4:00	618	19	yes	no	no	no	no	yes	no	yes	no
5:00	1,529	43	yes	no	yes	no	yes	yes	no	yes	yes
6:00	2,651	67	yes	no	yes	yes	yes	yes	no	yes	yes
7:00	3,866	70	yes	no	yes	yes	yes	yes	no	yes	yes
8:00	3,239	75	yes	no	yes	yes	yes	yes	no	yes	yes
9:00	2,196	75	yes	no	yes	yes	yes	yes	no	yes	yes
10:00	1,950	70	yes	no	yes	yes	yes	yes	no	yes	yes
11:00	2,223	90	yes	no	yes	yes	yes	yes	yes	yes	yes
12:00	2,298	100	yes	no	yes	yes	yes	yes	yes	yes	yes
13:00	2,378	100	yes	no	yes	yes	yes	yes	yes	yes	yes
14:00	3,064	130	yes	yes	yes	yes	yes	yes	yes	yes	yes
15:00	3,727	165	yes	yes	yes	yes	yes	yes	yes	yes	yes
16:00	4,171	185	yes	yes	yes	yes	yes	yes	yes	yes	yes
17:00	3,945	194	yes	yes	yes	yes	yes	yes	yes	yes	yes
18:00	3,175	181	yes	yes	yes	yes	yes	yes	yes	yes	yes
19:00	2,131	123	yes	yes	yes	yes	yes	yes	yes	yes	yes
20:00	1,482	98	yes	no	yes	yes	yes	yes	yes	yes	yes
21:00	969	78	yes	no	yes	yes	yes	yes	no	yes	yes
22:00	749	39	yes	no	yes	no	yes	yes	no	yes	no
23:00	499	19	yes	no	no	no	no	yes	no	no	no
Total	47,677	1,940		6	Not Met	16	Met		10	Met	

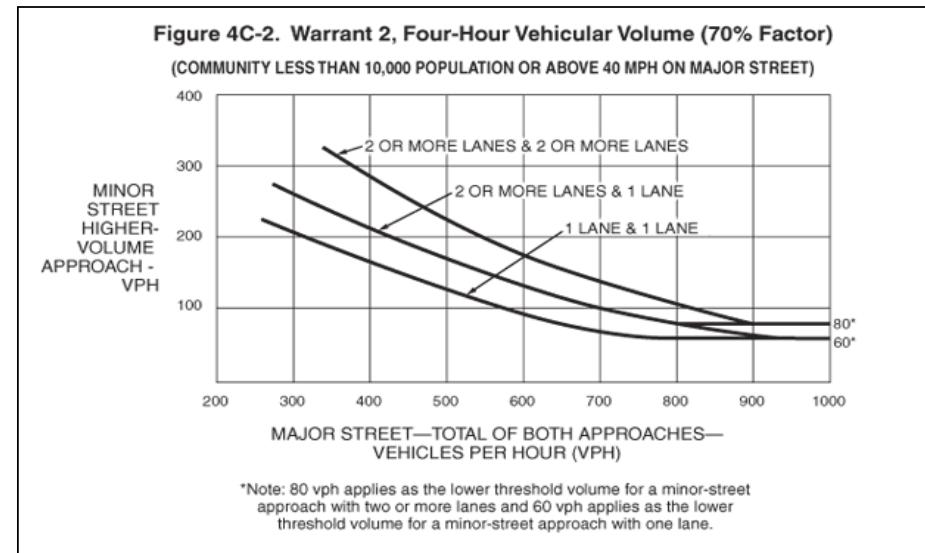
* Vehicles per hour on higher-volume minor-street approach (one direction only)

Intersection: Gun Club Rd at Addison Ave

Warrant 2: 4 Hour Analysis (70% Factor) - 2050 Background Volumes

Day 1	Major Gun Club Rd NB/SB	Minor* Addison Ave WB		Warrant 2 (Figure 4C-2)
Time of Day	Number of Lanes			
	3	1		
0:00	180	0		no
1:00	153	0		no
2:00	197	0		no
3:00	321	19		no
4:00	604	19		no
5:00	1,388	43		no
6:00	2,600	67		Yes
7:00	3,777	70		Yes
8:00	3,544	75		Yes
9:00	2,129	75		Yes
10:00	2,013	70		Yes
11:00	2,223	90		Yes
12:00	2,256	100		Yes
13:00	2,495	100		Yes
14:00	2,926	130		Yes
15:00	3,855	165		Yes
16:00	4,108	185		Yes
17:00	4,035	194		Yes
18:00	2,806	181		Yes
19:00	1,845	123		Yes
20:00	1,801	98		Yes
21:00	1,223	78		Yes
22:00	775	39		no
23:00	492	19		no
Total	47,746	1,940	16	Met

*The minor volume used in this analysis comes from the minor approach with the higher total volume during the full study day.

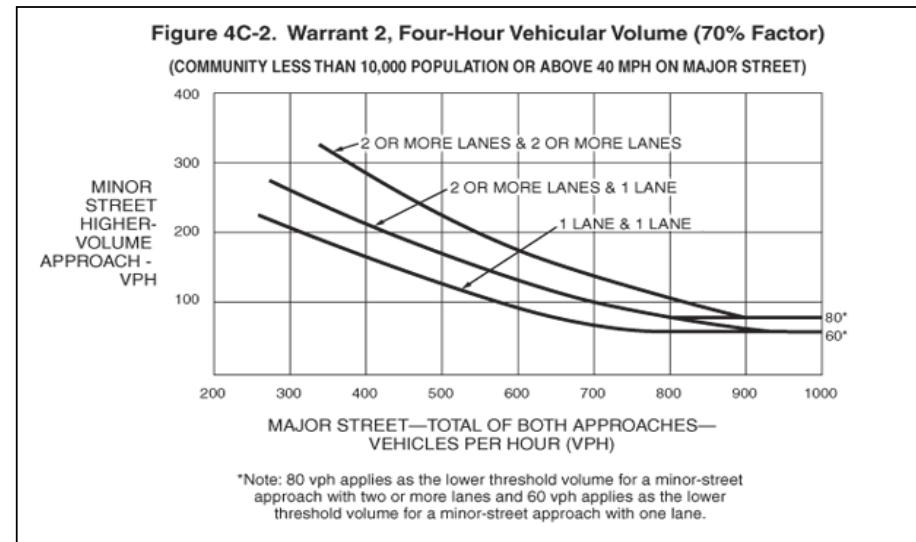


Intersection: Gun Club Rd at Addison Ave

Warrant 2: 4 Hour Analysis (70% Factor) - 2050 Background Volumes

Day 2	Major Gun Club Rd NB / SB	Minor* Addison Ave WB		Warrant 2 (Figure 4C-2)
Time of Day	Number of Lanes			
	3	1		
0:00	277	0		no
1:00	170	0		no
2:00	216	0		no
3:00	299	19		no
4:00	623	19		no
5:00	1,386	43		no
6:00	2,703	67		Yes
7:00	3,073	70		Yes
8:00	2,068	75		Yes
9:00	2,233	75		Yes
10:00	1,960	70		Yes
11:00	2,051	90		Yes
12:00	2,269	100		Yes
13:00	2,382	100		Yes
14:00	2,852	130		Yes
15:00	3,560	165		Yes
16:00	4,082	185		Yes
17:00	4,045	194		Yes
18:00	3,169	181		Yes
19:00	1,942	123		Yes
20:00	1,476	98		Yes
21:00	988	78		Yes
22:00	688	39		no
23:00	413	19		no
Total	44,925	1,940	16	Met

*The minor volume used in this analysis comes from the minor approach with the higher total volume during the full study day.

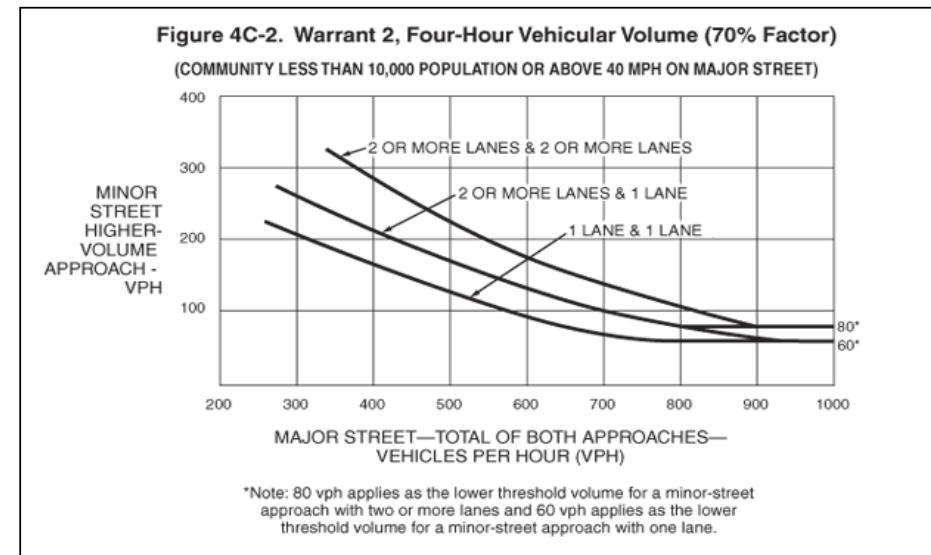


Intersection: Gun Club Rd at Addison Ave

**Warrant 2: 4 Hour Analysis (70% Factor) - 2050 Background
Volumes**

Day 3	Major Gun Club Rd NB / SB	Minor* Addison Ave WB		Warrant 2 (Figure 4C-2)
Time of Day	Number of Lanes			
	3	1		
0:00	218	0		no
1:00	148	0		no
2:00	172	0		no
3:00	279	19		no
4:00	618	19		no
5:00	1,529	43		no
6:00	2,651	67		Yes
7:00	3,866	70		Yes
8:00	3,239	75		Yes
9:00	2,196	75		Yes
10:00	1,950	70		Yes
11:00	2,223	90		Yes
12:00	2,298	100		Yes
13:00	2,378	100		Yes
14:00	3,064	130		Yes
15:00	3,727	165		Yes
16:00	4,171	185		Yes
17:00	3,945	194		Yes
18:00	3,175	181		Yes
19:00	2,131	123		Yes
20:00	1,482	98		Yes
21:00	969	78		Yes
22:00	749	39		no
23:00	499	19		no
Total	47,677	1,940	16	Met

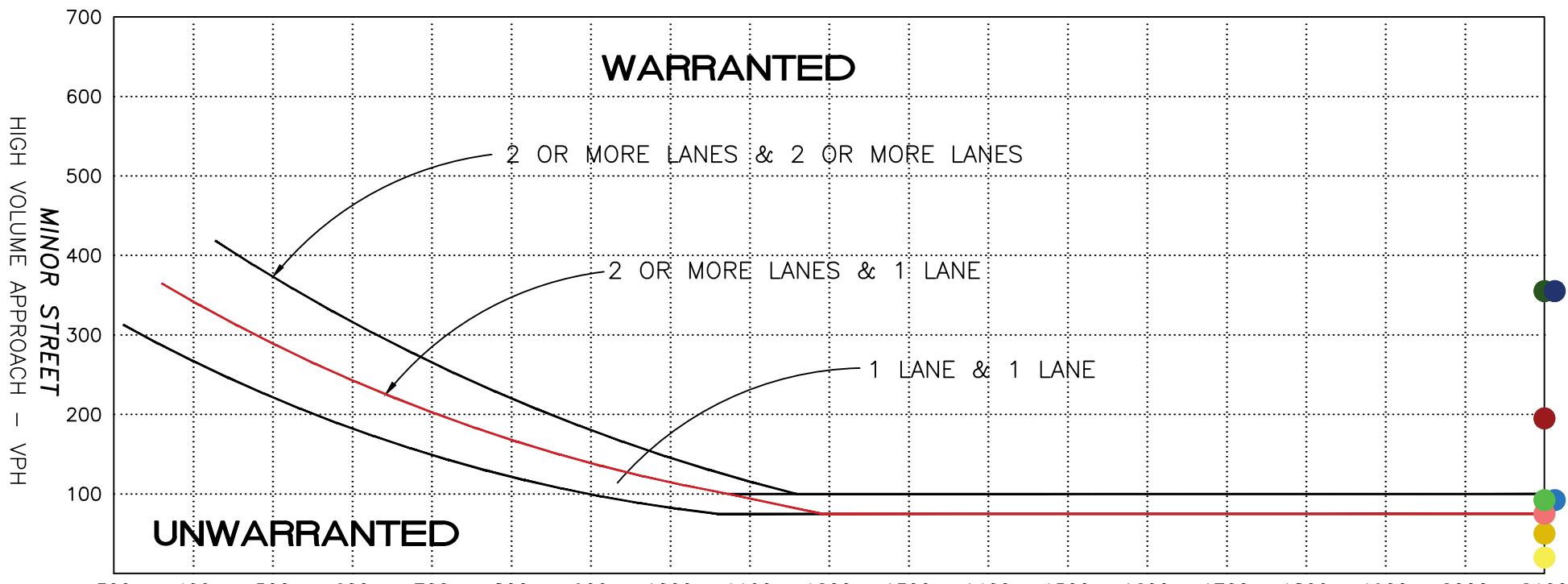
*The minor volume used in this analysis comes from the minor approach with the higher total volume during the full study day.



PEAK HOUR VOLUME WARRANT (70%)
APPLIED FOR PLANNING PURPOSES

KEY FOR INTERSECTIONS

MET	AM	PM	
No	Yellow	Yellow	Year 2027 Background
Yes	Red	Maroon	Year 2050 Background
Yes	Green	Dark Green	Year 2027 Bkgrd + Project
Yes	Blue	Dark Blue	Year 2050 Bkgrd + Project



NOTE: 100 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACH WITH TWO OR MORE LANES
AND 75 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACHING WITH ONE LANE.

***Internal Capture
and Pass-By Calculations***



NCHRP 684 Internal Trip Capture Estimation Tool					
Project Name:	Gun Club Business Park Development		Organization:	Fox Tuttle Transportation Group, LLC	
Project Location:	Aurora, CO		Performed By:	C. Slade	
Scenario Description:	Site Trip Generation		Date:	9-Nov-23	
Analysis Year:			Checked By:		
Analysis Period:	AM Peak Hour		Date:		

Table 1-A: Base Vehicle-Trip Generation Estimates (Single-Use Site Estimate)						
Land Use	Development Data (For Information Only)			Estimated Vehicle-Trips ³		
	ITE LUCs ¹	Quantity	Units	Total	Entering	Exiting
Office				0		
Retail	822	13	ksf	29	17	12
Restaurant	930	7	ksf	9	5	4
Cinema/Entertainment				0		
Residential				0		
Hotel				0		
All Other Land Uses ²	130	1,466	ksf	474	384	90
				512	406	106

Table 2-A: Mode Split and Vehicle Occupancy Estimates						
Land Use	Entering Trips			Exiting Trips		
	Veh. Occ. ⁴	% Transit	% Non-Motorized	Veh. Occ. ⁴	% Transit	% Non-Motorized
Office			5%			5%
Retail			5%			5%
Restaurant						
Cinema/Entertainment						
Residential						
Hotel						
All Other Land Uses ²			5%			5%

Table 3-A: Average Land Use Interchange Distances (Feet Walking Distance)						
Origin (From)	Destination (To)					
	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel
Office						
Retail						
Restaurant						
Cinema/Entertainment						
Residential						
Hotel						

Table 4-A: Internal Person-Trip Origin-Destination Matrix*						
Origin (From)	Destination (To)					
	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel
Office		0	0	0	0	0
Retail	0		2	0	0	0
Restaurant	0	1		0	0	0
Cinema/Entertainment	0	0	0		0	0
Residential	0	0	0	0		0
Hotel	0	0	0	0	0	

Table 5-A: Computations Summary				Table 6-A: Internal Trip Capture Percentages by Land Use		
	Total	Entering	Exiting	Land Use	Entering Trips	Exiting Trips
All Person-Trips	512	406	106	Office	N/A	N/A
Internal Capture Percentage	1%	1%	3%	Retail	6%	17%
External Vehicle-Trips ⁵	480	383	97	Restaurant	40%	25%
External Transit-Trips ⁶	0	0	0	Cinema/Entertainment	N/A	N/A
External Non-Motorized Trips ⁶	26	20	6	Residential	N/A	N/A
				Hotel	N/A	N/A

¹Land Use Codes (LUCs) from *Trip Generation Manual*, published by the Institute of Transportation Engineers.

²Total estimate for all other land uses at mixed-use development site is not subject to internal trip capture computations in this estimator.

³Enter trips assuming no transit or non-motorized trips (as assumed in ITE *Trip Generation Manual*).

⁴Enter vehicle occupancy assumed in Table 1-A vehicle trips. If vehicle occupancy changes for proposed mixed-use project, manual adjustments must be made to Tables 5-A, 9-A (O and D). Enter transit, non-motorized percentages that will result with proposed mixed-use project complete.

⁵Vehicle-trips computed using the mode split and vehicle occupancy values provided in Table 2-A.

⁶Person-Trips

*Indicates computation that has been rounded to the nearest whole number.

NCHRP 684 Internal Trip Capture Estimation Tool					
Project Name:	Gun Club Business Park Development		Organization:	Fox Tuttle Transportation Group, LLC	
Project Location:	Aurora, CO		Performed By:	C. Slade	
Scenario Description:	Site Trip Generation		Date:	9-Nov-23	
Analysis Year:			Checked By:		
Analysis Period:	PM Peak Hour		Date:		

Table 1-P: Base Vehicle-Trip Generation Estimates (Single-Use Site Estimate)

Land Use	Development Data (For Information Only)			Estimated Vehicle-Trips ³		
	ITE LUCs ¹	Quantity	Units	Total	Entering	Exiting
Office				0		
Retail	822	13	ksf	81	41	40
Restaurant	930	7	ksf	77	42	35
Cinema/Entertainment				0		
Residential				0		
Hotel				0		
All Other Land Uses ²	130	1,466	ksf	474	104	370
				632	187	445

Table 2-P: Mode Split and Vehicle Occupancy Estimates

Land Use	Entering Trips			Exiting Trips		
	Veh. Occ. ⁴	% Transit	% Non-Motorized	Veh. Occ. ⁴	% Transit	% Non-Motorized
Office			5%			5%
Retail			5%			5%
Restaurant						
Cinema/Entertainment						
Residential						
Hotel						
All Other Land Uses ²			5%			5%

Table 3-P: Average Land Use Interchange Distances (Feet Walking Distance)

Origin (From)	Destination (To)					
	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel
Office						
Retail						
Restaurant						
Cinema/Entertainment						
Residential						
Hotel						

Table 4-P: Internal Person-Trip Origin-Destination Matrix*

Origin (From)	Destination (To)					
	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel
Office		0	0	0	0	0
Retail	0		12	0	0	0
Restaurant	0	14		0	0	0
Cinema/Entertainment	0	0	0		0	0
Residential	0	0	0	0		0
Hotel	0	0	0	0	0	

Table 5-P: Computations Summary

	Total	Entering	Exiting
All Person-Trips	632	187	445
Internal Capture Percentage	8%	14%	6%
External Vehicle-Trips ⁵	554	155	399
External Transit-Trips ⁶	0	0	0
External Non-Motorized Trips ⁶	26	6	20

Table 6-P: Internal Trip Capture Percentages by Land Use

Land Use	Entering Trips	Exiting Trips
Office	N/A	N/A
Retail	34%	30%
Restaurant	29%	40%
Cinema/Entertainment	N/A	N/A
Residential	N/A	N/A
Hotel	N/A	N/A

¹Land Use Codes (LUCs) from *Trip Generation Manual*, published by the Institute of Transportation Engineers.

²Total estimate for all other land uses at mixed-use development site is not subject to internal trip capture computations in this estimator.

³Enter trips assuming no transit or non-motorized trips (as assumed in ITE *Trip Generation Manual*).

⁴Enter vehicle occupancy assumed in Table 1-P vehicle trips. If vehicle occupancy changes for proposed mixed-use project, manual adjustments must be

⁵Vehicle-trips computed using the mode split and vehicle occupancy values provided in Table 2-P.

⁶Person-Trips

*Indicates computation that has been rounded to the nearest whole number.

Vehicle Pass-By Rates by Land Use

Source: ITE *Trip Generation Manual*, 11th Edition

Land Use Code	821								
Land Use	Shopping Plaza (40 - 150k)								
Setting	General Urban/Suburban								
Time Period	Weekday PM Peak Period								
# Data Sites	15								
Average Pass-By Rate	40%								
	Pass-By Characteristics for Individual Sites								
GLA (000)	State or Province	Survey Year	# Interviews	Pass-By Trip (%)	Non-Pass-By Trips			Adj Street Peak Hour Volume	Source
					Primary (%)	Diverted (%)	Total (%)		
45	Florida	1992	844	56	24	20	44	—	30
50	Florida	1992	555	41	41	18	59	—	30
52	Florida	1995	665	42	33	25	58	—	30
53	Florida	1993	162	59	—	—	41	—	30
57.23	Kentucky	1993	247	31	53	16	69	2659	34
60	Florida	1995	1583	40	38	22	60	—	30
69.4	Kentucky	1993	109	25	42	33	75	1559	34
77	Florida	1992	365	46	—	—	54	—	30
78	Florida	1991	702	55	23	22	45	—	30
82	Florida	1992	336	34	—	—	66	—	30
92.857	Kentucky	1993	133	22	50	28	78	3555	34
100.888	Kentucky	1993	281	28	50	22	72	2111	34
121.54	Kentucky	1993	210	53	30	17	47	2636	34
144	New Jersey	1990	176	32	44	24	68	—	24
146.8	Kentucky	1993	—	36	39	25	64	—	34

Vehicle Pass-By Rates by Land Use

Source: ITE *Trip Generation Manual*, 11th Edition

Land Use Code	932								
Land Use	High-Turnover (Sit-Down) Restaurant								
Setting	General Urban/Suburban								
Time Period	Weekday PM Peak Period								
# Data Sites	12								
Average Pass-By Rate	43%								
	Pass-By Characteristics for Individual Sites								
GFA (000)	State or Province	Survey Year	# Interviews	Pass-By Trip (%)	Non-Pass-By Trips			Adj Street Peak Hour Volume	Source
Primary (%)					Diverted (%)	Total (%)			
2.9	Kentucky	1993	41	37	27	36	63	3935	2
3.1	Kentucky	1993	21	38	29	33	62	2580	2
4.6	Florida	1992	276	63	—	—	37	—	30
5	Florida	1992	65	58	—	—	42	—	30
5.3	Kentucky	1993	24	50	37	13	50	1615	2
5.7	Florida	1994	308	57	—	—	43	—	30
5.8	Florida	1992	150	32	—	—	68	—	30
6.2	Florida	1995	521	46	43	11	54	—	30
7.1	Indiana	1993	—	23	23	54	77	1565	2
8	Florida	1995	664	40	39	21	60	—	30
11	Florida	1996	267	38	43	19	62	—	30
12	Florida	1996	317	29	51	20	71	—	30

Sensitivity Analysis

Table A1 - Peak Hour Intersection Level of Service and 95th Percentile Queue Summary - Sensitivity Analysis

Intersection and Lanes Groups	2027 Bkgrd + Project [Sensitivity]						2050 Bkgrd + Project [Sensitivity]					
	AM Peak			PM Peak			AM Peak			PM Peak		
	Delay	LOS	95th Q	Delay	LOS	95th Q	Delay	LOS	95th Q	Delay	LOS	95th Q
STOP SIGN CONTROL												
101. Gun Club Rd at Aspen Business Park Access (3/4 Mvmt)	1 A			0 A			0 A			0 A		
Eastbound Right	13	B	3'	16	C	10'	12	B	3'	16	C	10'
Northbound Left	13	B	13'	13	B	3'	11	B	10'	13	B	3'
Northbound Through	0	A	0'	0	A	0'	0	A	0'	0	A	0'
Southbound Through	0	A	0'	0	A	0'	0	A	0'	0	A	0'
Southbound Right	0	A	0'	0	A	0'	0	A	0'	0	A	0'
104. Gun Club Rd at North Access [3/4 Mvmt]	0 A			0 A			0 A			0 A		
Eastbound Right	15	C	3'	13	B	5'	25	D	3'	36	E	20'
Northbound Left	14	B	5'	10	A	3'	56	F	23'	71	F	20'
Northbound Through	0	A	0'	0	A	0'	0	A	0'	0	A	0'
Southbound Through	0	A	0'	0	A	0'	0	A	0'	0	A	0'
Southbound Right	0	A	0'	0	A	0'	0	A	0'	0	A	0'
SIGNAL CONTROL												
103. Gun Club Rd at Addison Avenue	14 B			18 B			15 B			24 C		
Eastbound Left	38	D	83'	50	D	275'	49	D	103'	52	D	361'
Eastbound Through+Right	41	D	24'	36	D	42'	52	D	28'	46	D	48'
Westbound Left	36	D	11'	34	C	30'	47	D	42'	49	D	103'
Westbound Through+Right	41	D	21'	39	D	27'	50	D	6'	52	D	13'
Northbound Left	11	B	31'	16	B	11'	12	B	46'	23	C	24'
Northbound Through	12	B	287'	5	A	244'	15	B	569'	15	B	589'
Northbound Right	8	A	0'	0	A	0'	8	A	0'	9	A	0'
Southbound Left	9	A	12'	10	B	19'	20	C	65'	31	C	116'
Southbound Through	15	B	362'	23	C	530'	13	B	380'	25	C	624'
Southbound Right	10	B	35'	12	B	0'	9	A	40'	13	B	0'

Note: Delay represented in average seconds per vehicle.

Intersection

Int Delay, s/veh 0.5

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	0	10	69	1118	1087	100
Future Vol, veh/h	0	10	69	1118	1087	100
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	200	-	-	200
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	95	95	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	11	75	1177	1144	109

Major/Minor	Minor2	Major1	Major2		
Conflicting Flow All	-	572	1253	0	-
Stage 1	-	-	-	-	-
Stage 2	-	-	-	-	-
Critical Hdwy	-	6.94	4.14	-	-
Critical Hdwy Stg 1	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-
Follow-up Hdwy	-	3.32	2.22	-	-
Pot Cap-1 Maneuver	0	463	551	-	-
Stage 1	0	-	-	-	-
Stage 2	0	-	-	-	-
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	-	463	551	-	-
Mov Cap-2 Maneuver	-	-	-	-	-
Stage 1	-	-	-	-	-
Stage 2	-	-	-	-	-

Approach	EB	NB	SB	
HCM Control Delay, s	13	0.8	0	
HCM LOS	B			

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	551	-	463	-	-
HCM Lane V/C Ratio	0.136	-	0.023	-	-
HCM Control Delay (s)	12.6	-	13	-	-
HCM Lane LOS	B	-	B	-	-
HCM 95th %tile Q(veh)	0.5	-	0.1	-	-

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑	↑	↑	↑	↑↑	↑	↑	↑↑	↑
Traffic Volume (vph)	81	1	5	1	77	1026	15	20	1161	202
Future Volume (vph)	81	1	5	1	77	1026	15	20	1161	202
Turn Type	pm+pt	NA	pm+pt	NA	pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases	7	4	3	8	5	2		1	6	
Permitted Phases						2		2	6	
Detector Phase	7	4	3	8	5	2	2	1	6	6
Switch Phase										
Minimum Initial (s)	5.0	5.0	5.0	8.0	5.0	8.0	8.0	5.0	8.0	8.0
Minimum Split (s)	11.0	38.0	11.0	38.0	11.0	28.0	28.0	11.0	28.0	28.0
Total Split (s)	11.0	38.0	11.0	38.0	11.0	30.0	30.0	11.0	30.0	30.0
Total Split (%)	12.2%	42.2%	12.2%	42.2%	12.2%	33.3%	33.3%	12.2%	33.3%	33.3%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0
Lead/Lag	Lead	Lag	Lead	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes									
Recall Mode	None	None	None	None	None	C-Max	C-Max	None	C-Max	C-Max

Intersection Summary

Cycle Length: 90

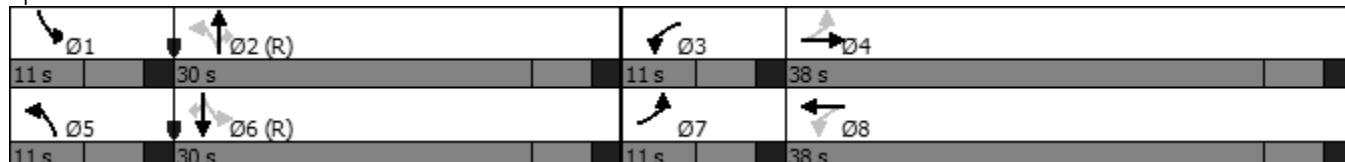
Actuated Cycle Length: 90

Offset: 0 (0%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 100

Control Type: Actuated-Coordinated

Splits and Phases: 103: Gun Club Road & Addison Avenue





Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	88	24	5	17	84	1115	16	22	1262	220
v/c Ratio	0.59	0.14	0.03	0.11	0.27	0.43	0.01	0.06	0.53	0.19
Control Delay	51.0	17.0	30.4	20.2	6.6	9.2	0.0	5.3	12.7	2.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	51.0	17.0	30.4	20.2	6.6	9.2	0.0	5.3	12.7	2.3
Queue Length 50th (ft)	51	1	3	1	7	68	0	2	171	0
Queue Length 95th (ft)	83	24	11	21	31	287	0	12	362	35
Internal Link Dist (ft)		564		952		985			915	
Turn Bay Length (ft)	300		350		350		350	350		350
Base Capacity (vph)	150	581	158	579	312	2582	1204	374	2364	1130
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.59	0.04	0.03	0.03	0.27	0.43	0.01	0.06	0.53	0.19

Intersection Summary

HCM 6th Signalized Intersection Summary
11/22/2023

103: Gun Club Road & Addison Avenue
2027 Bkgnd + Proj (PCE Sensitivity Analysis) - AM Peak Hour

Movement	EBL	EBT	EBC	WBL	WBT	WBC	NBL	NBT	NBC	SBL	SBT	SBC
Lane Configurations	↑	↑		↑	↑		↑	↑↑		↑	↑↑	↑
Traffic Volume (veh/h)	81	1	21	5	1	15	77	1026	15	20	1161	202
Future Volume (veh/h)	81	1	21	5	1	15	77	1026	15	20	1161	202
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00			1.00	1.00		1.00	1.00		1.00	1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	88	1	23	5	1	16	84	1115	16	22	1262	220
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	250	4	98	242	6	91	285	2109	941	325	2019	901
Arrive On Green	0.06	0.06	0.06	0.05	0.06	0.06	0.05	0.59	0.59	0.02	0.57	0.57
Sat Flow, veh/h	1781	66	1529	1781	94	1505	1781	3554	1585	1781	3554	1585
Grp Volume(v), veh/h	88	0	24	5	0	17	84	1115	16	22	1262	220
Grp Sat Flow(s), veh/h/ln	1781	0	1595	1781	0	1599	1781	1777	1585	1781	1777	1585
Q Serve(g_s), s	4.1	0.0	1.3	0.2	0.0	0.9	1.7	16.7	0.4	0.5	21.4	6.3
Cycle Q Clear(g_c), s	4.1	0.0	1.3	0.2	0.0	0.9	1.7	16.7	0.4	0.5	21.4	6.3
Prop In Lane	1.00			1.00		0.94	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	250	0	103	242	0	97	285	2109	941	325	2019	901
V/C Ratio(X)	0.35	0.00	0.23	0.02	0.00	0.17	0.30	0.53	0.02	0.07	0.62	0.24
Avail Cap(c_a), veh/h	250	0	567	248	0	569	297	2109	941	382	2019	901
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	37.0	0.0	40.0	35.5	0.0	40.1	10.3	10.8	7.5	8.8	13.0	9.7
Incr Delay (d2), s/veh	0.8	0.0	1.2	0.0	0.0	0.8	0.6	1.0	0.0	0.1	1.5	0.6
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	1.8	0.0	0.5	0.1	0.0	0.4	0.5	5.3	0.1	0.1	7.0	2.2
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	37.8	0.0	41.2	35.6	0.0	41.0	10.8	11.8	7.5	8.9	14.5	10.4
LnGrp LOS	D	A	D	D	A	D	B	B	A	A	B	B
Approach Vol, veh/h	112				22			1215			1504	
Approach Delay, s/veh	38.5				39.7			11.7			13.8	
Approach LOS	D				D			B			B	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	8.1	59.4	10.7	11.8	10.4	57.1	11.0	11.5				
Change Period (Y+Rc), s	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0				
Max Green Setting (Gmax), s	5.0	24.0	5.0	32.0	5.0	24.0	5.0	32.0				
Max Q Clear Time (g_c+l1), s	2.5	18.7	2.2	3.3	3.7	23.4	6.1	2.9				
Green Ext Time (p_c), s	0.0	3.0	0.0	0.1	0.0	0.5	0.0	0.0				
Intersection Summary												
HCM 6th Ctrl Delay				14.1								
HCM 6th LOS				B								

Intersection

Int Delay, s/veh 0.2

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	0	6	21	1101	1377	91
Future Vol, veh/h	0	6	21	1101	1377	91
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	300	-	-	300
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	7	23	1197	1497	99

Major/Minor	Minor2	Major1	Major2		
Conflicting Flow All	-	749	1596	0	-
Stage 1	-	-	-	-	-
Stage 2	-	-	-	-	-
Critical Hdwy	-	6.94	4.14	-	-
Critical Hdwy Stg 1	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-
Follow-up Hdwy	-	3.32	2.22	-	-
Pot Cap-1 Maneuver	0	354	407	-	-
Stage 1	0	-	-	-	-
Stage 2	0	-	-	-	-
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	-	354	407	-	-
Mov Cap-2 Maneuver	-	-	-	-	-
Stage 1	-	-	-	-	-
Stage 2	-	-	-	-	-

Approach	EB	NB	SB	
HCM Control Delay, s	15.4	0.3	0	
HCM LOS	C			

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	407	-	354	-	-
HCM Lane V/C Ratio	0.056	-	0.018	-	-
HCM Control Delay (s)	14.4	-	15.4	-	-
HCM Lane LOS	B	-	C	-	-
HCM 95th %tile Q(veh)	0.2	-	0.1	-	-

Intersection

Int Delay, s/veh 0.3

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	0	37	10	1454	1400	15
Future Vol, veh/h	0	37	10	1454	1400	15
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	200	-	-	200
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	95	95	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	40	11	1531	1474	16

Major/Minor	Minor2	Major1	Major2		
Conflicting Flow All	-	737	1490	0	-
Stage 1	-	-	-	-	-
Stage 2	-	-	-	-	-
Critical Hdwy	-	6.94	4.14	-	-
Critical Hdwy Stg 1	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-
Follow-up Hdwy	-	3.32	2.22	-	-
Pot Cap-1 Maneuver	0	361	447	-	-
Stage 1	0	-	-	-	-
Stage 2	0	-	-	-	-
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	-	361	447	-	-
Mov Cap-2 Maneuver	-	-	-	-	-
Stage 1	-	-	-	-	-
Stage 2	-	-	-	-	-

Approach	EB	NB	SB	
HCM Control Delay, s	16.2	0.1	0	
HCM LOS	C			

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	447	-	361	-	-
HCM Lane V/C Ratio	0.024	-	0.111	-	-
HCM Control Delay (s)	13.3	-	16.2	-	-
HCM Lane LOS	B	-	C	-	-
HCM 95th %tile Q(veh)	0.1	-	0.4	-	-

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑ ↗	↑ ↘	↑ ↗	↑ ↘	↑ ↗	↑↑	↑ ↗	↑ ↘	↑↑	↑ ↗
Traffic Volume (vph)	288	3	23	3	38	1403	13	33	1316	71
Future Volume (vph)	288	3	23	3	38	1403	13	33	1316	71
Turn Type	pm+pt	NA	pm+pt	NA	pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases	7	4	3	8	5	2		1	6	
Permitted Phases						2		2	6	
Detector Phase	7	4	3	8	5	2	2	1	6	6
Switch Phase										
Minimum Initial (s)	5.0	8.0	5.0	8.0	5.0	8.0	8.0	5.0	8.0	8.0
Minimum Split (s)	11.0	33.0	11.0	33.0	11.0	28.0	28.0	11.0	28.0	28.0
Total Split (s)	16.0	33.0	11.0	28.0	11.0	35.0	35.0	11.0	35.0	35.0
Total Split (%)	17.8%	36.7%	12.2%	31.1%	12.2%	38.9%	38.9%	12.2%	38.9%	38.9%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0
Lead/Lag	Lead	Lag	Lead	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes									
Recall Mode	None	None	None	None	None	C-Max	C-Max	None	C-Max	C-Max

Intersection Summary

Cycle Length: 90

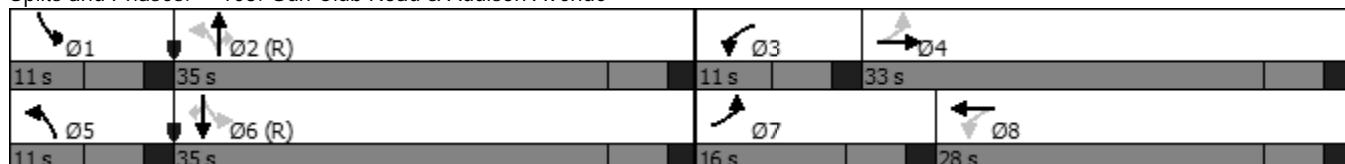
Actuated Cycle Length: 90

Offset: 0 (0%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 105

Control Type: Actuated-Coordinated

Splits and Phases: 103: Gun Club Road & Addison Avenue





Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	313	86	25	28	41	1525	14	36	1430	77
v/c Ratio	1.29	0.32	0.15	0.17	0.18	0.71	0.01	0.17	0.66	0.07
Control Delay	189.2	12.3	31.1	19.5	6.2	11.8	0.0	8.3	17.0	0.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	189.2	12.3	31.1	19.5	6.2	11.8	0.0	8.3	17.0	0.1
Queue Length 50th (ft)	~257	2	14	2	3	143	0	4	245	0
Queue Length 95th (ft)	#275	42	30	27	m11	m#244	m0	19	#530	0
Internal Link Dist (ft)		564		952		985			915	
Turn Bay Length (ft)	300		350		350		350	350		350
Base Capacity (vph)	243	536	164	413	228	2156	1035	211	2153	1034
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	1.29	0.16	0.15	0.07	0.18	0.71	0.01	0.17	0.66	0.07

Intersection Summary

- ~ Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.
- m Volume for 95th percentile queue is metered by upstream signal.

HCM 6th Signalized Intersection Summary
11/22/2023

103: Gun Club Road & Addison Avenue
2027 Bkgnd + Proj (PCE Sensitivity Analysis) - PM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑		↑	↑		↑	↑↑	↑	↑	↑↑	↑
Traffic Volume (veh/h)	288	3	76	23	3	23	38	1403	13	33	1316	71
Future Volume (veh/h)	288	3	76	23	3	23	38	1403	13	33	1316	71
Initial Q (Q _b), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00			1.00	1.00		1.00	1.00	1.00	1.00	1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No			No			No		
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	313	3	83	25	3	25	41	1525	14	36	1430	77
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	375	8	218	292	15	124	205	1788	797	307	1778	793
Arrive On Green	0.11	0.14	0.14	0.06	0.09	0.09	0.07	1.00	1.00	0.03	0.50	0.50
Sat Flow, veh/h	1781	56	1538	1781	173	1439	1781	3554	1585	1781	3554	1585
Grp Volume(v), veh/h	313	0	86	25	0	28	41	1525	14	36	1430	77
Grp Sat Flow(s), veh/h/ln	1781	0	1594	1781	0	1611	1781	1777	1585	1781	1777	1585
Q Serve(g_s), s	10.0	0.0	4.4	1.1	0.0	1.5	1.0	0.0	0.0	0.9	30.3	2.3
Cycle Q Clear(g_c), s	10.0	0.0	4.4	1.1	0.0	1.5	1.0	0.0	0.0	0.9	30.3	2.3
Prop In Lane	1.00			0.97	1.00		0.89	1.00	1.00	1.00	1.00	1.00
Lane Grp Cap(c), veh/h	375	0	226	292	0	139	205	1788	797	307	1778	793
V/C Ratio(X)	0.84	0.00	0.38	0.09	0.00	0.20	0.20	0.85	0.02	0.12	0.80	0.10
Avail Cap(c_a), veh/h	375	0	478	292	0	394	240	1788	797	348	1778	793
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	2.00	2.00	2.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	35.3	0.0	35.0	33.6	0.0	38.2	15.0	0.0	0.0	10.0	18.8	11.8
Incr Delay (d2), s/veh	15.0	0.0	1.1	0.1	0.0	0.7	0.5	5.4	0.0	0.2	4.0	0.2
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	3.4	0.0	1.8	0.5	0.0	0.6	0.3	1.3	0.0	0.3	11.2	0.8
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	50.4	0.0	36.1	33.7	0.0	39.0	15.5	5.4	0.0	10.2	22.8	12.0
LnGrp LOS	D	A	D	C	A	D	B	A	A	B	C	B
Approach Vol, veh/h	399				53			1580			1543	
Approach Delay, s/veh	47.3				36.5			5.6			21.9	
Approach LOS	D				D			A			C	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	9.0	51.3	11.0	18.8	9.2	51.0	16.0	13.8				
Change Period (Y+Rc), s	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0				
Max Green Setting (Gmax), s	5.0	29.0	5.0	27.0	5.0	29.0	10.0	22.0				
Max Q Clear Time (g_c+l1), s	2.9	2.0	3.1	6.4	3.0	32.3	12.0	3.5				
Green Ext Time (p_c), s	0.0	12.0	0.0	0.4	0.0	0.0	0.0	0.1				

Intersection Summary

HCM 6th Ctrl Delay 17.8
HCM 6th LOS B

Notes

User approved pedestrian interval to be less than phase max green.

Intersection

Int Delay, s/veh 0.2

Movement	EBL	EBC	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	0	30	14	1700	1390	54
Future Vol, veh/h	0	30	14	1700	1390	54
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	300	-	-	300
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	33	15	1848	1511	59

Major/Minor	Minor2	Major1	Major2		
Conflicting Flow All	-	756	1570	0	-
Stage 1	-	-	-	-	-
Stage 2	-	-	-	-	-
Critical Hdwy	-	6.94	4.14	-	-
Critical Hdwy Stg 1	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-
Follow-up Hdwy	-	3.32	2.22	-	-
Pot Cap-1 Maneuver	0	*502	*751	-	-
Stage 1	0	-	-	-	-
Stage 2	0	-	-	-	-
Platoon blocked, %	1	1	-	-	-
Mov Cap-1 Maneuver	-	*502	*751	-	-
Mov Cap-2 Maneuver	-	-	-	-	-
Stage 1	-	-	-	-	-
Stage 2	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	12.7	0.1	0
HCM LOS	B		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	* 751	-	502	-	-
HCM Lane V/C Ratio	0.02	-	0.065	-	-
HCM Control Delay (s)	9.9	-	12.7	-	-
HCM Lane LOS	A	-	B	-	-
HCM 95th %tile Q(veh)	0.1	-	0.2	-	-

Notes

~: Volume exceeds capacity \$: Delay exceeds 300s +: Computation Not Defined *: All major volume in platoon

Intersection

Int Delay, s/veh 0.2

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	0	10	69	2403	1782	100
Future Vol, veh/h	0	10	69	2403	1782	100
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	250	-	-	250
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	98	98	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	11	75	2452	1818	109

Major/Minor	Minor2	Major1	Major2		
Conflicting Flow All	-	909	1927	0	-
Stage 1	-	-	-	-	-
Stage 2	-	-	-	-	-
Critical Hdwy	-	7.14	5.34	-	-
Critical Hdwy Stg 1	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-
Follow-up Hdwy	-	3.92	3.12	-	-
Pot Cap-1 Maneuver	0	*511	*643	-	-
Stage 1	0	-	-	-	-
Stage 2	0	-	-	-	-
Platoon blocked, %	1	1	-	-	-
Mov Cap-1 Maneuver	-	*511	*643	-	-
Mov Cap-2 Maneuver	-	-	-	-	-
Stage 1	-	-	-	-	-
Stage 2	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	12.2	0.3	0
HCM LOS	B		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	* 643	-	511	-	-
HCM Lane V/C Ratio	0.117	-	0.021	-	-
HCM Control Delay (s)	11.3	-	12.2	-	-
HCM Lane LOS	B	-	B	-	-
HCM 95th %tile Q(veh)	0.4	-	0.1	-	-

Notes

~: Volume exceeds capacity \$: Delay exceeds 300s +: Computation Not Defined *: All major volume in platoon

Lane Group	EBL	EBT	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑	↑	↑	↑	↑	↑↑↑	↑	↑	↑↑↑	↑
Traffic Volume (vph)	81	1	25	1	50	77	2276	50	80	1836	202
Future Volume (vph)	81	1	25	1	50	77	2276	50	80	1836	202
Turn Type	pm+pt	NA	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases	7	4	3	8		5	2		1	6	
Permitted Phases					8	2		2	6		6
Detector Phase	7	4	3	8	8	5	2	2	1	6	6
Switch Phase											
Minimum Initial (s)	5.0	8.0	5.0	8.0	8.0	5.0	8.0	8.0	5.0	8.0	8.0
Minimum Split (s)	11.0	38.0	11.0	38.0	38.0	11.0	28.0	28.0	11.0	28.0	28.0
Total Split (s)	11.0	38.0	38.0	38.0	38.0	11.0	28.0	28.0	11.0	28.0	28.0
Total Split (%)	9.6%	33.0%	33.0%	33.0%	33.0%	9.6%	24.3%	24.3%	9.6%	24.3%	24.3%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0
Lead/Lag	Lead	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes										
Recall Mode	None	None	None	None	None	None	C-Max	C-Max	None	C-Max	C-Max

Intersection Summary

Cycle Length: 115

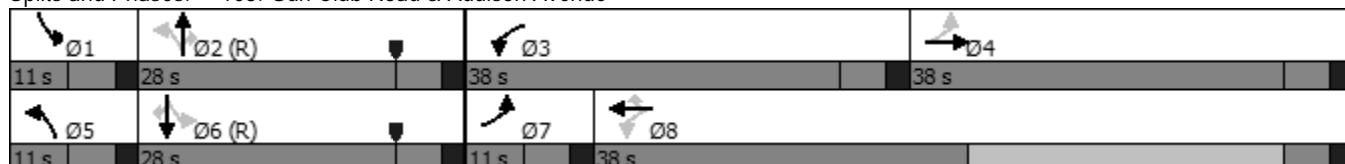
Actuated Cycle Length: 115

Offset: 19 (17%), Referenced to phase 2:NBT and 6:SBTL, Start of Yellow

Natural Cycle: 110

Control Type: Actuated-Coordinated

Splits and Phases: 103: Gun Club Road & Addison Avenue





Lane Group	EBL	EBT	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	88	24	27	1	54	84	2322	54	87	1873	220
v/c Ratio	0.59	0.15	0.14	0.01	0.21	0.41	0.71	0.05	0.46	0.54	0.19
Control Delay	62.1	22.0	40.4	47.0	1.8	14.5	17.4	0.1	21.2	13.5	2.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	62.1	22.0	40.4	47.0	1.8	14.5	17.4	0.1	21.2	13.5	2.5
Queue Length 50th (ft)	~76	1	17	1	0	15	416	0	16	287	6
Queue Length 95th (ft)	103	28	42	6	0	46	569	0	65	380	40
Internal Link Dist (ft)		564		1036			985			915	
Turn Bay Length (ft)	300					300		300	300		300
Base Capacity (vph)	150	460	492	955	881	203	3263	1086	189	3449	1137
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.59	0.05	0.05	0.00	0.06	0.41	0.71	0.05	0.46	0.54	0.19

Intersection Summary

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

HCM 6th Signalized Intersection Summary
11/22/2023

103: Gun Club Road & Addison Avenue
2050 Bkgd + Project (PCEs Sensitivity Analysis) - AM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑		↑	↑	↑	↑	↑↑↑	↑	↑	↑↑↑	↑
Traffic Volume (veh/h)	81	1	21	25	1	50	77	2276	50	80	1836	202
Future Volume (veh/h)	81	1	21	25	1	50	77	2276	50	80	1836	202
Initial Q (Q _b), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00			1.00	1.00		1.00	1.00	1.00	1.00	1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No			No		No		No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	88	1	23	27	1	0	84	2322	54	87	1873	220
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.98	0.92	0.92	0.98	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	234	4	103	212	125		220	3270	1015	186	3271	1016
Arrive On Green	0.04	0.07	0.07	0.04	0.07	0.00	0.04	0.64	0.64	0.04	0.64	0.64
Sat Flow, veh/h	1781	66	1529	1781	1870	1585	1781	5106	1585	1781	5106	1585
Grp Volume(v), veh/h	88	0	24	27	1	0	84	2322	54	87	1873	220
Grp Sat Flow(s), veh/h/ln	1781	0	1595	1781	1870	1585	1781	1702	1585	1781	1702	1585
Q Serve(g_s), s	5.0	0.0	1.6	1.6	0.1	0.0	1.8	34.5	1.5	1.9	23.9	6.7
Cycle Q Clear(g_c), s	5.0	0.0	1.6	1.6	0.1	0.0	1.8	34.5	1.5	1.9	23.9	6.7
Prop In Lane	1.00		0.96	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	234	0	108	212	125		220	3270	1015	186	3271	1016
V/C Ratio(X)	0.38	0.00	0.22	0.13	0.01		0.38	0.71	0.05	0.47	0.57	0.22
Avail Cap(c_a), veh/h	234	0	444	632	520		225	3270	1015	191	3271	1016
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	0.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	47.9	0.0	50.7	46.4	50.1	0.0	10.5	13.6	7.7	18.3	11.7	8.6
Incr Delay (d2), s/veh	1.0	0.0	1.0	0.3	0.0	0.0	1.1	1.3	0.1	1.8	0.7	0.5
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	2.4	0.0	0.7	0.7	0.0	0.0	0.6	11.0	0.5	1.3	7.6	2.3
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	48.9	0.0	51.8	46.6	50.1	0.0	11.6	15.0	7.8	20.2	12.5	9.1
LnGrp LOS	D	A	D	D	D		B	B	A	C	B	A
Approach Vol, veh/h	112				28			2460			2180	
Approach Delay, s/veh	49.5				46.8			14.7			12.4	
Approach LOS	D				D			B			B	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	10.7	79.6	10.9	13.8	10.7	79.7	11.0	13.7				
Change Period (Y+Rc), s	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0				
Max Green Setting (Gmax), s	5.0	22.0	32.0	32.0	5.0	22.0	5.0	32.0				
Max Q Clear Time (g_c+l1), s	3.9	36.5	3.6	3.6	3.8	25.9	7.0	2.1				
Green Ext Time (p_c), s	0.0	0.0	0.0	0.1	0.0	0.0	0.0	0.0				
Intersection Summary												
HCM 6th Ctrl Delay				14.7								
HCM 6th LOS				B								
Notes												
Unsignalized Delay for [WBR] is excluded from calculations of the approach delay and intersection delay.												

Intersection						
Int Delay, s/veh	0.3					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations			↑	↑↑↑↑	↑↑↑↑	↑
Traffic Vol, veh/h	0	6	21	2386	2112	91
Future Vol, veh/h	0	6	21	2386	2112	91
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	300	-	-	300
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	98	98	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	7	23	2435	2155	99
Major/Minor	Minor2	Major1		Major2		
Conflicting Flow All	-	1078	2254	0	-	0
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	7.14	5.34	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.92	3.12	-	-	-
Pot Cap-1 Maneuver	0	184	93	-	-	-
Stage 1	0	-	-	-	-	-
Stage 2	0	-	-	-	-	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	-	184	93	-	-	-
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Approach	EB	NB		SB		
HCM Control Delay, s	25.3	0.5		0		
HCM LOS	D					
Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR	
Capacity (veh/h)	93	-	184	-	-	
HCM Lane V/C Ratio	0.245	-	0.035	-	-	
HCM Control Delay (s)	55.9	-	25.3	-	-	
HCM Lane LOS	F	-	D	-	-	
HCM 95th %tile Q(veh)	0.9	-	0.1	-	-	

Intersection

Int Delay, s/veh 0.2

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	0	37	10	2064	2375	15
Future Vol, veh/h	0	37	10	2064	2375	15
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	250	-	-	250
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	98	98	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	40	11	2106	2423	16

Major/Minor	Minor2	Major1	Major2		
Conflicting Flow All	-	1212	2439	0	-
Stage 1	-	-	-	-	-
Stage 2	-	-	-	-	-
Critical Hdwy	-	7.14	5.34	-	-
Critical Hdwy Stg 1	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-
Follow-up Hdwy	-	3.92	3.12	-	-
Pot Cap-1 Maneuver	0	*378	*476	-	-
Stage 1	0	-	-	-	-
Stage 2	0	-	-	-	-
Platoon blocked, %	1	1	-	-	-
Mov Cap-1 Maneuver	-	*378	*476	-	-
Mov Cap-2 Maneuver	-	-	-	-	-
Stage 1	-	-	-	-	-
Stage 2	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	15.7	0.1	0
HCM LOS	C		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	* 476	-	378	-	-
HCM Lane V/C Ratio	0.023	-	0.106	-	-
HCM Control Delay (s)	12.7	-	15.7	-	-
HCM Lane LOS	B	-	C	-	-
HCM 95th %tile Q(veh)	0.1	-	0.4	-	-

Notes

~: Volume exceeds capacity \$: Delay exceeds 300s +: Computation Not Defined *: All major volume in platoon

Lane Group	EBL	EBT	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑ ↗	↑ ↘	↑ ↗	↑ ↘	↗ ↗	↑ ↗	↑↑↑	↗ ↗	↑ ↗	↑↑↑	↗ ↗
Traffic Volume (vph)	288	3	88	3	103	38	1978	48	128	2226	71
Future Volume (vph)	288	3	88	3	103	38	1978	48	128	2226	71
Turn Type	pm+pt	NA	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases	7	4	3	8		5	2		1	6	
Permitted Phases					8	2		2	6		6
Detector Phase	7	4	3	8	8	5	2	2	1	6	6
Switch Phase											
Minimum Initial (s)	5.0	8.0	5.0	8.0	8.0	5.0	8.0	8.0	5.0	8.0	8.0
Minimum Split (s)	11.0	38.0	11.0	38.0	38.0	11.0	28.0	28.0	11.0	28.0	28.0
Total Split (s)	25.0	39.0	14.0	28.0	28.0	11.0	55.0	55.0	12.0	56.0	56.0
Total Split (%)	20.8%	32.5%	11.7%	23.3%	23.3%	9.2%	45.8%	45.8%	10.0%	46.7%	46.7%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0
Lead/Lag	Lead	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes										
Recall Mode	Max	None	None	None	None	None	C-Max	C-Max	None	C-Max	C-Max

Intersection Summary

Cycle Length: 120

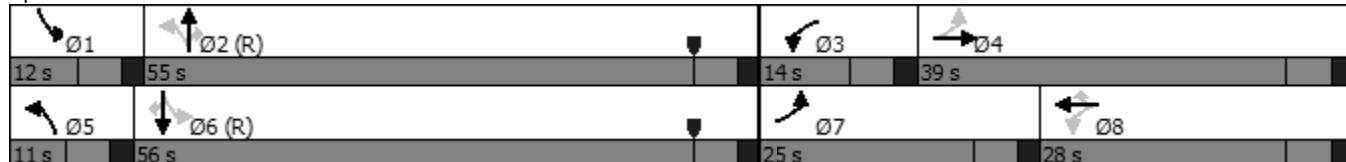
Actuated Cycle Length: 120

Offset: 0 (0%), Referenced to phase 2:NBTL and 6:SBTL, Start of Yellow

Natural Cycle: 110

Control Type: Actuated-Coordinated

Splits and Phases: 103: Gun Club Road & Addison Avenue





Lane Group	EBL	EBT	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	313	86	96	3	112	41	2018	52	139	2271	77
v/c Ratio	0.83	0.24	0.48	0.02	0.40	0.26	0.85	0.06	0.56	0.83	0.08
Control Delay	57.8	11.9	43.5	53.0	4.1	21.5	50.3	0.6	28.2	27.1	0.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	57.8	11.9	43.5	53.0	4.1	21.5	50.3	0.6	28.2	27.1	0.2
Queue Length 50th (ft)	214	2	57	2	0	21	541	0	49	538	0
Queue Length 95th (ft)	#361	48	103	13	0	m24	m589	m0	116	624	0
Internal Link Dist (ft)		564		1036			985			915	
Turn Bay Length (ft)	300				300		300	300		300	
Base Capacity (vph)	379	498	205	341	446	159	2388	844	249	2744	942
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.83	0.17	0.47	0.01	0.25	0.26	0.85	0.06	0.56	0.83	0.08

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

HCM 6th Signalized Intersection Summary
11/22/2023

103: Gun Club Road & Addison Avenue
2050 Bkgd + Project (PCEs Sensitivity Analysis) - PM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑		↑	↑	↑	↑	↑↑↑	↑	↑	↑↑↑	↑
Traffic Volume (veh/h)	288	3	76	88	3	103	38	1978	48	128	2226	71
Future Volume (veh/h)	288	3	76	88	3	103	38	1978	48	128	2226	71
Initial Q (Q _b), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00			1.00	1.00		1.00	1.00	1.00	1.00	1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	313	3	83	96	3	0	41	2018	52	139	2271	77
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.98	0.92	0.92	0.98	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	434	9	244	265	124		143	2681	832	204	2778	862
Arrive On Green	0.16	0.16	0.16	0.07	0.07	0.00	0.04	0.70	0.70	0.05	0.54	0.54
Sat Flow, veh/h	1781	56	1538	1781	1870	1585	1781	5106	1585	1781	5106	1585
Grp Volume(v), veh/h	313	0	86	96	3	0	41	2018	52	139	2271	77
Grp Sat Flow(s), veh/h/ln	1781	0	1594	1781	1870	1585	1781	1702	1585	1781	1702	1585
Q Serve(g_s), s	19.0	0.0	5.8	5.9	0.2	0.0	1.3	30.1	1.2	4.3	43.8	2.8
Cycle Q Clear(g_c), s	19.0	0.0	5.8	5.9	0.2	0.0	1.3	30.1	1.2	4.3	43.8	2.8
Prop In Lane	1.00		0.97	1.00		1.00	1.00		1.00	1.00	1.00	1.00
Lane Grp Cap(c), veh/h	434	0	253	265	124		143	2681	832	204	2778	862
V/C Ratio(X)	0.72	0.00	0.34	0.36	0.02		0.29	0.75	0.06	0.68	0.82	0.09
Avail Cap(c_a), veh/h	434	0	438	266	343		162	2681	832	204	2778	862
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.33	1.33	1.33	1.00	1.00	1.00
Upstream Filter(l)	1.00	0.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	41.9	0.0	44.9	47.7	52.4	0.0	21.7	13.1	8.8	21.8	22.5	13.1
Incr Delay (d2), s/veh	9.9	0.0	0.8	0.8	0.1	0.0	1.1	2.0	0.1	8.9	2.8	0.2
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	9.7	0.0	2.3	2.7	0.1	0.0	0.5	7.2	0.5	2.4	16.0	1.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	51.9	0.0	45.7	48.5	52.4	0.0	22.8	15.1	8.9	30.6	25.3	13.3
LnGrp LOS	D	A	D	D	D		C	B	A	C	C	B
Approach Vol, veh/h	399				99			2111			2487	
Approach Delay, s/veh	50.5				48.7			15.1			25.2	
Approach LOS	D				D			B			C	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	12.0	69.0	13.9	25.1	9.7	71.3	25.0	14.0				
Change Period (Y+Rc), s	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0				
Max Green Setting (Gmax), s	6.0	49.0	8.0	33.0	5.0	50.0	19.0	22.0				
Max Q Clear Time (g_c+l1), s	6.3	32.1	7.9	7.8	3.3	45.8	21.0	2.2				
Green Ext Time (p_c), s	0.0	11.9	0.0	0.4	0.0	3.8	0.0	0.0				

Intersection Summary

HCM 6th Ctrl Delay 23.5
HCM 6th LOS C

Notes

User approved pedestrian interval to be less than phase max green.

Unsignalized Delay for [WBR] is excluded from calculations of the approach delay and intersection delay.

Intersection

Int Delay, s/veh 0.4

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	0	30	14	2355	2395	54
Future Vol, veh/h	0	30	14	2355	2395	54
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	300	-	-	300
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	98	98	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	33	15	2403	2444	59

Major/Minor	Minor2	Major1	Major2		
Conflicting Flow All	-	1222	2503	0	-
Stage 1	-	-	-	-	-
Stage 2	-	-	-	-	-
Critical Hdwy	-	7.14	5.34	-	-
Critical Hdwy Stg 1	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-
Follow-up Hdwy	-	3.92	3.12	-	-
Pot Cap-1 Maneuver	0	147	69	-	-
Stage 1	0	-	-	-	-
Stage 2	0	-	-	-	-
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	-	147	69	-	-
Mov Cap-2 Maneuver	-	-	-	-	-
Stage 1	-	-	-	-	-
Stage 2	-	-	-	-	-

Approach	EB	NB	SB	
HCM Control Delay, s	36.3	0.4	0	
HCM LOS	E			

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	69	-	147	-	-
HCM Lane V/C Ratio	0.221	-	0.222	-	-
HCM Control Delay (s)	71.4	-	36.3	-	-
HCM Lane LOS	F	-	E	-	-
HCM 95th %tile Q(veh)	0.8	-	0.8	-	-